

ISOAP Stage 1 (Screening and Initial Assessment) Long Form

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Checked by:

Cty-Rte-PM: TUO-49-18.514

Major Street: North Washington Street / Golden Chain Highway

Minor Street: (#1) Shaws Flat Road, (#2) School Street

Project EA: N/A

Date: 7/8/2025

Step 1.1 Is ISOAP Required?

Applicability criteria

- ☐ New public road, private road, or high-volume (1,000 ADT) driveway
- ☐ New freeway interchange
- ☒ Change in type of traffic control (stop, yield, signal)
- ☐ Pedestrian hybrid beacon (PHB) at an intersection
- ☐ Major physical changes to intersection approaches (including ramp terminals), such as adding a leg to an intersection or widening to provide an additional through or turn lane

Step 1.2 Determine Intended Project Outcome, Place Type, Design Vehicle, and Gather Data

Determine desired result of project, collaborating with functional units and stakeholders as needed (for example, safety improvement, improve walkability, reduced queuing):
Reduce delay and queuing, improve pedestrian safety

Gather available existing traffic data

Major street:

- Route classification: Conventional Highway
- Lane configuration: One lane in each direction, SB: left-turn and through-right lanes, NB: left-turn, through, and right-turn lanes
- Existing ADT: 7,121 NB (north of Shaws Flat Road/Columbia Way); 7,067 SB (north of Shaws Flat Road/Columbia Way)
- Future ADT:
- Speed limit: 35

Minor street:

- Route classification: Local Road
- Lane configuration: (#1) EB: One lane approach, left-through and right-turn lane; WB: One lane approach, left-through-right lane; (#2) EB One lane approach, left-right lane

- Existing ADT: 1,944 EB (north of North Washington Street/Golden Chain Highway); 1,938 WB (north of North Washington Street/Golden Chain Highway)
- Future ADT:
- Speed limit: 25

Existing land use: Suburban, with a high school to the west, residential uses to the east, and a mix of residential and commercial uses to the north and south

Future land use: Same

In addition, gather if available:

- More detailed counts (hourly, peak hour, turning movement, truck, bicycle, pedestrian, etc.)
- Roadway geometrics
- Collision data

Gather planning information

- Route Concept Report, Transportation Concept Report, or Multimodal Corridor Plan
- Active Transportation Plan
- General Plan or Specific Plan

Future concept of the state highway (long-term and/or ultimate configuration: State Route 49 exists as a 2-lane conventional highway in the study area and is anticipated to remain so.

Determine place type

Select the appropriate place type (may not reflect current land use): Suburban areas

Determine design vehicle

Designated truck network: California Legal Advisory Route

Design vehicle: California Legal truck

Step 1.3 Ped and Bike Planning and Feasibility Assessment

Conduct a qualitative assessment of the needs of bicyclists and peds. Consider land use and connectivity. Take note of schools and senior centers or housing.

Existing pedestrian or bicycle facilities: Concrete sidewalks and marked school crossings exist across State Route 49 and across Shaws Flat Road next to Sonora High School. No bike facilities currently exist in the study area.

Select type of appropriate type of bicycle facility for the major street: Class I bike path/shared use path

Select type of appropriate type of bicycle facility for the minor street: Class III bike route

Any additional comments on future pedestrian or bicycle facilities: The 2020 Tuolumne County Active Transportation Plan includes the Shaws Flat Road Pedestrian & Bicycle Improvements Project, which proposes to "improve and enhance pedestrian/bicycle facilities along Shaws Flat Road" from Sonora High School to Roble Road (p. 80). Additionally, the 2024 Tuolumne County Regional Transportation Plan includes the Gold Rush Shared Use Path, which would enhance pedestrian and bicycle connectivity along the State Route 49 corridor, in its list of priority projects (p. 147). Existing and/or future pedestrian/bicycle traffic could be accommodated with these improved facilities.

Step 1.4 General R/W and Operational Feasibility Assessment

Consider appropriate strategies to analyze considering the right-of-way required using footprints based on typical designs. Use CAP-X or rules of thumb for lane configurations. Use more advanced tools (Synchro, Sidra) if turning movement counts are available.

Attach any exhibits as appropriate.

Comment as needed: Attached memorandum contains general safety and operational feasibility assessment. Operational conditions with Existing (2025) volumes were considered with Existing Control (side-street stop control [SSSC]). Operational conditions with Cumulative (2040) volumes were considered under the following scenarios: Existing Control (SSSC), signalization (SIG), 4-legged roundabout (RAB) and 5-legged RAB.

Step 1.5 Transit and Freight Assessment

Transit considerations

Existing transit providers and routes: Tuolumne County Transit serves the study area via Route 2. Sonora High School is a Flag Stop, meaning that buses will stop when riders are present at the bus stop and when traffic conditions permit safe stoppages.

If transit facilities are currently lacking, will land use changes create a need for more transit facilities in the future? No

Consider the need for transit shelters and passenger queuing, location of bus bays (near side or far side), and whether location requires storage for multiple transit vehicles for transfers.

Comment as needed: Additional bike and pedestrian traffic in the study area due to the Gold Rush Shared Use Path could increase the need for additional transit service. Transit service frequencies could be increased through the study area, which could require additional transit vehicles.

Freight considerations

The appropriate design vehicle was determined in Step 1.2. Consult with the District Truck Access Manager (DTAM) if less than an STAA truck is to be accommodated.

Determine if oversized vehicles need to be accommodated: No

If yes, specify type of vehicle to be accommodated:

Do all truck turning movements need to be accommodated?: No

If no, specify any unnecessary turns (for instance, EBL – eastbound left): (#1) and (#2):
NBL, NBR, SBL, SBR

Step 1.6 Initial Safety Assessment

Analyze existing collision history. Note any existing known collision pattern or other safety considerations: See attached memorandum for details on collision history.

Consider the relative safety of the proposed intersection control strategies. The SPICE tool, FHWA ICE Tool, or SSI methodology may be used.

For each considered strategy, provide a summary of initial safety and operational assessments, accommodations of pedestrians and bicyclists, and/or constraints, note any impacts to transit or freight): Four potential intersection control strategies are being considered: SSSC, SIG, 4-legged RAB, and 5-legged RAB.

Step 1.7 Eliminate Infeasible Strategies

Eliminate strategies that do not satisfy the need, have unmitigable environmental impacts, inadequately address safety, or exceed available and potentially available funding.

List rejected strategies and note reason for elimination: SSSC would not be considered because of excessive delay and queueing.

Step 1.8 Findings and Recommendations

Provide the recommended action, whether there is a single viable option that concludes ISOAP or if there are multiple strategies for proceeding to Stage 2. If there is only one viable strategy but funding is insufficient, consider:

- Other potential funding sources (SHOPP, CMAQ, Minor, ATP, measure, developer fees)
- Phased implementation
- Interim improvements

Recommendation: The traffic signal, 4-legged RAB, and the 5-legged RAB are the intersection control alternatives that have acceptable delay/LOS and queueing. It is recommended that they proceed to Stage 2.

The District ISOAP Coordinator and designated Traffic Operations functional manager, if applicable, will review the ISOAP form. If satisfactory and there is only one viable strategy, the ISOAP form will be submitted to the District Traffic Safety Engineer for

review and concurrence of the recommendation. The District ISOAP Coordinator will respond with comments or an approval memo.

Attachments: Safety and Operational Analysis Memo

State Route 49 & Shaws Flat Road / School Street Intersections

Intersection Safety and Operational Analysis Assessment – Draft

July 2025

Prepared For



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Appendix B. Traffic Counts

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Appendix F. Cumulative (2040) with Bypass Build Conditions with Signal – SimTraffic Outputs

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Appendix H. Cumulative (2040) with Bypass Build Conditions with 5-Legged Roundabout – SIDRA Outputs

Appendix I. Cumulative (2040) without Bypass Build Conditions with 4-Legged and 5-Legged Roundabouts – SimTraffic and SIDRA Outputs

INTRODUCTION

This memorandum discusses the results of Stage 1 of the Intersection Safety and Operational Analysis Process (ISOAP) conducted for the study intersections of State Route 49 (SR 49) & Shaws Flat Road (Intersection 1) and SR 49 & School Street (Intersection 2) in the City of Sonora, California. The study intersections are shown in **Figure 1**.

The ISOAP is an updated intersection evaluation process utilized by the California Department of Transportation (Caltrans) to evaluate proposed traffic control, design geometrics, and other access improvements for intersections on the State Highway System. ISOAP represents a data-driven and performance-based framework that enables stakeholders to identify optimal solutions and strategies for new or improved intersections that consider all users.

This ISOAP was triggered by the Gold Rush Multi-Use Path Project (the “Project”) which is being implemented by the Tuolumne County Transportation Council (TCTC). The Project, when complete, would extend a total of approximately 15 miles and would continuously connect the historic gold rush settlements of Columbia, Sonora, and Jamestown, in addition to the Chicken Ranch Tribal Lands. The Project would implement a Class I ped-bike facility in addition to traffic, intersection, and street crossing safety enhancements, ADA improvements, bike and pedestrian amenities, and transit stop opportunities along its corridor. The Project is divided into three phases, with Phase 2 extending from Sonora High School next to the study intersections to Parrots Ferry Road along SR 49.

Additionally, the local roadway network in the vicinity of the Project is expected to experience changes in traffic volumes due to the planned implementation of the Greenley Road Extension/North-South Connector (the “Bypass”). The Bypass would connect Fir Drive near the interchange of State Route 108 & Mono Way to SR 49 near Preston Place via an alignment east of the City of Sonora. The Bypass would allow a portion of existing and future north-south traffic along SR 49 to circumnavigate the City of Sonora. Tuolumne County has included the Bypass in the Regional Transportation Plan and has jointly studied the Bypass with TCTC, the City of Sonora, the State of California, and the Federal Highway Administration since 1975 (**Appendix A**). The Bypass is currently in the Project Initiation Document stage with Caltrans and expected to be constructed in phases in the next few decades. Because implementation is expected to take a long time, the ISOAP was also conducted without the Bypass, based on a request from the stakeholders.

The purpose of this memorandum is to document and present the operational analyses of different intersection control alternatives at the study intersections to accommodate local and regional growth, including side-street stop control (SSSC; existing control), traffic signal (SIG) control, and roundabout (RAB) control. The following scenarios were simulated to analyze the adequacy of various intersection control alternatives under various expected conditions:

No Build

- Existing (2025) Conditions with SSSC

- Cumulative (2040) Conditions with SSSC

Build with Bypass

- Cumulative (2040) Conditions with SIG
- Cumulative (2040) Conditions with 4-legged RAB
- Cumulative (2040) Conditions with 5-legged RAB

Build without Bypass

- Cumulative (2040) Conditions with 4-legged RAB
- Cumulative (2040) Conditions with 5-legged RAB

This analysis is part of Stage 1 of ISOAP to identify potential improvements at the study intersections. The operational analyses additionally confirm the potential improvements at the study intersections identified in the *Vision Sonora Plan*.

Caltrans provided Traffic Accident Surveillance and Analysis System (TASAS) crash data at the intersection. The Federal Highway Administration (FHWA) Interactive Highway Safety Design Model (IHSDM) software and National Cooperative Highway Research Program (NCHRP) Report 1043 (Guide for Roundabouts) crash reduction factors were applied to recalculate crash reduction factors to quantify potential safety benefits with implementation of the various control measures.

Figure 1. Existing Intersection Configuration



Source: Nemap.com

STUDY AREA AND DATA COLLECTION

Study Area

The traffic study limits within Caltrans right-of-way extend along SR 49 between Shaws Flat Road and School Street. The study intersections are listed below:

Study Intersections

1. SR 49 & Shaws Flat Road
2. SR 49/North Washington Street & School Street

Roadway Network

SR 49 is a major two-lane roadway that runs in the north-south direction in the Project vicinity that serves as a major roadway for vehicles traveling to and from Tuolumne County and the City of Sonora. The posted speed limit on SR 49 is 35 mph within the Project area.

Shaws Flat Road is an east-west roadway in Tuolumne County, serving as a local access route connecting the community to SR 49. The posted speed limit along Shaws Flat Road is 25 mph.

School Street is a local east-west roadway in Sonora that connects SR 49 to Snell Street. The roadway provides direct access to municipal facilities and to Sonora High School. The roadway experiences high levels of congestion during the AM and School peak hours when students and staff commute to and from Sonora High School.

Columbia Way is a residential street that runs primarily in an east-west direction, connecting neighborhoods near downtown Sonora. The posted speed limit on Columbia Way is 25 mph, consistent with other residential streets in the area.

Data Collection

Data was collected during the AM, PM, and School peak periods along the SR 49 corridor to determine vehicle, pedestrian, and bicycle volumes, and the location of congestion and queues within the study area.

Intersection Turning Movement Volumes

Intersection turning movement counts, including vehicle, pedestrian, and bicycle data, were conducted at SR 49 & Shaws Flat Road (Intersection 1) during the AM peak period (7:00 – 9:00 AM), the School peak period (2:00 – 4:00 PM), and the PM peak period (4:00 – 6:00 PM) on a non-holiday weekday, Tuesday, May 20, 2025, while local schools were in session. To ensure a conservative analysis, the highest 60-minute volumes within the AM, School, and PM peak periods were identified and used.

As data collection did not occur at SR 49 & School Street (Intersection 2), demand volumes were developed by using intersection turning movement counts from the *2015 Tuolumne County General Plan and Regional Transportation Plan Update Environmental Impact Report Traffic*

Study for the AM and PM peak hours, and from the 2013 *Vision Sonora Plan* for the School peak hour. Counts from these sources were proportionally grown by turning movement to 2025 demand volumes.

Turning movement volumes between SR 49 & Shaws Flat Road (Intersection 1) and SR 49 & School Street (Intersection 2) were balanced.

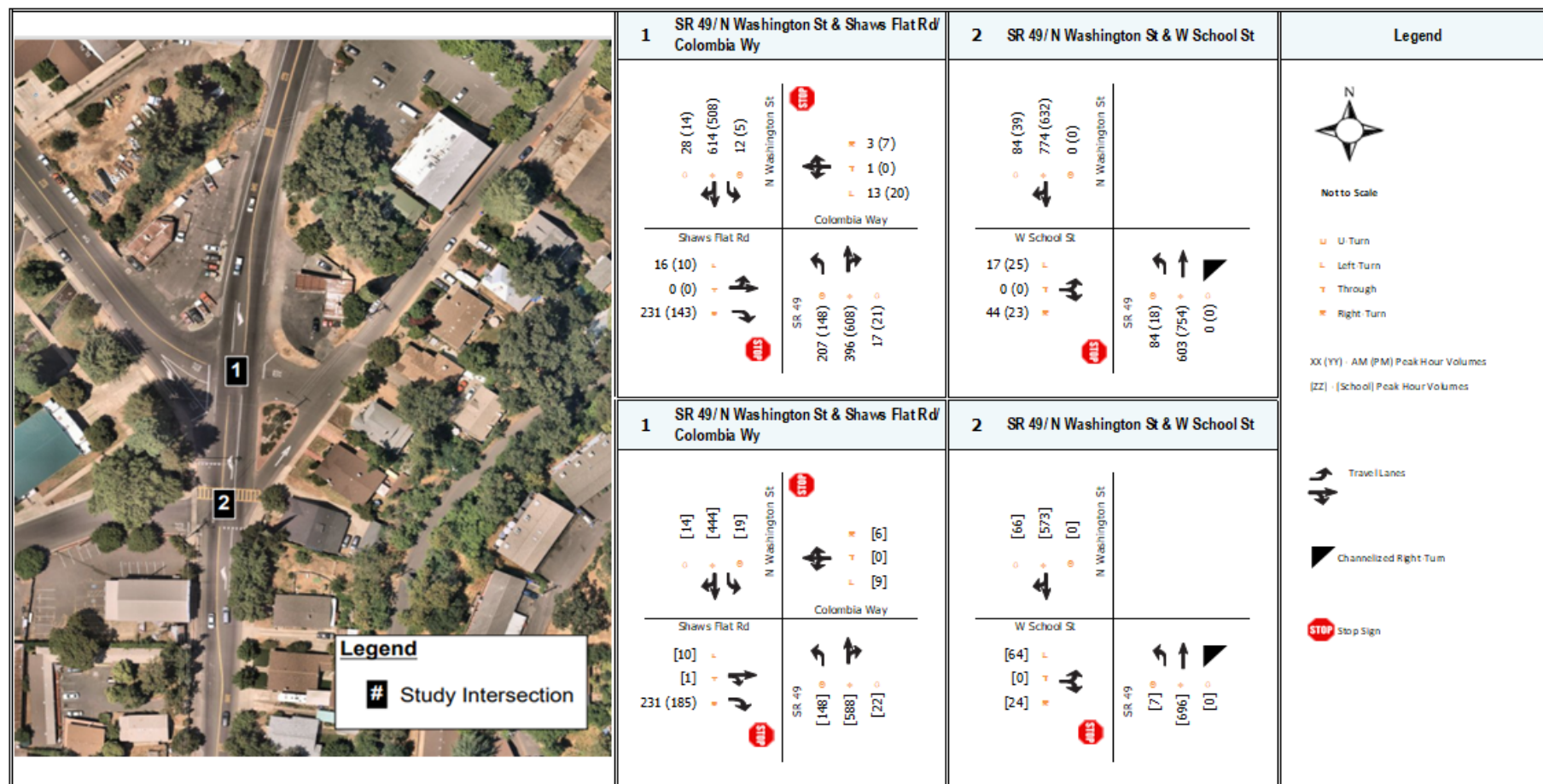
The existing intersection lane configurations and turning movement volumes for the AM, School, and PM peak hours are shown in **Figure 2**.

Annual Daily Traffic (ADT)

ADT and vehicle classification data were collected along Shaws Flat Road west of SR 49 and along SR 49 north of Shaws Flat Road.

The raw traffic count data is included in **Appendix B**.

Figure 2. Existing Lane Geometries and AM/School/PM Peak Hour Traffic Count Volumes



EXISTING (2025) CONDITIONS ANALYSIS

Existing Intersection Operations

Study intersections were analyzed using methodologies consistent with HCM 7th Edition (*Highway Capacity Manual* - Transportation Research Board, 2022) using the Synchro/SimTraffic Version 12 micro-simulation modeling program. Vehicle delay was calculated using the SimTraffic program and was correlated to LOS thresholds from HCM 7th. Level of Service (LOS) is an indicator of operating conditions on a roadway or at an intersection and is defined in categories ranging from A to F. LOS A indicates free-flowing traffic and LOS F indicates substantial congestion with stop-and-go traffic and long delays at intersections.

For signalized intersections and all-way stop-controlled intersections, the LOS reported is the weighted average control delay of all movements measured in seconds per vehicle. For side-street stop-controlled and roundabout-controlled intersections, the LOS reported is based on the maximum delay of all movements measured in seconds per vehicle.

Existing intersection peak-hour demand volumes, lane configurations, and signal timings are used as inputs in the LOS calculations. The LOS criteria used for this methodology is summarized in **Table 1** for both signalized and unsignalized intersections. Results from an average of approximately ten (10) SimTraffic simulation runs were used to determine the delay and LOS at all intersections.

Table 1: Intersection Level of Service (LOS) Thresholds

Level of Service	Average Delay (seconds / vehicle)		Description
	Signalized	Unsignalized	
A	≤ 10.0	≤ 10.0	Little or no traffic delay
B	> 10.0 and ≤ 20.0	> 10.0 and ≤ 15.0	Minimal traffic delay
C	> 20.0 and ≤ 35.0	> 15.0 and ≤ 25.0	Average traffic delay
D	> 35.0 and ≤ 55.0	> 25.0 and ≤ 35.0	Long traffic delay
E	> 55.0 and ≤ 80.0	> 35.0 and ≤ 50.0	Very long traffic delay
F	> 80.0	> 50.0	Extreme traffic delay

Source: *Highway Capacity Manual*, 7th Edition

The SimTraffic simulation model was calibrated to reflect the field observed queues during the peak hours. Intersection lane geometries, signal timings, roadway speeds, and turning movement volumes including pedestrian and bike data are coded in the SimTraffic model for all study intersections. Global peak hour volumes were used for the SimTraffic model.

Existing conditions were simulated in SimTraffic by giving a seeding time (loading time) of 15 minutes (to account for the queues extending from the previous hours in the peak period) and by providing a recording time of one hour to analyze the hourly operations. The existing counts at the intersections do not reflect the volumes in queue; therefore, the traffic volume was adjusted to replicate the demand volume that corresponds to the queue length observed in the

field during the peak periods. The volume served (flow) is then compared to the counts to validate the intersection throughput.

There is a certain amount of random variability inherent to a microscopic simulation model such as SimTraffic. This is due to the random distribution of vehicle entry time, vehicle type, traffic fleet composition, and driver behavior. This is very similar to the random variability of the day-to-day observations in the field. Therefore, the simulation model is run multiple times with different random number seeds. The calibration procedure then looks at a statistical comparison between the multiple model runs and the field data. For this study, approximately ten (10) runs using different random number seeds were conducted for the AM, School, and PM peak hours.

As part of the analysis, demand volumes were developed to reflect vehicle queueing and to ensure volume balancing across intersections throughout the network. In general, count volumes were increased to capture the vehicles in queues and to reflect the actual demand.

Table 2 shows the adjusted demand volume and served volume comparison for the AM, School, PM peak hours, respectively. The overall served volumes, compared to the counts at the study intersections, typically range between 0 percent and 4 percent. This variation falls within the industry standards of up to 10-15 percent variation for volume calibration.

Detailed SimTraffic outputs for Existing Conditions are summarized in **Appendix C**.

Existing LOS Results

Intersection LOS results are summarized in **Table 3** for the AM and PM peak hours. Based on the LOS results, one (1) intersection in the AM, School, and PM peak hours operates at unacceptable LOS E or F:

AM Peak Hour

Intersection 1 operates at LOS F therefore is unacceptable.

School Peak Hour

Intersection 1 operates at LOS F therefore is unacceptable.

PM Peak Hour

Intersection 1 operates at LOS F therefore is unacceptable.

Existing Queueing Results

The average and 95th percentile queue lengths (measured in feet) were also determined using SimTraffic. The queue lengths are summarized in **Table 4** for the AM, School, and PM peak hours.

In the School and PM peak hours, there is no significant queueing at the study intersections and storage capacity is adequate. For the AM peak hour, queues at the EBR movement at Intersection 1 and the NBL and SBTR movements at Intersection 2 exceed capacity.

Table 2. Existing Volume Summary

#	Intersection	Control	Volume (vph)	Overall	Northbound					Southbound					Eastbound					Westbound				
					U	L	T	R	Total	U	L	T	R	Total	U	L	T	R	Total	U	L	T	R	Total
AM Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Demand	1,538	-	207	396	17	620	-	12	614	28	654	-	16	-	231	247	-	13	1	3	17
			Count	1,538	-	207	396	17	620	-	12	614	28	654	-	16	-	231	247	-	13	1	3	17
			Served	1,515	-	195	423	16	634	-	10	593	30	633	-	15	-	219	234	-	10	1	3	14
			% Served (Demand)	99%	-	94%	107%	94%	102%	-	83%	97%	107%	97%	-	94%	-	95%	95%	-	77%	100%	100%	82%
			% Served (Count)	99%	-	94%	107%	94%	102%	-	83%	97%	107%	97%	-	94%	-	95%	95%	-	77%	100%	100%	82%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,605	-	77	603	10	690	-	-	774	84	858	-	17	-	40	57	-	-	-	-	-
			Served	1,557	-	70	580	11	661	-	-	754	82	836	-	17	-	43	60	-	-	-	-	-
			% Served (Demand)	97%	-	91%	96%	110%	96%	-	-	97%	98%	97%	-	100%	-	108%	105%	-	-	-	-	-
School Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Demand	1,446	-	148	588	22	758	-	6	444	14	477	-	10	1	185	196	-	9	0	6	15
			Count	1,446	-	148	588	22	758	-	19	444	14	477	-	10	1	185	196	-	9	0	6	15
			Served	1,414	-	138	537	16	691	-	4	530	16	550	-	14	1	142	157	-	9	1	6	16
			% Served (Demand)	98%	-	93%	91%	73%	91%	-	67%	119%	114%	115%	-	140%	100%	77%	80%	-	100%	-	100%	107%
			% Served (Count)	98%	-	93%	91%	73%	91%	-	21%	119%	114%	115%	-	140%	100%	77%	80%	-	100%	-	100%	107%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,433	-	7	696	3	706	-	-	573	66	639	-	64	-	24	88	-	-	-	-	-
			Served	1,449	-	28	651	8	687	-	-	642	58	700	-	28	-	34	62	-	-	-	-	-
			% Served (Demand)	101%	-	400%	94%	267%	97%	-	-	112%	88%	110%	-	44%	-	142%	70%	-	-	-	-	-
PM Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Demand	1,445	-	148	588	22	758	-	19	444	14	477	-	10	-	185	195	-	9	-	6	15
			Count	1,445	-	148	588	22	758	-	19	444	14	477	-	10	-	185	195	-	9	-	6	15
			Served	1,465	-	144	600	20	764	-	3	509	15	527	-	9	-	139	148	-	19	-	7	26
			% Served (Demand)	101%	-	97%	102%	91%	101%	-	16%	115%	107%	110%	-	90%	-	75%	76%	-	211%	-	117%	173%
			% Served (Count)	101%	-	97%	102%	91%	101%	-	16%	115%	107%	110%	-	90%	-	75%	76%	-	211%	-	117%	173%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,433	-	7	696	3	706	-	-	573	66	639	-	64	-	24	88	-	-	-	-	-
			Served	1,489	-	18	739	12	769	-	-	628	42	670	-	24	-	26	50	-	-	-	-	-
			% Served (Demand)	104%	-	257%	106%	400%	109%	-	-	110%	64%	105%	-	38%	-	108%	57%	-	-	-	-	-

Note: Results are based on an average of 10 SimTraffic simulation runs.

Table 3. Existing Intersection LOS Summary

#	Intersection	Control	Delay (s) / LOS	SimTraffic Intersection LOS Summary																
				Total	Northbound				Southbound				Eastbound				Westbound			
					U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
AM Peak																				
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Delay	150.4	-	12.9	1.7	0.4	-	21.6	10.9	10.7	-	98.3	-	106.7	-	150.4	34.4	43.1
			LOS	F	-	B	A	A	-	C	B	B	-	F	-	F	-	F	D	E
2	SR 49/ N Washington St & W School St	SSSC	Delay	14.9	-	10.9	14.9	9.2	-	-	7.7	5.4	-	7.8	-	5.7	-	-	-	-
			LOS	B	-	B	B	A	-	-	A	A	-	A	-	A	-	-	-	-
School Peak																				
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Delay	69.0	-	8.1	1.3	0.3	-	4.2	3.0	1.0	-	38.8	29.2	15.6	-	69.0	33.7	17.5
			LOS	F	-	A	A	A	-	A	A	A	-	E	D	C	-	F	D	C
2	SR 49/ N Washington St & W School St	SSSC	Delay	7.2	-	4.7	7.2	4.2	-	-	5.9	3.7	-	6.6	-	5.5	-	-	-	-
			LOS	A	-	A	A	A	-	-	A	A	-	A	-	A	-	-	-	-
PM Peak																				
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Delay	98.4	-	7.3	1.3	0.2	-	7.8	2.8	0.9	-	38.0	-	10.6	-	98.4	-	25.4
			LOS	F	-	A	A	A	-	A	A	A	-	E	-	B	-	F	-	D
2	SR 49/ N Washington St & W School St	SSSC	Delay	7.9	-	4.8	7.9	4.6	-	-	5.5	2.5	-	6.3	-	4.5	-	-	-	-
			LOS	A	-	A	A	A	-	-	A	A	-	A	-	A	-	-	-	-

Note: Results are based on an average of approximately 10 SimTraffic simulation runs. Side-street stop control (SSSC) overall delay looks at worst movement.

Table 4. Existing Intersection Queueing Summary

No.	Intersection	Movement/ Approach	Storage Length	Existing					
				AM Peak		School Peak		PM Peak	
				Avg Queue	95th Queue	Avg Queue	95th Queue	Avg Queue	95th Queue
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	EBLT	1,180	171	472	10	44	5	21
		EBR	130	148	243	53	110	43	88
		WBLTR	780	17	47	13	39	30	67
		NBT	100	58	89	42	73	39	70
		NBL	90	0	12	2	11	0	5
		SBL	50	4	21	5	28	1	10
		SBTR	1,200	50	347	0	0	3	23
2	SR 49/ N Washington St & W School St	EBLTR	700	29	51	29	51	27	51
		NBL	40	26	78	7	26	4	20
		NBTR	400	103	335	38	105	41	127
		SBTR	100	54	120	32	86	15	55

Note: Results based on an average of approximately 10 SimTraffic simulation runs. **Bold** indicates queue exceeds storage capacity.

FORECAST VOLUMES

For this Project, 2040 demand volumes during the AM and PM peak hours for both study intersections, SR 49 & Shaws Flat Road (Intersection 1) and SR 49 & School Street (Intersection 2), were obtained from the *2015 Tuolumne County General Plan and Regional Transportation Plan Update Environmental Impact Report Traffic Study*.

Demand volumes at both study intersections during the School peak hour were obtained by first developing a scaling factor using 2012 AM peak hour counts and 2040 AM peak hour demand volumes at SR 49 & Shaws Flat Road (Intersection 1) from the *2013 Vision Sonora Plan* and the *2015 Tuolumne County General Plan and Regional Transportation Plan Update Environmental Impact Report Traffic Study*, respectively. The scaling factor was then applied to 2012 School peak hour counts at SR 49 & Shaws Flat Road (Intersection 1) to develop 2040 School peak hour demand volumes.

2040 School peak hour demand volumes for SR 49 & School Street (Intersection 2) were then determined by proportionally computing movement volumes using 2012 School peak hour counts at both study intersections from the *2013 Vision Sonora Plan* and the 2040 School peak hour demand volumes at SR 49 & Shaws Flat Road (Intersection 1).

Pedestrian and bicycle volumes were additionally scaled up from 2025 conditions to 2040 conditions using the scaling factor.

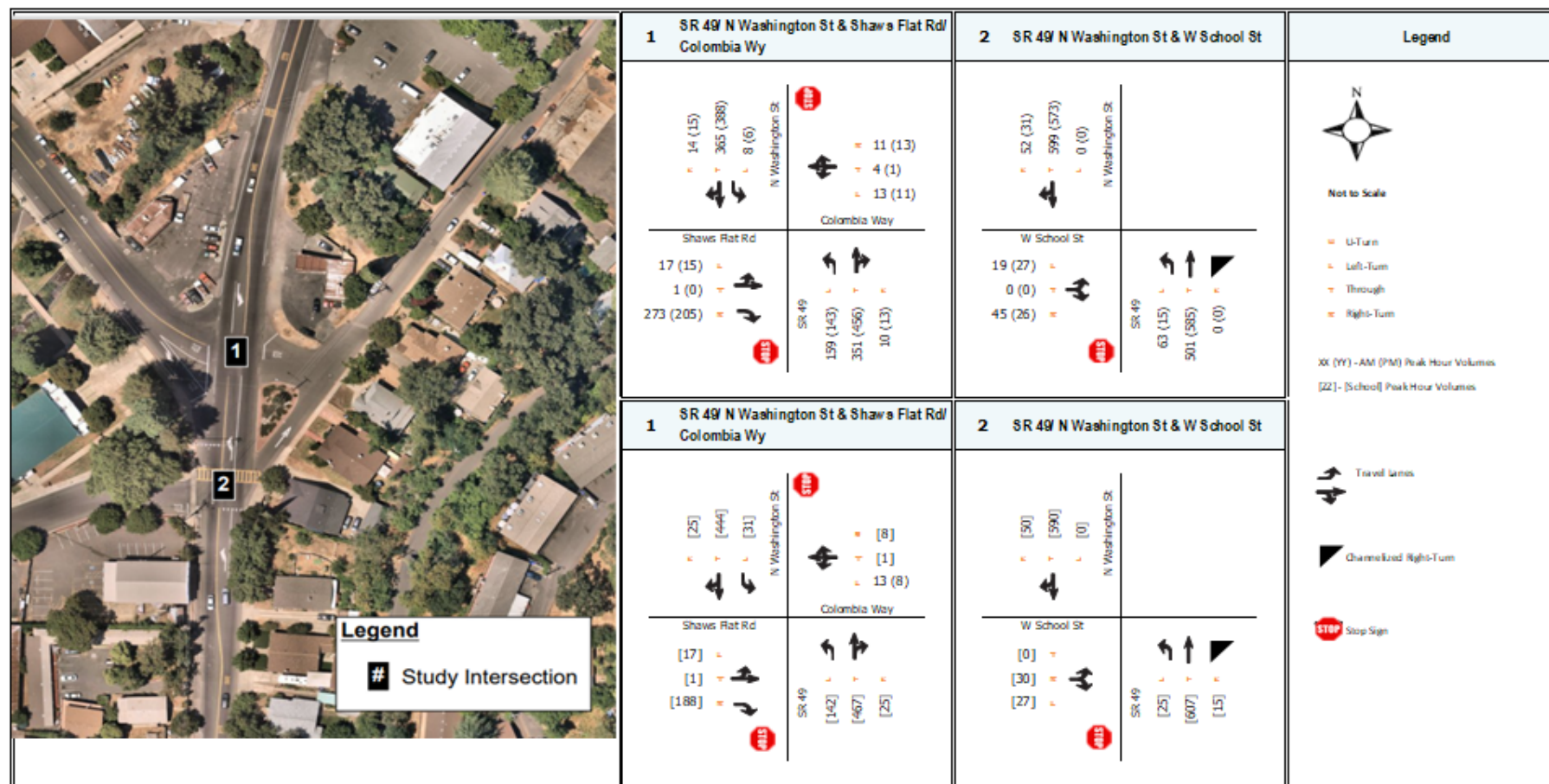
Demand volumes were balanced before input into SimTraffic.

Volume developments are shown in **Appendix D**.

AM, School, and PM peak hour demand volumes for Cumulative (2040) No Build Conditions are shown in **Figure 3**.

Note that the Cumulative 2040 volumes sourced from the *2015 Tuolumne County General Plan and Regional Transportation Plan Update Environmental Impact Report Traffic Study* are inclusive of the traffic effects from construction of the Greenley Road Bypass. To understand the operating conditions of the study intersections under Cumulative 2040 Conditions before construction of the Bypass, a uniform growth rate of 1 percent was applied by movement to Existing 2025 Conditions demand volumes in SIDRA for the 15-year study period from 2025 to 2040.

Figure 3. Cumulative No Build Lane Geometries and AM/School/PM Peak Hour Traffic Demand Volumes



CUMULATIVE (2040) NO BUILD CONDITIONS ANALYSIS

The calibrated Existing Conditions Synchro/SimTraffic model was used as a base model for the Cumulative No Build Conditions. Measures of effectiveness, including volume outputs, delay/LOS, and queue lengths were extracted for the Cumulative No Build Conditions, similar to Existing Conditions.

Table 5 shows the demand volume and served volume comparison for the AM peak hour and PM peak hours. The overall served volumes fall below the overall demand volumes for both study intersections at all peak hours by at least 5 percent, indicating that the existing SSSC would not adequately serve future demand volumes.

Intersection delay and LOS analyses are shown in **Table 6** for the AM, School, and PM peak hours. Intersection 1 operates at unacceptable LOS E or F during all peak hours.

The average and 95th percentile queue lengths are shown in **Table 7**. During the AM peak hour, the 95th percentile EBR queue exceeds the storage capacity at Intersection 1, while the 95th percentile NBL queue exceeds the storage capacity at Intersection 2.

During the School and PM peak hours, there is no significant queueing at the study intersections and storage capacity is adequate.

Table 5. Cumulative No Build Conditions Volume Summary

#	Intersection	Control	Volume (vph)	Overall	Northbound					Southbound					Eastbound					Westbound				
					U	L	T	R	Total	U	L	T	R	Total	U	L	T	R	Total	U	L	T	R	Total
AM Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Demand	1,538	-	207	396	17	620	-	12	614	28	654	-	16	0	231	247	-	13	1	3	17
			Served	1,273	-	165	400	9	574	-	6	363	14	383	-	17	1	269	287	-	12	4	13	29
			% Served (Demand)	83%	-	80%	101%	53%	93%	-	50%	59%	50%	59%	-	106%	-	116%	116%	-	92%	400%	433%	171%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,606	-	84	603	-	687	-	-	774	84	858	-	17	-	44	61	-	-	-	-	-
			Served	1,349	-	64	501	-	565	-	-	597	53	650	-	39	-	95	134	-	-	-	-	-
			% Served (Demand)	84%	-	76%	83%	-	82%	-	-	77%	63%	76%	-	229%	-	216%	220%	-	-	-	-	-
School Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Demand	1,446	-	148	588	22	758	-	19	444	14	477	-	10	1	185	196	-	9	0	6	15
			Served	1,330	-	138	459	26	623	-	34	432	24	490	-	17	2	180	199	-	7	1	10	18
			% Served (Demand)	92%	-	93%	78%	118%	82%	-	179%	97%	171%	103%	-	170%	200%	97%	102%	-	78%	-	167%	120%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,433	-	7	696	3	706	-	-	573	66	639	-	64	-	24	88	-	-	-	-	-
			Served	1,316	-	25	597	16	638	-	-	570	53	623	-	26	-	29	55	-	-	-	-	-
			% Served (Demand)	92%	-	357%	86%	533%	90%	-	-	99%	80%	97%	-	41%	-	121%	63%	-	-	-	-	-
PM Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Demand	1,445	-	148	588	22	758	-	19	444	14	477	-	10	-	185	195	-	9	0	6	15
			Served	1,258	-	143	459	14	616	-	5	376	16	397	-	14	-	207	221	-	11	1	12	24
			% Served (Demand)	87%	-	97%	78%	64%	81%	-	26%	85%	114%	83%	-	140%	-	112%	113%	-	122%	-	200%	160%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,430	-	7	696	-	703	-	-	573	66	639	-	64	-	24	88	-	-	-	-	-
			Served	1,255	-	16	588	-	604	-	-	566	34	600	-	26	-	25	51	-	-	-	-	-
			% Served (Demand)	88%	-	229%	84%	-	86%	-	-	99%	52%	94%	-	41%	-	104%	58%	-	-	-	-	-

Note: Results are based on an average of approximately 10 SimTraffic simulation runs.

Table 6. Cumulative No Build Conditions Intersection LOS Summary

#	Intersection	Control	Delay (s) / LOS	SimTraffic Intersection LOS Summary																	
				Total	Northbound				Southbound				Eastbound				Westbound				
					U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
AM Peak																					
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Delay	53.7	-	5.1	1.4	0.3	-	3.1	2.6	0.6	-	21.8	21.3	18.3	-	53.7	15.3	8.6	
			LOS	F	-	A	A	A	-	A	A	A	-	C	C	C	-	F	C	A	
2	SR 49/ N Washington St & W School St	SSSC	Delay	12.3	-	5.3	7.7	-	-	-	7.2	4.4	-	12.3	-	8.4	-	-	-	-	
			LOS	B	-	A	A	-	-	-	A	A	-	B	-	A	-	-	-	-	
School Peak																					
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Delay	48.0	-	6.1	1.3	0.3	-	4.5	2.9	1.2	-	27.2	19.4	13.2	-	48.0	16.9	10.7	
			LOS	E	-	A	A	A	-	A	A	A	-	D	C	B	-	E	C	B	
2	SR 49/ N Washington St & W School St	SSSC	Delay	7.2	-	5.1	7.0	3.2	-	-	6.2	3.3	-	7.2	-	5.4	-	-	-	-	
			LOS	A	-	A	A	A	-	-	A	A	-	A	-	A	-	-	-	-	
PM Peak																					
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	SSSC	Delay	44.4	-	4.7	1.2	0.2	-	4.4	2.5	0.7	-	20.5	-	11.5	-	44.4	11.2	8.9	
			LOS	E	-	A	A	A	-	A	A	A	-	C	-	B	-	E	B	A	
2	SR 49/ N Washington St & W School St	SSSC	Delay	6.6	-	4.3	6.6	-	-	-	5.8	3.3	-	6.6	-	4.5	-	-	-	-	
			LOS	A	-	A	A	-	-	-	A	A	-	A	-	A	-	-	-	-	

Note: Results are based on an average of approximately 10 SimTraffic simulation runs. The delay for the worst performing movement (or shared movement) on the side street approaches or the left-turn movements on the major street approaches are reported as overall intersection delay for side-street stop control (SSSC).

Table 7. Cumulative No Build Conditions Intersection Queueing Summary

No.	Intersection	Movement/ Approach	Storage Length	Cumulative No Build					
				AM Peak		School Peak		PM Peak	
				Avg Queue	95th Queue	Avg Queue	95th Queue	Avg Queue	95th Queue
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	EBLT	1,180	20	110	13	55	8	26
		EBR	130	84	161	57	117	60	117
		WBLTR	780	26	60	16	45	19	50
		NBT	100	42	75	39	70	35	65
		NBL	90	0	5	11	34	2	13
		SBL	50	2	14	7	36	5	30
		SBTR	1,200	5	32	0	0	0	0
2	SR 49/ N Washington St & W School St	EBLTR	700	90	155	28	53	27	53
		NBL	40	19	54	7	26	4	22
		NBTR	400	45	115	35	93	27	87
		SBTR	100	41	93	26	79	15	55

Note: Results based on an average of approximately 10 SimTraffic simulation runs. **Bold** indicates queue exceeds storage capacity.

INTERSECTION CONTROL ALTERNATIVES

The existing controls at the study intersections are Side-Street Stop Controls (SSSC). Under Cumulative (2045) Conditions, the following three (3) intersection control alternatives were evaluated at the study intersections:

- Alternative 1 – Signal (SIG)
- Alternative 2 – 4-Legged Roundabout (4LRAB)
- Alternative 3 – 5-Legged Roundabout (5LRAB)

Each intersection alternative is shown in **Figure 4** through **Figure 6** below.

Figure 4. Alternative 1 – Signal (SIG)



Figure 5. Alternative 2 – 4-Legged Roundabout (4LRAB)



Figure 6. Alternative 3 – 5-Legged Roundabout (5LRAB)



CUMULATIVE (2040) BUILD WITH BYPASS CONDITIONS ANALYSIS

The calibrated Cumulative (2040) No Build Conditions Synchro/SimTraffic model was used as a base model. The model was updated to reflect lane geometry changes proposed under Cumulative Build Conditions.

From Cumulative No Build Conditions to Cumulative Build Conditions, traffic patterns are not expected to change. Therefore, traffic demand volumes from the calibrated Cumulative No Build Conditions were used as the Cumulative Build Conditions demand volumes.

Measures of effectiveness, including volume outputs, delay/LOS, and queue lengths were extracted for the Cumulative Build Conditions for each intersection control alternative.

Alternative 1 – Signalized (SIG)

Under Alternative 1, Intersection 1 is signalized while Intersection 2 remains SSSC. There are no geometric modifications at the study intersections.

Table 8 shows the demand volume and served volume comparison for the AM, School, and PM peak hours. The overall served volumes fall below the overall demand volumes for both study intersections at all peak hours by at least 5 percent, indicating that signalized control at Intersection 1 would not adequately serve future demand volumes.

Intersection delay and LOS are shown in **Table 9** for AM, School, and PM peak hours. All approaches operate at an acceptable LOS D or better during all peak hours.

The average and 95th percentile queue lengths are shown in **Table 10**. During the AM peak hour, the 95th percentile NBL queue at Intersection 2 exceeds the storage capacity. During the School peak hour, the 95th percentile SBL queue at Intersection 1 and the 95th percentile NBL and SBTR queues at Intersection 2 exceed their respective storage lengths. No queues during the PM peak hour exceed their respective storage lengths.

Table 8. Cumulative Build Conditions Volume Summary – Signalized

#	Intersection	Control	Volume (vph)	Overall	Northbound					Southbound					Eastbound					Westbound				
					U	L	T	R	Total	U	L	T	R	Total	U	L	T	R	Total	U	L	T	R	Total
AM Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	Signal	Demand	1,538	-	207	396	17	620	-	12	614	28	654	-	16	0	231	247	-	13	1	3	17
			Count	1,538	-	207	396	17	620	-	12	614	28	654	-	16	0	231	247	-	13	1	3	17
			Served	1,213	-	160	346	9	515	-	6	364	14	384	-	14	1	270	285	-	12	4	13	29
			% Served (Demand)	79%	-	77%	87%	53%	83%	-	50%	59%	50%	59%	-	88%	-	117%	115%	-	92%	400%	433%	171%
			% Served (Count)	79%	-	77%	87%	53%	83%	-	50%	59%	50%	59%	-	88%	-	117%	115%	-	92%	400%	433%	171%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,606	-	84	603	-	687	-	-	774	84	858	-	17	-	44	61	-	-	-	-	-
			Served	1,271	-	62	498	-	560	-	-	600	52	652	-	16	-	43	59	-	-	-	-	-
			% Served (Demand)	79%	-	74%	83%	-	82%	-	-	78%	62%	76%	-	94%	-	98%	97%	-	-	-	-	-
School Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	Signal	Demand	1,446	-	148	588	22	758	-	19	444	14	477	-	10	1	185	196	-	9	0	6	15
			Count	1,446	-	148	588	22	758	-	19	444	14	477	-	10	1	185	196	-	9	0	6	15
			Served	1,365	-	142	460	43	645	-	29	443	25	497	-	18	1	187	206	-	7	1	9	17
			% Served (Demand)	94%	-	96%	78%	195%	85%	-	153%	100%	179%	104%	-	180%	100%	101%	105%	-	78%	-	150%	113%
			% Served (Count)	94%	-	96%	78%	195%	85%	-	153%	100%	179%	104%	-	180%	100%	101%	105%	-	78%	-	150%	113%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,430	-	7	696	-	703	-	-	573	66	639	-	64	-	24	88	-	-	-	-	-
			Served	1,343	-	24	622	-	646	-	-	593	49	642	-	23	-	32	55	-	-	-	-	-
			% Served (Demand)	94%	-	343%	89%	-	92%	-	-	103%	74%	100%	-	36%	-	133%	63%	-	-	-	-	-
PM Peak Hour																								
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	Signal	Demand	1,445	-	148	588	22	758	-	19	444	14	477	-	10	-	185	195	-	9	0	6	15
			Count	1,445	-	148	588	22	758	-	19	444	14	477	-	10	-	185	195	-	9	0	6	15
			Served	1,282	-	143	464	12	619	-	5	394	16	415	-	15	-	206	221	-	12	1	14	27
			% Served (Demand)	89%	-	97%	79%	55%	82%	-	26%	89%	114%	87%	-	150%	-	111%	113%	-	133%	-	233%	180%
			% Served (Count)	89%	-	97%	79%	55%	82%	-	26%	89%	114%	87%	-	150%	-	111%	113%	-	133%	-	233%	180%
2	SR 49/ N Washington St & W School St	SSSC	Demand	1,430	-	7	696	-	703	-	-	573	66	639	-	64	-	24	88	-	-	-	-	-
			Served	1,277	-	17	592	-	609	-	-	582	35	617	-	26	-	25	51	-	-	-	-	-
			% Served (Demand)	89%	-	243%	85%	-	87%	-	-	102%	53%	97%	-	41%	-	104%	58%	-	-	-	-	-

Note: Results are based on an average of approximately 10 SimTraffic simulation runs.

Table 9. Cumulative Build Conditions Intersection LOS Summary – Signalized

#	Intersection	Control	Delay (s) / LOS	SimTraffic Intersection LOS Summary																
				Total	Northbound				Southbound				Eastbound				Westbound			
					U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
AM Peak																				
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	Signal	Delay	10.5	-	16.7	5.3	2.7	-	27.2	15.4	10.5	-	12.4	13.1	6.8	-	12.9	13.7	3.7
			LOS	B	-	B	A	A	-	C	B	B	-	B	B	A	-	B	B	A
2	SR 49/ N Washington St & W School St	SSSC	Delay	7.1	-	5.6	9.9	-	-	-	5.4	2.5	-	7.9	-	4.8	-	-	-	-
			LOS	A	-	A	A	-	-	-	A	A	-	A	-	A	-	-	-	-
School Peak																				
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	Signal	Delay	10.6	-	20.4	5.1	2.6	-	28.6	14.0	7.9	-	19.5	12.8	7.0	-	15.6	9.7	4.6
			LOS	B	-	C	A	A	-	C	B	A	-	B	B	A	-	B	A	A
2	SR 49/ N Washington St & W School St	SSSC	Delay	11.1	-	12.1	16.9	-	-	-	6.0	3.5	-	10.1	-	5.9	-	-	-	-
			LOS	B	-	B	B	-	-	-	A	A	-	B	-	A	-	-	-	-
PM Peak																				
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	Signal	Delay	9.5	-	17.2	4.7	3.0	-	27.2	14.2	7.8	-	13.8	-	5.7	-	14.8	10.3	4.9
			LOS	A	-	B	A	A	-	C	B	A	-	B	-	A	-	B	B	A
2	SR 49/ N Washington St & W School St	SSSC	Delay	8.2	-	5.9	11.6	-	-	-	5.3	2.3	-	7.5	-	4.2	-	-	-	-
			LOS	A	-	A	B	-	-	-	A	A	-	A	-	A	-	-	-	-

Note: Results are based on an average of approximately 10 SimTraffic simulation runs.

Table 10. Cumulative Build Conditions Intersection Queueing Summary – Signalized

No.	Intersection	Movement/ Approach	Storage Length	Cumulative No Build					
				AM Peak		School Peak		PM Peak	
				Avg Queue	95th Queue	Avg Queue	95th Queue	Avg Queue	95th Queue
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	EBLT	1,180	7	35	8	26	6	21
		EBR	130	56	106	44	88	43	87
		WBLTR	780	16	46	11	35	16	46
		NBT	100	58	84	57	84	55	83
		NBL	90	54	87	61	87	60	86
		SBL	50	6	31	23	62	5	29
		SBTR	1,200	106	187	117	210	103	191
2	SR 49/ N Washington St & W School St	EBLTR	700	29	55	29	58	27	50
		NBL	40	19	58	15	68	5	29
		NBTR	400	67	179	133	342	92	235
		SBTR	100	26	80	37	105	24	79

Note: Results based on an average of approximately 10 SimTraffic simulation runs. **Bold** indicates queue exceeds storage capacity.

Alternative 2 – 4-Legged Roundabout (4LRAB)

Under Alternative 2, Intersection 1 is reconstructed as a 1-lane roundabout while Intersection 2 retains its existing geometry and SSSC. Intersection 1 was analyzed using SIDRA Intersection 9.1 software with the following options selected:

- HCM 6 Methodology
- Entry/Circ Flow Adjustment set to “None”
- Environment Factor set to 1.05

Note that HCM 7 is not available in SIDRA. However, HCM 7 did not update the roundabout methodology from HCM 6.

Intersection 2 was analyzed using Synchro/SimTraffic Version 12.

Delay, LOS, and queue lengths for Intersection 1 are shown in **Table 11**. Delay and LOS for Intersection 2 are shown in **Table 12**. Queue lengths for Intersection 2 are shown in **Table 13**.

All approaches operate at an acceptable LOS D or better during all peak hours. The 95th percentile NBL queue at Intersection 2 exceeds the storage length during all peak hours.

Table 11. Cumulative Build Conditions Intersection LOS & Queueing Summary – 4LRAB – Intersection 1

#	Intersection	Control	Approach	Storage Length	AM Peak Hour						School Peak Hour						PM Peak Hour					
					Demand Volume (vph)	v/c	Avg. Delay (sec)	LOS	95th % Queue		Demand Volume (vph)	v/c	Avg. Delay (sec)	LOS	95th % Queue		Demand Volume (vph)	v/c	Avg. Delay (sec)	LOS	95th % Queue	
									Vehicles	Length (ft)					Vehicles	Length (ft)					Vehicles	Length (ft)
1	SR 49 & Shaws Flat Road	4LRAB	South Leg (NB)	100	583	0.506	7.4	A	4.7	119.3	659	0.587	9.2	A	5.8	148.6	614	0.541	7.7	A	5.3	135.9
			East Leg (WB)	780	28	0.042	5.4	A	0.2	5.7	17	0.028	5.8	A	0.2	4.0	25	0.039	5.6	A	0.2	5.3
			North Leg (SB)	1290	387	0.416	8.1	A	2.8	70.6	500	0.510	9.2	A	3.9	100.7	409	0.412	7.6	A	2.8	71.1
			West Leg (EB)	1180	291	0.392	9.2	A	2.6	66.3	206	0.300	8.3	A	1.9	48.5	220	0.289	7.5	A	1.8	45.5
			Overall		1,289	0.506	9.2	A			1,382	0.587	9.2	A			1,268	0.541	7.7	A		

Note: Results are based on HCM 6 methodology in SIDRA Intersection 9.1 Software. Overall results report the worst movement. **Bold** indicates queue exceeds storage capacity.

Table 12. Cumulative Build Conditions Intersection LOS Summary – SSSC – Intersection 2

#	Intersection	Control	Delay (s) / LOS	SimTraffic Intersection LOS Summary									
				Total	Northbound			Southbound			Eastbound		
					L	T	R	L	T	R	L	T	R
AM Peak													
2	SR 49/ N Washington St & W School St	SSSC	Delay	3.9	-	6.1	-	-	2.2	1.4	5.8	-	4.3
			LOS	A	-	A	-	-	A	A	A	-	A
School Peak													
2	SR 49/ N Washington St & W School St	SSSC	Delay	4.3	-	6.6	-	-	2.1	1.3	5.9	-	4.4
			LOS	A	-	A	-	-	A	A	A	-	A
PM Peak													
2	SR 49/ N Washington St & W School St	SSSC	Delay	4.2	-	6.5	-	-	2.0	1.2	6.1	-	4.3
			LOS	A	-	A	-	-	A	A	A	-	A

Note: Results are based on an average of approximately 10 SimTraffic simulation runs.

Table 13. Cumulative Build Conditions Intersection Queueing Summary – SSSC – Intersection 2

#	Intersection	Movement/ Lane Group	Storage Capacity (feet)	Queue Summary					
				AM Peak		School Peak		PM Peak	
				Average	95th Percentile	Average	95th Percentile	Average	95th Percentile
2	SR 49/ N Washington St & W School St	EBTR	700	29	53	28	54	27	55
		NBL	40	22	68	29	84	34	92
		NBTR	404	27	75	24	68	23	68
		SBTR	95	0	0	0	0	0	0

Note: Results are based on an average of approximately 10 SimTraffic simulation runs.

Alternative 3 – 5-Legged Roundabout (5LRAB)

Under Alternative 3, both study intersections would be reconstructed and combined into a 1-lane roundabout. The single roundabout intersection was analyzed using SIDRA Intersection 9.1 software with the following options selected:

- HCM 6 Methodology
- Entry/Circ Flow Adjustment set to “None”
- Environment Factor set to 1.05

Note that HCM 7 is not available in SIDRA. However, HCM 7 did not update the roundabout methodology from HCM 6.

Intersection delay and LOS, v/c, and 95th percentile queue lengths are shown in **Table 14** for AM, School, and PM peak hours. All approaches operate at an acceptable LOS D or better during all peak hours. Additionally, there are no queueing deficiencies for any peak hour.

Table 14. Cumulative Build Conditions Intersection LOS & Queueing Summary – 5LRAB

#	Intersection	Scenario	Approach	Storage Length	AM Peak Hour						School Peak Hour						PM Peak Hour					
					Demand Volume (vph)	v/c	Avg. Delay (sec)	LOS	95th % Queue		Demand Volume (vph)	v/c	Avg. Delay (sec)	LOS	95th % Queue		Demand Volume (vph)	v/c	Avg. Delay (sec)	LOS	95th % Queue	
									Vehicles	Length (ft)					Vehicles	Length (ft)					Vehicles	Length (ft)
1	SR 49 & Shaws Flat Road	2040 RAB	South Leg (NB)	400	554	0.486	7.4	A	4.1	105.7	607	0.538	8.3	A	5.0	128.4	587	0.519	8.0	A	4.7	120.1
			Northeast Leg (SWB)	780	28	0.042	5.4	A	0.2	5.7	17	0.028	5.7	A	0.2	3.9	25	0.039	5.7	A	0.2	5.5
			North Leg (SB)	1290	387	0.417	8.1	A	2.8	71.0	500	0.509	9.2	A	3.9	99.4	409	0.413	7.6	A	2.8	72.3
			Northwest Leg (SEB)	1180	291	0.364	8.8	A	2.4	60.4	206	0.300	8.3	A	1.9	48.4	220	0.288	7.5	A	1.8	45.4
			Southwest Leg (NEB)	1060	64	0.101	6.4	A	0.6	14.5	57	0.093	6.5	A	0.5	13.6	53	0.081	5.9	A	0.4	11.4
			Overall		1,324	0.486	8.8	A			1,387	0.538	9.2	A			1,294	0.519	8.0	A		

Note: Results are based on HCM 6 methodology in SIDRA Intersection 9.1 Software. Overall results report the worst movement. **Bold** indicates queue exceeds storage capacity.

CUMULATIVE (2040) BUILD WITHOUT BYPASS CONDITIONS ANALYSIS

The calibrated Cumulative (2040) No Build Conditions Synchro/SimTraffic model was used as a base model for the Cumulative (2040) Build without Bypass Conditions analysis.

Volumes were updated as discussed in the Forecasted Volumes section. Under Alternative 2 (4LRAB), Intersection 1 was modeled using SIDRA 9.1 while Intersection 2 was modeled using Synchro/SimTraffic 12. Under Alternative 3 (5LRAB), the single roundabout was modeled using SIDRA 9.1.

The analysis results are summarized in **Table 15**.

Overall intersection delays were slightly compared to those from Cumulative (2040) Build with Bypass Conditions. This outcome is expected as the volumes without the Bypass are higher and thus would result in increased delays.

Table 15. Cumulative Build with Bypass Conditions Intersection LOS & Queueing Summary – 4LRAB & 5LRAB

No.	Intersection	Peak Hour	Cumulative w/out Bypass							
			4-Legged Roundabout				5-Legged Roundabout			
			Control	v/c	Avg. Delay	LOS	Control	v/c	Avg. Delay	LOS
1	SR 49 & Shaws Flat Rd	AM Peak Hour	RAB	0.75	13.2	B	RAB	0.749	13.2	B
		School Peak Hour		0.62	9.2	A		0.601	9.5	A
		PM Peak Hour		0.69	9.3	A		0.661	9.7	A
2	SR 49 & School Street	AM Peak Hour	SSSC (RIRO)	-	4.6	A	Intersections merged for this alternative			
		School Peak Hour		-	4.6	A				
		PM Peak Hour		-	5.0	A				
Total		A-D	6				3			
		E-F	0				0			

Note: Results are based on HCM 6 methodology in SIDRA Intersection 9.1 Software and HCM 7 methodology in Synchro/SimTraffic 12.

INTERSECTION SAFETY ASSESSMENT

Five years of crash data (dating from 10/1/2019 - 9/30/2024) for SR 49 & Shaws Flat Road (Intersection 1) were reviewed. The crash data were provided by Caltrans from the Traffic Accident Surveillance and Analysis System (TASAS). No specific crash data were received for SR 49 & School Street (Intersection 2). Table 16 presents a comparison between the crash rate at Intersection 1 and the statewide average crash rate at similar facilities.

Table 16. Existing Crash Data Summary

Intersection	Total number of crashes	Actual (per million vehicle miles)			Statewide Average (per million vehicle miles)		
		Fatal Crashes	Fatal + Injury Crashes	Total*	Fatal Crashes	Fatal + Injury Crashes	Total*
(#1) SR 49 & Shaws Flat Rd	5	0.00	0.11	0.14	0.006	0.21	0.47

Source: Caltrans Traffic Accident Surveillance and Analysis System.

* Total crash rate includes fatal, injury, and property damage crashes.

Intersection 1 experienced a total of five crashes over a five-year study period, comprising of four injury-related crashes and no fatalities. As illustrated in Table 16, the crash rate at this study intersection is lower than the statewide average for similar facilities. This indicates that the intersection generally has lesser safety risk compared to other intersections sharing similar roadway characteristics.

Alternative 1 proposes implementing a signal at Intersection 1. The Caltrans Local Roadway Safety Manual (LRSM) recognizes installing signals as a countermeasure with a crash reduction factor (CRF) of 30%.

Alternative 2 proposes implementing a 4LRAB at Intersection 1. Converting a SSSC intersection into a single-lane roundabout in a suburban setting yields a crash reduction factor of 78%, as outlined by NCHRP Report 1043.

Alternative 3 proposes implementing a 5LRAB that would combine both study intersections. Similar to Alternative 2, converting the two study intersections into a single-lane roundabout would result in a crash reduction of 78% according to NCHRP Report 1043.

The CRF's associated with each countermeasure were separately applied to the existing crash rate to develop the modified crash rates shown in **Table 17**.

Table 17. Crash Rates with Proposed Countermeasures

Intersection	Intersection Travel Crash Rate (per million vehicle miles)		
	Existing (Caltrans)	Reduction with Signal (Alternative 1)	Reduction with 4-Leg (Alternative 2) or 5-Leg (Alternative 3) RAB
SR 49 & Shaws Flat Road	0.14	0.10	0.03

The implementations of either a traffic signal or a single-lane roundabout at Intersection 1, as proposed in Alternative 1 and Alternative 2, respectively, or a five-legged roundabout at both study intersections as proposed in Alternative 3, are anticipated to reduce the actual crash rate below the statewide average.

SUMMARY

As part of the Sonora ISOAP Stage 1 study, an analysis of different intersection control alternatives at the site was conducted. A comparison of the LOS and delay between No Build and each Build alternative is shown in **Table 18**. A queueing comparison is shown in **Table 19**, **Table 20**, and **Table 21** for the AM, School, and PM peak hours, respectively.

Based on the operational analysis results, the following can be concluded:

- Under Existing (2025) Conditions, Intersection 1 operates at unacceptable LOS F during all peak hours and with queueing deficiencies in the AM peak period. Intersection 2 operates at an acceptable LOS D or better during all peak hours and has queueing deficiencies in the AM period.
- Under Cumulative (2040) No Build Conditions, Intersection 1 operates at unacceptable LOS E or worse during all peak hours and with queueing deficiencies in the AM peak period. Intersection 2 operates at an acceptable LOS D or better during all peak hours and has queueing deficiencies in the AM period.
- Under Cumulative (2040) Build with Bypass Signalized Conditions (Alternative 1), all study intersections operate at acceptable LOS and delay during all peak hours. During the AM peak hour, the 95th percentile NBL queue at Intersection 2 exceeds the storage capacity. During the School peak hour, the 95th percentile SBL queue at Intersection 1 and the 95th percentile NBL and SBTR queues at Intersection 2 exceed their respective storage lengths. No queues during the PM peak hour exceed their respective storage lengths. The overall served volumes fall below the overall demand volumes for both study intersections at all peak hours by at least 5 percent, indicating that signalized control at Intersection 1 would not adequately serve future demand volumes.
- Under Cumulative (2040) Build with Bypass 4LRAB Conditions (Alternative 2), all study intersections operate at acceptable LOS and delay during all peak hours. The 95th percentile NBL queue at Intersection 2 exceeds the storage length during all peak hours.
- Under Cumulative (2040) Build with Bypass 5LRAB Conditions (Alternative 3), all approaches operate at acceptable LOS and delay and all queues are contained within their respective storage capacities during all peak hours.
- Under Cumulative (2040) Build without Bypass 4LRAB or 5LRAB Conditions, all approaches operate at acceptable LOS and delay. Delays increase relative to Cumulative (2040) with Bypass Build Conditions due to higher expected volumes.
- Existing crash rates would be expected to decrease with the proposed countermeasures.

Based on the findings of this analysis, the 5-legged Roundabout (Alternative 3) is recommended for further study under Stage 2 of the ISOAP.

Table 18. Intersection LOS Comparison

No.	Intersection	Peak Hour	Existing			Cumulative w/o Bypass								Cumulative w/ Bypass													
						4-Legged Roundabout				5-Legged Roundabout				No Build			Signal			4-Legged Roundabout				5-Legged Roundabout			
			Control	Avg. Delay	LOS	Control	v/c	Avg. Delay	LOS	Control	v/c	Avg. Delay	LOS	Control	Avg. Delay	LOS	Control	Avg. Delay	LOS	Control	v/c	Avg. Delay	LOS	Control	v/c	Avg. Delay	LOS
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	AM Peak Hour	SSSC	150.4	F	RAB	0.75	13.2	B	RAB	0.749	13.2	B	SSSC	53.7	F	Signal	10.5	B	RAB	0.51	9.2	A	RAB	0.486	8.8	A
		School Peak Hour		69.0	F		0.62	9.2	A		0.601	9.5	A		48.0	E		10.6	B		0.59	9.2	A		0.538	9.2	A
		PM Peak Hour		98.4	F		0.69	9.3	A		0.661	9.7	A		44.4	E		9.5	A		0.54	7.7	A		0.519	8.0	A
2	SR 49/ N Washington St & W School St	AM Peak Hour	SSSC	14.9	B	SSSC (RIRO)	-	4.6	A	Intersections merged for this alternative	SSSC	12.3	B	SSSC	7.1	A	SSSC	-	3.9	A	Intersections merged for this alternative						
		School Peak Hour		7.2	A		-	4.6	A			7.2	A		11.1	B		-	4.3	A							
		PM Peak Hour		7.9	A		-	5.0	A			6.6	A		8.2	A		-	4.2	A							
Total		A-D	3			6				3				3			6			6				3			
		E-F	3			0				0				3			0			0				0			

Note: No Build, SSSC, and Signal results based on an average of approximately 10 SimTraffic simulation runs. RAB results based on SIDRA Intersection 9.1 Software. Overall intersection delay and LOS for SSSC and RAB are those of the worst movements.

Table 19. Intersection AM Peak Hour Queueing Comparison

No.	Intersection	Movement/ Approach	Storage Length	Existing		Cumulative with Bypass									
						No Build		Signalized		4-Legged Roundabout			5-Legged Roundabout		
				Avg Queue	95th Queue	Avg Queue	95th Queue	Avg Queue	95th Queue	Approach	Storage Length	95th Queue	Approach	Storage Length	95th Queue
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	EBLT	1,180	171	472	20	110	7	35	West Leg	1,180	66	Northwest Leg	1,180	60
		EBR	130	148	243	84	161	56	106						
		WBLTR	780	17	47	26	60	16	46	East Leg	780	6	Northeast Leg	780	6
		NBT	100	58	89	42	75	58	84	South Leg	100	119			
		NBL	90	0	12	0	5	54	87						
		SBL	50	4	21	2	14	6	31	North Leg	1,290	71	North Leg	1,290	71
		SBTR	1,200	50	347	5	32	106	187						
2	SR 49/ N Washington St & W School St	EBLTR	700	29	51	90	155	29	55	West Leg	500	57	Southwest Leg	1,060	15
		NBL	40	26	78	19	54	19	58	South Leg	400	2	South Leg	400	106
		NBTR	400	103	335	45	115	67	179						
		SBTR	100	54	120	41	93	26	80	North Leg	100	0			

Note: Bold indicates queue exceeds storage capacity.

Table 20. Intersection School Peak Hour Queueing Comparison

No.	Intersection	Movement/ Approach	Storage Length	Existing		Cumulative with Bypass									
						SSSC		Signalized		4-Legged Roundabout			5-Legged Roundabout		
				Avg Queue	95th Queue	Avg Queue	95th Queue	Avg Queue	95th Queue	Approach	Storage Length	95th Queue	Approach	Storage Length	95th Queue
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	EBLT	1,180	10	44	13	55	8	26	West Leg	1,180	49	Northwest Leg	1,180	48
		EBR	130	53	110	57	117	44	88						
		WBLTR	780	13	39	16	45	11	35	East Leg	780	4	Northeast Leg	780	4
		NBT	100	42	73	39	70	57	84	South Leg	100	149			
		NBL	90	2	11	11	34	61	87						
		SBL	50	5	28	7	36	23	62	North Leg	1,290	101	North Leg	1,290	99
		SBTR	1,200	0	0	0	0	117	210						
2	SR 49/ N Washington St & W School St	EBLTR	700	29	51	28	53	29	58	West Leg	500	78	Southwest Leg	1,060	14
		NBL	40	7	26	7	26	15	68	South Leg	400	2	South Leg	400	128
		NBTR	400	38	105	35	93	133	342						
		SBTR	100	32	86	26	79	37	105	North Leg	100	0			

Note: **Bold** indicates queue exceeds storage capacity.

Table 21. Intersection PM Peak Hour Queueing Comparison

No.	Intersection	Movement/ Approach	Storage Length	Existing		Cumulative with Bypass									
						SSSC		Signalized		4-Legged Roundabout			5-Legged Roundabout		
				Avg Queue	95th Queue	Avg Queue	95th Queue	Avg Queue	95th Queue	Approach	Storage Length	95th Queue	Approach	Storage Length	95th Queue
1	SR 49/ N Washington St & Shaws Flat Rd/ Colombia Wy	EBLT	1,180	5	21	8	26	6	21	West Leg	1,180	46	Northwest Leg	1,180	45
		EBR	130	43	88	60	117	43	87						
		WBLTR	780	30	67	19	50	16	46	East Leg	780	5	Northeast Leg	780	6
		NBT	100	39	70	35	65	55	83	South Leg	100	136			
		NBL	90	0	5	2	13	60	86						
		SBL	50	1	10	5	30	5	29	North Leg	1,290	71	North Leg	1,290	72
		SBTR	1,200	3	23	0	0	103	191						
2	SR 49/ N Washington St & W School St	EBLTR	700	27	51	27	53	27	50	West Leg	500	58	Southwest Leg	1,060	11
		NBL	40	4	20	4	22	5	29	South Leg	400	2	South Leg	400	120
		NBTR	400	41	127	27	87	92	235						
		SBTR	100	15	55	15	55	24	79	North Leg	100	0			

Note: **Bold** indicates queue exceeds storage capacity.

APPENDIX

Appendix A. TCTC Greenley Road Bypass PID Approval Informational Item



Peter Rei
Chairman

Darin Grossi
Executive Director

TUOLUMNE COUNTY TRANSPORTATION COUNCIL

Date: August 16, 2022

To: Tuolumne County Board of Supervisors

From: Darin Grossi, Executive Director, Tuolumne County Transportation Council
Kim MacFarlane, Director of Tuolumne County Public Works

Subject: Consideration of approving the Public Works Department to collaborate with the Tuolumne County Transportation Council and City of Sonora on the development of a Project Initiating Document for Phase I of the North/South Connector (Greenley Road Extension)

Introduction:

At a joint meeting of the Board of Supervisors and Sonora City Council on December 17, 2021, the Greenley Road Extension Project was discussed as a project of mutual interest. Resolution 127-21 was passed resolving to maintain open communication on issues effecting citizens with a collective mission of creating safe and healthy communities that promote economic prosperity and enhance quality of life for both citizens and visitors. The purpose of this agenda item is to request authorization for County staff to collaborate with the Tuolumne County Transportation Council and City of Sonora staff on the development of a Project Initiation Documents for Phase I of the North/South Connector project Greenley Road Extension. Additionally, the approval to use \$125,000 in Regional Traffic Impact Mitigation Funds is requested.

Background:

Greenley Road Extension/North-South Connector: The Greenley Road Extension, in one form or another, has been included in every Tuolumne County Regional Transportation Plan (RTP) since the first RTP in 1975. In the early 1980's, the Greenley Road Extension became a part of the Federal Aid Secondary program with both Federal and State funding, and proceeded through preliminary design, State and Federal environmental assessment, alternate route studies to 1987, when a Finding of No Significant Impact was adopted by the Federal Highway Administration (FHA) (February 1987). After numerous hearings and protests from property owners, a route was adopted in September 1987, and the project proceeded to final design.

In 1989, the project was ready for property acquisition but became politically stalled. The Federal Aid Secondary program was ending, and the federal funds had to be spent by 1991. In 1990, the project was defunded from the Federal Aid Secondary program, and the federal funding was spent on road maintenance projects.

In 1991, the Greenley Road Extension became the North-South Connector project without a specific routing as a part of the Countywide Road Program and Traffic Mitigation Fee program adopted by both Tuolumne County and the City of Sonora.

Since 1991, the North-South Connector Project has remained on the list of regional improvements and in the subsequent Regional Transportation Plans with variances in priority. Numerous studies have supported the project's validity.

In 2006, the County Public Works Department completed an alignment study of the North-South connector. The study looked at more than 100 alternative alignments over a large geographic area. Upon conclusion of the study, staff recommended that the Stage I Greenley Road Extension alignments be further considered inclusive of more public outreach, direct consultation with property owners along the route and completion of a Preliminary Environmental Scoping document.

In 2020-2021, Caltrans conducted a Congested Corridor Plan for SR49. This comprehensive study included all modes of transportation for the purpose of reducing traffic congestion on SR49 in Jamestown and Sonora. The study also considered a western alignment of 49 and the Greenley Road Extension. The Caltrans study determined that the long awaited Greenley Road Extension remains the most effective way to reduce traffic congestion in downtown Sonora. The Greenley Road Extension project was identified a local road solution for highway congestion making this project eligible for SB1 Congested Corridor Program funding.

A recent analysis of sidewalk, bike lanes and transit improvement along SR49 determine such improvements would reduce Vehicle Miles Travelled (VMT) daily by 1,237. The proposed Gold Rush Trail Multi-Use Path along SR49 from Chicken Ranch Rancheria, of Me Wuk Indians through Jamestown and Sonora and out to Columbia College with inclusion of an E-bike program would reduce daily vehicle miles travelled in Tuolumne County another 8,023 miles. These alternative multimodal improvements have the potential to provide many benefits to county and city residents. However, the projects will have very little benefit to traffic congestion relief.

Another study currently underway is the Tuolumne County Evacuation Routes and Communications Strategies Report. Thus far, the study has modelled six fire scenarios and based unabated upon rates of spread considered traffic impacts to area roadways. Under several fire scenarios, SR49 through Sonora becomes grid locked. While strategies to move traffic through Sonora are being developed, clearly the addition of Greenley Road Extension would greatly benefit evacuations. Additionally, a forty-foot-wide roadway with vegetation control within the right of way would provide a very beneficial fire break to residents along Browns Flat in the County and Meyer Hills area of Sonora.

In summary, construction of the Greenley Road Extension should have the following beneficial effects to the downtown area:

1. Reduced traffic congestion in the downtown area with reduced traffic on SR49, Washington Street, Elkin Street, Stewart Street, Lyons Street and Mono Way (Restano Way to Greenley Road).

2. Reduced greenhouse gas emissions due to less fuel use with a smoother flow of traffic. Improved traffic flow means less wasted fuel and lower concentrations of emissions which also benefits public health.
3. Safer conditions for pedestrians and vehicles due to less traffic congestion and driver frustration and improved multimodal facilities.
4. Potential economic development with less through traffic in the downtown area; improved traffic conditions for Sonora bound travelers and better access to parking.
5. The provision for an alternate evacuation route, as well as a firebreak around the historic downtown and residential areas.
6. Improved access to critical infrastructure such as Sonora Regional Hospital, Sherriff's Office, Law and Justice Center, Human Services/Public Health as well as shopping areas.

Total Estimated Cost of the Project

Our rough estimate off the total project cost is about \$15 -20 million. If we can get the project through the environmental phase (about \$2 million) the project is eligible for state discretionary SB1 Congested Corridor Program funding. The project is also eligible for Traffic Impact Mitigation Funding which could leverage State-Local Partnership funding.

The next phase, Project Initiation Documents (PID) scope of work includes engineering analysis of alternative alignments, right of way needs, consultation with property owners, public meetings, environmental assessment, economic impact analysis (which might also address possible annexation scenarios), updated cost estimated/schedule, financing plan and perhaps study of mining claims. We estimate the PI phase to be about \$375,000 or more. Upon completion of the PID, the project could be presented to the Board of Supervisors and City Council to determine if there is support to move the project into the design and environmental phase (\$2 million).

Timing

The PID will take about two years to complete. If the County/City/TCTC decide to move forward into the environmental phase through construction, we would estimate an additional 5-8 year project delivery timeframe.

Impact on Resources

To minimize the impacts on staff time, our recommendation is: 1) The TCTC be the lead agency to deliver the Project Initiation Documents (PID); 2) Creation of a Project Development Team, consisting of the Mayor, BOS District 1 Representative, City Manager, City Community Development Director, County Public Works Director, County Community Development Director, possibly County Administrative staff and Caltrans to provide project oversight; 3) a Memorandum of Understanding should be developed between the agencies to express shared goals, objectives, rules and responsibilities; and 4) a diverse consultant team and experienced project manager be contracted to complete the PID.

Project Financing

The PID phase is expected to cost about \$375,000. The City has committed to \$125,000 and the TCTC has also committed to \$125,000. It is recommended that the County use Regional Traffic Impact Mitigation Fees to cover the remaining cost.

The North-South Connector/Greenley Road Extension project is eligible for funding in both the 1991 Regional Road Fund 6141 or the County Wide Traffic Circulation Improvement Program Fund 6144. As the Board is aware, the traffic impact mitigation programs are due for an update. Several major projects have been completed to date and priorities are due for reconsideration. The last time the Board of Supervisors considered funding priorities for traffic impact fee programs expenditures was in 2009 which established the following three categories of priorities:

Category 1

Completed: 1) Widening of Mono Way from Greenley Road to Fir Drive/Sanguinetti

In Progress: 2) SR108/49 five lane widening from 5th Avenue to South Entrance of Main Street (includes several projects listed as State/Local Partnership Projects)

Category 2

Completed: 1) Final improvements to Greenley Road/Mono Way Intersection

Pending: 2) Greenley Road Extension ([portion of North/South Connector project])

Category 3

All other projects listed in Resolution 187-98 (attached)

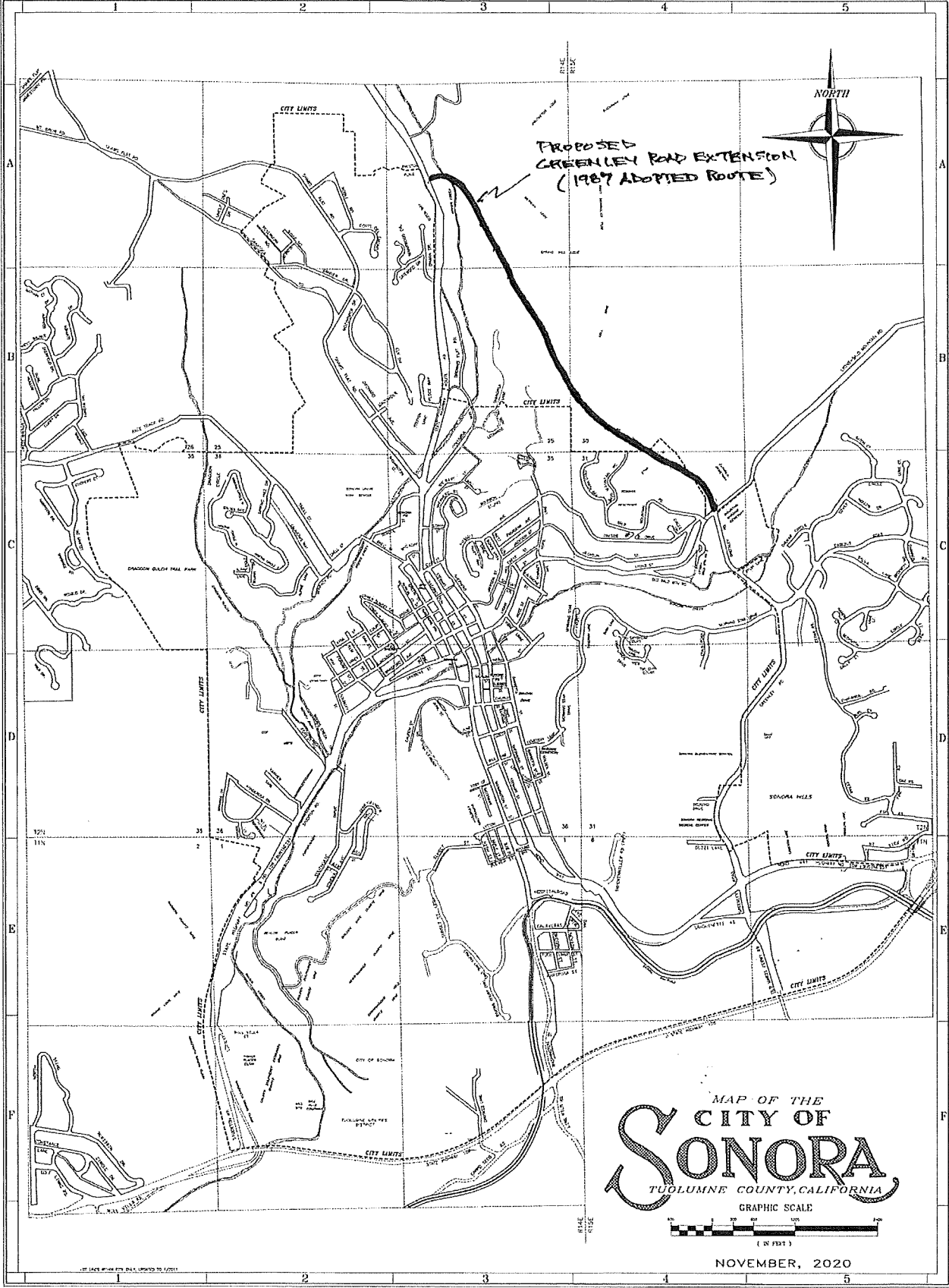
The benefits of Greenley Road Extension noted above continues to reinforce the projects' prioritization for delivery. Staff recommends \$125,000 from funds 6141 and 6144 be made available for completion of the Project Initiation Document for Greenley Extension. Public Works estimates the combined fund balance will be approximately \$2 million after completion of the Rawhide Road Bridge Improvement Project.

Attachments:

1. 1987 Greenley Road Alignment
2. 2006 North/South Connector Alignments studied
3. Emergency Evacuation Route Graphics of overcapacity roadways
4. Regional Traffic Impact Mitigation Funding Project Lists

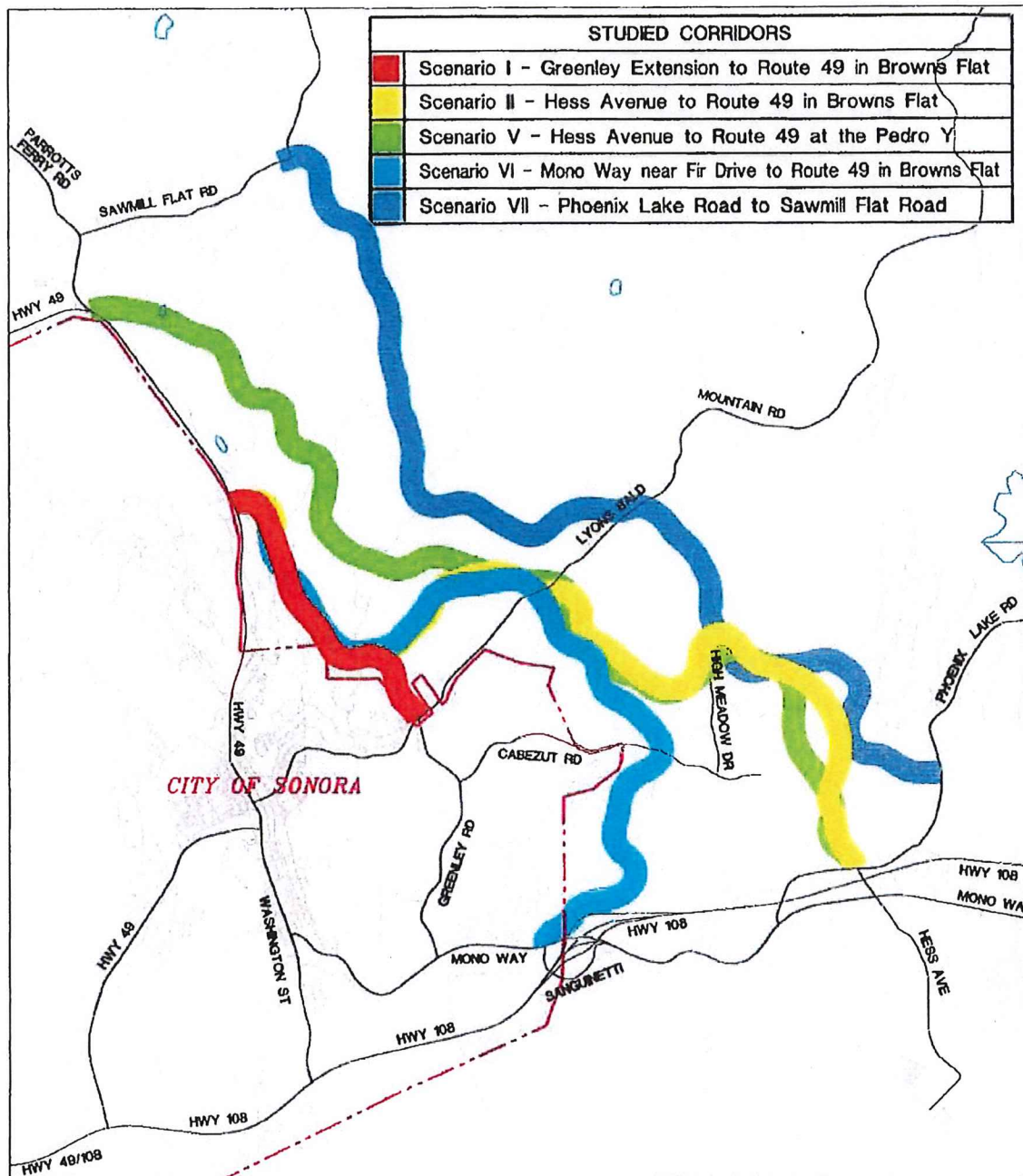
Recommended Action:

- 1) Authorize Public Works staff to collaborate with the TCTC and City of Sonora on the development of a project Initiation Document (PID) for the North/South Connector Phase I Greenley Road Extension.
- 2) Prioritize \$200,000 in Traffic Impact Mitigation Funds 6141 and 6144 for the Greenley Road Extension project.



- Scenario VI – Fir Drive near Mono Way to Route 49 in Brown's Flat
- Scenario VI+ – Fir Drive near Mono Way to Route 49 in Brown's Flat plus the Greenley Extension

A No Build Alternative will be included in the environmental document to measure the benefits of the build alternatives.



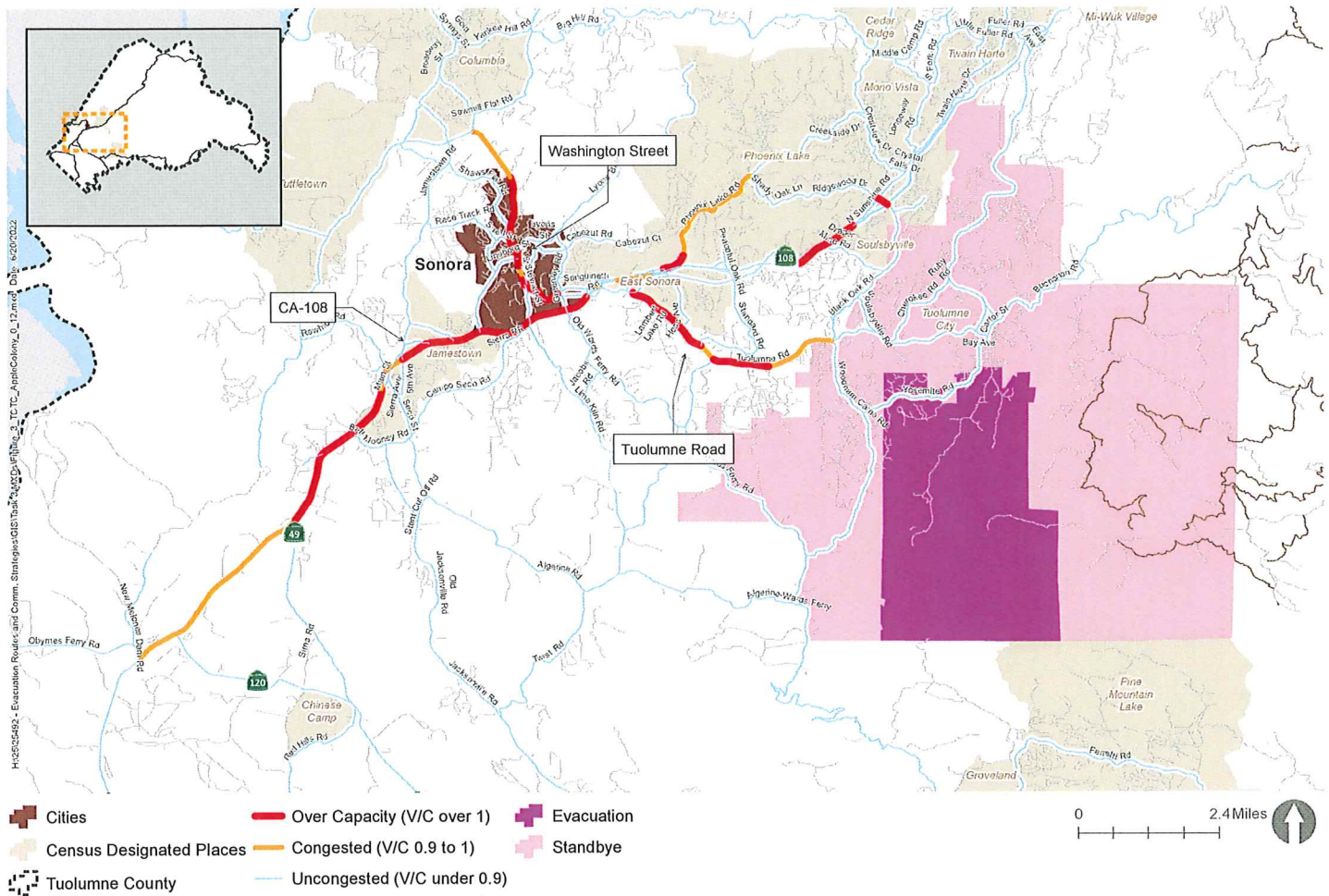


Figure 8

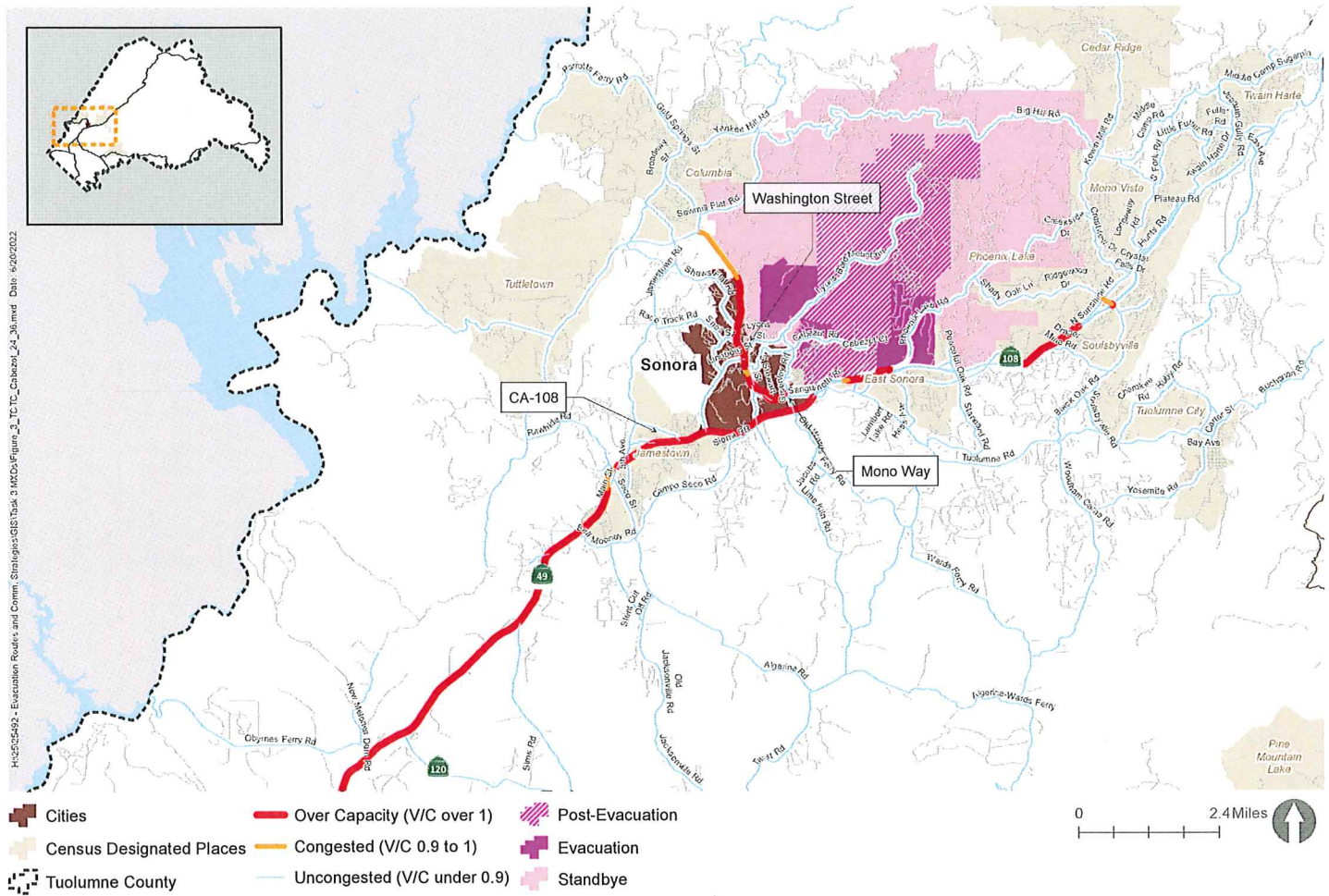


Figure 11

Cabezut Fire Scenario - 24 to 36 Hours
Tuolumne County, CA

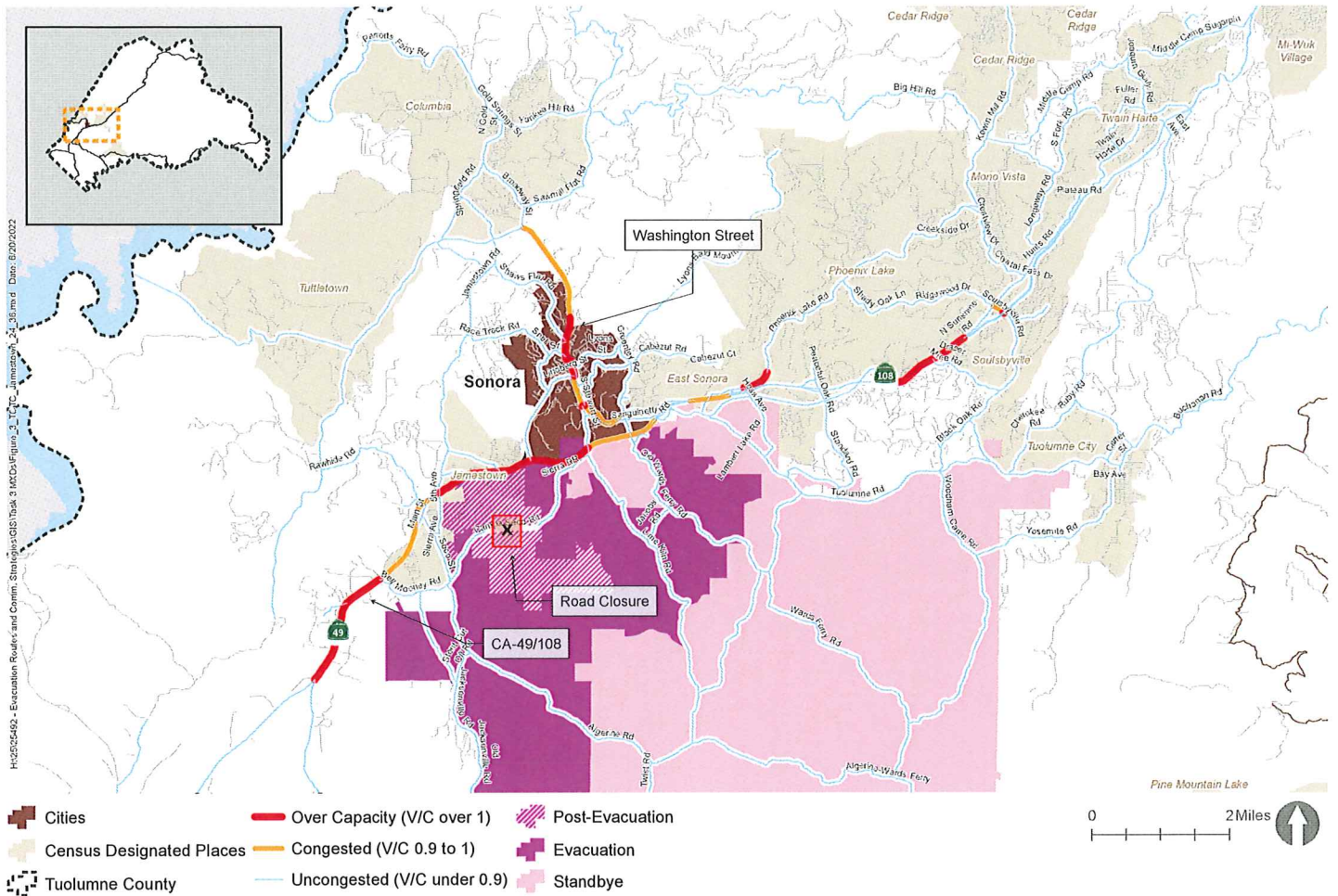


Figure 18

1999 Road Improvement Fund Project List

County-Wide Traffic Circulation Improvement Program

Table I

Adopted as "Table 1" by Board of Supervisors - Resolution 187-98, August 25, 1998
Implementing Ordinance (2254), Effective October 25, 1998 (Fund 6144-301560)
(Projects not in Priority Order)

<u>Local Road Projects</u>	<u>Estimated Cost</u>
	<i>(in thousands)</i>
Cabezut Extension	\$ 7,225
Greenley Road/Mono Way Intersection	\$ 700
Greenley Road Widening (Cabezut Road - Lyons Street)	\$ 750
Mono Way Widening to Four Lanes (Greenley Road - Route 108)	\$ 2,300
Mono Way/State Route 108 Widening to Four Lanes (Phoenix Lake Road - Standard Road)	\$ 6,357
North/South Connector on East Side of Sonora	\$ 7,500
Old Wards Ferry Road/Sanguinetti Road Intersection Alignment	\$ 700
Parrotts Ferry Road Upgrade	\$ 4,845
Peaceful Oak Road Extension to Phoenix Lake Road	\$ 1,375
Standard Road Upgrade	\$ 2,600
Tuolumne Road Widening (Mono Way to Standard Road)	\$ 2,084
Total Local Road Projects	\$ 36,436

Local Intersections Anticipated to Satisfy Signal Warrants by 2015

Cabezut Road Extension/Phoenix Lake Road	
County Road 59/Bonds Flat Road	
Greenley Road/Lyons Bald Mountain Road	
Parrotts Ferry Road/Sawmill Flat Road	
South Washington Street/Church Street	
South Washington Street/Snell and Elkin Streets	
Standard Road/Tuolumne Road	
Tuolumne Road/Cherokee Road	
Tuolumne Road/Woodham Carne Road (including realigning)	
Total Signal Fund Cost	\$ 1,250

<u>State/Local Partnership Projects (50/50 funding)</u>	<u>Local Share</u>
Ferretti Road/State Route 120 Intersection Improvements	\$ 125
SR 108/Jamestown Road/5th Avenue Intersection Improvements	\$ 350
SR 108 four lane widening from Main Street (Jamestown) to Chicken Ranch Road	\$ 3,733
SR 108 four lane widening from Main Street (Jamestown) to SR 49 Junction (south of Sonora)	\$ 5,151
SR 108 /Rawhide Road/Main Street Intersection Improvements	\$ 350
SR 108/South Washington Street Intersection	\$ 200
Total Local Share of Partnership Projects	\$ 9,909
Total Improvement Program Projects (Local Funding)	\$ 47,595

Regional Roads Fund (No. 6141) and Sub-Network Road Fund (No. 6140) Project List

Alternative #3, Exhibit I

Circulation Improvement Projects

Approved by the Board of Supervisors as a 15 Year Program in February 1991

Road Name	Project Limits	Project Description	Total Construction Year Cost	Percent Dev. Share	Develop. Share of Cost	General Public Cost
Regional Roads						
Mono Way	(Fund 6141-301550)	Widen to 5 lanes	2,275,000	100%	2,275,000	0
North/South Connector	Fir Dr to Phoenix Lake Rd	Construct 2-lane connector road to major collector standards	9,641,000	42%	4,049,220	5,591,780
Washington Street	Mono Way to SR 49	Reconstruct intersection at Mono Way ("H" Alternative)	Funded	N	0	0
S. Washington St/Lime Kiln Rd	At Mono Way	Reconstruct intersection at Mono Way ("H" Alternative)	486,646	29%	141,127	345,519
Mono Way, Sanguinetti Rd and Greenley Rd - South	Bulwer St to SR 108	Construct left turn lane at SR 108 and continuous left turn lane between Bulwer St and Hospital Rd	6,028,795	29%	1,748,351	4,280,444
Parrotts Ferry Road	Mono Way to SR 108/Sanguinetti Rd to Fir Dr	Alt. 2DFK from 6/25/90 Omni Means Report. Widen Mono Way to 5 lanes, improve Sanguinetti Rd to 3 lanes and construct Greenley Rd - South connector to Sanguinetti Rd	3,317,867	18%	597,216	2,720,651
Rawhide Road	Sawmill Flat Rd to Airport Rd	Widen to 52 feet	5,072,096	36%	1,825,955	3,246,141
Greenley Road	SR 49/108 to SR 49	Widen to 40 feet in conjunction with bridge replacement and realignment	2,506,100	100%	2,506,100	0
Mono Way	Mono Way to Lyons St	Widen to 3 lanes (2 travel lanes with center left turn lane), improve intersection and modify signalization at Mono Way	1,502,994	34%	511,018	991,976
Washington/Stewart Streets	Stewart St to Sanguinetti Rd	Widen to 3 lanes (2 travel lanes with center left turn lane) and realign Hospital Rd	575,124	29%	166,786	408,338
Parrotts Ferry Road	At Church St	Reconfigure intersection of Church, Stewart and Washington Streets	1,546,278	18%	278,330	1,267,948
Jamestown/Shaws Flat Road	SR 49 to Sawmill Flat Rd	Widen to 64 feet	4,860,427	100%	4,860,427	0
Regional Roads Totals						
			\$37,812,327		\$18,959,529	\$18,852,798
Sub-Network Roads						
East Sonora						
North Hess Avenue	(Fund 6140-301500)	Construct 2-lane major collector on new alignment	2,230,000	29%	646,700	1,583,300
Peaceful Oak Road	Phoenix Lake Rd to Mono Way	Construct Peaceful Oak Rd/Phoenix Lake Rd connector to major collector standards	2,611,120	35%	913,892	1,697,228
Tuolumne Road	Phoenix Lake Rd to Mono Way	Widen to 64 feet	6,176,003	29%	1,791,041	4,384,962
Subtotals			\$11,017,123		\$3,351,633	\$7,665,490

Regional Roads Fund (No. 6141) and Sub-Network Road Fund (No. 6140) Project List

Alternative #3, Exhibit I

Circulation Improvement Projects

Approved by the Board of Supervisors as a 15 Year Program in February 1991

Road Name	Project Limits	Project Description	Total Construction Year Cost	Percent Dev. Share	Develop. Share of Cost	General Public Cost
Groveland						
Ferretti Road	SR 120 (W) to Pine Mtn Lk Ent	Capacity improvements	2,069,477	100%	2,069,477	0
Ferretti Road	Pine Mtn Lk Ent to Clements Rd	Widen to 40 feet	3,892,523	100%	3,892,523	0
Subtotals			\$5,962,000		\$5,962,000	\$0
Jamestown						
Preston Lane	Seco St to SR 49/108	Construct Preston Ln as a major collector and reconstruct the intersection at Main St and SR 49/108 to create a bypass of downtown Jamestown.	937,972	29%	272,012	665,960
Subtotals			\$937,972		\$272,012	\$665,960
Phoenix Lake Area						
Longeway Road	Soulsbyville Rd to Hunts Rd	Widen to 48 feet (2-12 foot travel lanes, 1-12 foot center turn lane and 6 foot shoulders) from Soulsbyville Rd to Hunts Rd	1,060,400	29%	307,516	752,884
Subtotals			\$1,060,400		\$307,516	\$752,884
Sonora						
Leland Drive	Racetrack Rd to SR 49	Extend Leland Dr south to connect with Ponderosa Dr	1,824,715	54%	985,346	839,369
Racetrack Road	Jamestown Rd to Leland Dr	Widen up to 36 feet and realign near Jamestown Rd	974,031	29%	282,469	691,562
Subtotals			\$2,798,746		\$1,267,815	\$1,530,931
Soulsbyville/Tuolumne						
Tuolumne Road	Standard Rd to Woodham Carne Rd	Widen to 4 lanes (2 travel lanes in each direction)	4,821,162	53%	2,555,216	2,265,946
Subtotals			\$4,821,162		\$2,555,216	\$2,265,946
Twain Harte						
Twain Harte Drive	Tiffeni Dr (W) to SR 108	Widening and shoulder improvements, construct left turn lane pockets at SR 108 and Tiffeni Dr (E) and SR 108 signage	1,649,500	100%	1,649,500	0
Twain Harte Drive	At Joaquin Gully Rd	Construct left turn pocket	266,000	100%	266,000	0
Manzanita Drive	Joaquin Gully to Tiffeni Dr (W)	Extend Manzanita Dr to connect with Tiffeni Dr (W)	382,000	55%	210,100	171,900
Subtotals			\$2,297,500		\$2,125,600	\$171,900
Sub-Network Roads Totals			\$28,894,903		\$15,841,792	\$13,053,111
Grand Totals			\$66,707,230		\$34,801,321	\$31,905,909

Appendix B. Traffic Counts

National Data & Surveying Services

Intersection Turning Movement Count

Location: Golden Chain Hwy/N Washington St/SR 49 & N Washington St/Columbia Way/W School St/Shaws Flat Rd
City: Sonora
Control: 2-Way top(EB/WB)

Project ID: 25-090066-001
Date: 5/20/2025

Data - Total

NS/EW Streets:	Golden Chain Hwy/N Washington St/SR 49				Golden Chain Hwy/N Washington St/SR 49				N Washington St/Columbia Way/W School St/Shaws Flat Rd				N Washington St/Columbia Way/W School St/Shaws Flat Rd				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
7:00 AM	22	63	0	0	0	118	5	0	6	0	30	0	5	0	3	0	252
7:15 AM	19	76	0	0	1	130	7	0	0	0	29	0	3	0	0	0	265
7:30 AM	43	103	4	0	0	168	5	0	2	0	56	0	7	1	0	0	389
7:45 AM	32	99	2	0	5	174	5	0	1	0	52	0	5	0	1	0	376
8:00 AM	67	96	9	0	4	139	5	0	5	0	55	0	0	0	1	0	381
8:15 AM	65	98	2	0	3	133	13	0	8	0	68	0	1	0	1	0	392
8:30 AM	29	104	7	0	1	126	1	0	1	0	55	0	1	0	2	0	327
8:45 AM	24	92	2	0	2	124	1	0	2	0	27	0	4	0	0	0	278
TOTAL VOLUMES :	NL 301	NT 731	NR 26	NU 0	SL 16	ST 1112	SR 42	SU 0	EL 25	ET 0	ER 372	EU 0	WL 26	WT 1	WR 8	WU 0	TOTAL 2660
APPROACH %'s :	28.45%	69.09%	2.46%	0.00%	1.37%	95.04%	3.59%	0.00%	6.30%	0.00%	93.70%	0.00%	74.29%	2.86%	22.86%	0.00%	
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	207	396	17	0	12	614	28	0	16	0	231	0	13	1	3	0	1538
PEAK HR FACTOR :	0.772	0.961	0.472	0.000	0.600	0.882	0.538	0.000	0.500	0.000	0.849	0.000	0.464	0.250	0.750	0.000	0.981
	0.901				0.889				0.813				0.531				

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
2:00 PM	29	122	5	0	0	117	4	0	0	1	21	1	1	3	1	0	305
2:15 PM	18	116	2	0	2	118	7	0	3	0	30	0	0	0	0	0	296
2:30 PM	30	140	9	0	0	120	2	0	2	0	22	0	4	0	1	0	330
2:45 PM	34	125	2	0	2	137	0	0	2	0	43	0	5	0	1	0	351
3:00 PM	24	155	4	0	0	152	5	0	2	1	27	0	1	0	2	0	373
3:15 PM	27	118	4	0	2	109	5	0	3	0	25	0	3	1	1	0	298
3:30 PM	56	139	5	1	2	136	5	0	4	0	48	2	2	0	1	0	401
3:45 PM	34	140	8	0	6	79	3	0	2	1	62	0	2	0	1	1	339
4:00 PM	24	138	4	0	11	110	2	0	1	0	37	0	1	0	1	0	329
4:15 PM	34	171	5	1	0	119	4	0	3	0	38	0	4	0	3	0	382
4:30 PM	26	133	6	1	2	128	1	0	3	0	47	0	7	0	1	0	355
4:45 PM	41	156	5	0	3	137	3	0	3	0	38	0	5	0	2	0	393
5:00 PM	47	148	5	0	0	124	6	0	1	0	20	0	4	0	1	0	356
5:15 PM	30	131	6	0	3	100	5	0	5	0	34	0	2	0	0	0	316
5:30 PM	32	124	2	0	1	109	3	0	4	0	22	0	3	0	2	0	302
5:45 PM	23	98	5	0	2	100	1	0	2	0	18	0	5	1	0	0	255
TOTAL VOLUMES :	NL 509	NT 2154	NR 77	NU 3	SL 36	ST 1895	SR 56	SU 0	EL 40	ET 3	ER 532	EU 3	WL 49	WT 5	WR 18	WU 1	TOTAL 5381
APPROACH %'s :	18.56%	78.53%	2.81%	0.11%	1.81%	95.37%	2.82%	0.00%	6.92%	0.52%	92.04%	0.52%	67.12%	6.85%	24.66%	1.37%	
PEAK HR :	04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :	148	608	21	2	5	508	14	0	10	0	143	0	20	0	7	0	1486
PEAK HR FACTOR :	0.787	0.889	0.875	0.500	0.417	0.927	0.583	0.000	0.833	0.000	0.761	0.000	0.714	0.000	0.583	0.000	0.945
	0.923				0.921				0.765				0.844				

National Data & Surveying Services

Intersection Turning Movement Count

Location: Golden Chain Hwy/N Washington St/SR 49 & N Washington St/Columbia Way/W School St/Shaws Flat Rd
City: Sonora
Control: 2-Way top(EB/WB)

Project ID: 25-090066-001
Date: 5/20/2025

Data - Cars

NS/EW Streets:		Golden Chain Hwy/N Washington St/SR 49				Golden Chain Hwy/N Washington St/SR 49				N Washington St/Columbia Way/W School St/Shaws Flat Rd				N Washington St/Columbia Way/W School St/Shaws Flat Rd				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
	7:00 AM	22	57	0	0	0	106	5	0	6	0	29	0	5	0	3	0	233
	7:15 AM	18	68	0	0	0	124	7	0	0	0	29	0	3	0	0	0	249
	7:30 AM	40	87	4	0	0	160	5	0	2	0	56	0	7	1	0	0	362
	7:45 AM	32	89	2	0	5	170	5	0	1	0	52	0	5	0	1	0	362
	8:00 AM	66	87	7	0	4	131	5	0	5	0	55	0	0	0	1	0	361
	8:15 AM	56	91	2	0	3	123	13	0	8	0	67	0	1	0	1	0	365
	8:30 AM	25	95	7	0	1	123	1	0	1	0	54	0	1	0	2	0	310
	8:45 AM	23	84	2	0	2	119	0	0	2	0	26	0	4	0	0	0	262
TOTAL VOLUMES :		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :		282	658	24	0	15	1056	41	0	25	0	368	0	26	1	8	0	2504
APPROACH %'s :		29.25%	68.26%	2.49%	0.00%	1.35%	94.96%	3.69%	0.00%	6.36%	0.00%	93.64%	0.00%	74.29%	2.86%	22.86%	0.00%	
PEAK HR :		07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :		194	354	15	0	12	584	28	0	16	0	230	0	13	1	3	0	1450
PEAK HR FACTOR :		0.735	0.973	0.536	0.000	0.600	0.859	0.538	0.000	0.500	0.000	0.858	0.000	0.464	0.250	0.750	0.000	0.993
		0.880				0.867				0.820				0.531				
PM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
	2:00 PM	28	115	3	0	0	109	3	0	0	1	18	1	1	3	1	0	283
	2:15 PM	18	107	2	0	2	115	7	0	3	0	29	0	0	0	0	0	283
	2:30 PM	30	132	9	0	0	112	2	0	1	0	20	0	4	0	1	0	311
	2:45 PM	34	120	2	0	1	127	0	0	2	0	40	0	5	0	1	0	332
	3:00 PM	22	149	4	0	0	141	3	0	2	1	26	0	1	0	2	0	351
	3:15 PM	24	110	4	0	2	100	5	0	3	0	23	0	3	1	1	0	276
	3:30 PM	54	132	5	1	2	134	5	0	4	0	46	2	2	0	1	0	388
	3:45 PM	31	138	8	0	6	71	3	0	2	1	62	0	2	0	1	1	326
	4:00 PM	22	137	4	0	11	102	2	0	0	0	37	0	1	0	1	0	317
	4:15 PM	33	168	4	1	0	111	3	0	3	0	38	0	4	0	3	0	368
	4:30 PM	26	128	6	1	2	124	1	0	3	0	47	0	4	0	1	0	343
	4:45 PM	39	152	5	0	3	131	2	0	3	0	38	0	4	0	2	0	379
	5:00 PM	45	147	5	0	0	121	6	0	1	0	19	0	4	0	1	0	349
	5:15 PM	30	130	6	0	3	94	4	0	5	0	34	0	2	0	0	0	308
	5:30 PM	31	121	2	0	1	106	3	0	4	0	22	0	3	0	2	0	295
	5:45 PM	23	95	5	0	2	97	1	0	2	0	18	0	5	1	0	0	249
TOTAL VOLUMES :		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :		490	2081	74	3	35	1795	50	0	38	3	517	3	45	5	18	1	5158
APPROACH %'s :		18.50%	78.59%	2.79%	0.11%	1.86%	95.48%	2.66%	0.00%	6.77%	0.53%	92.16%	0.53%	65.22%	7.25%	26.09%	1.45%	
PEAK HR :		04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :		143	595	20	2	5	487	12	0	10	0	142	0	16	0	7	0	1439
PEAK HR FACTOR :		0.794	0.885	0.833	0.500	0.417	0.929	0.500	0.000	0.833	0.000	0.755	0.000	1.000	0.000	0.583	0.000	0.949
		0.922				0.926				0.760				0.821				

National Data & Surveying Services

Intersection Turning Movement Count

Location: Golden Chain Hwy/N Washington St/SR 49 & N Washington St/Columbia Way/W School St/Shaws Flat Rd
City: Sonora
Control: 2-Way top(EB/WB)

Project ID: 25-090066-001
Date: 5/20/2025

Data - HT

NS/EW Streets:		Golden Chain Hwy/N Washington St/SR 49				Golden Chain Hwy/N Washington St/SR 49				N Washington St/Columbia Way/W School St/Shaws Flat Rd				N Washington St/Columbia Way/W School St/Shaws Flat Rd				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
	7:00 AM	0	6	0	0	0	12	0	0	0	0	1	0	0	0	0	0	19
	7:15 AM	1	8	0	0	1	6	0	0	0	0	0	0	0	0	0	0	16
	7:30 AM	3	16	0	0	0	8	0	0	0	0	0	0	0	0	0	0	27
	7:45 AM	0	10	0	0	0	4	0	0	0	0	0	0	0	0	0	0	14
	8:00 AM	1	9	2	0	0	8	0	0	0	0	0	0	0	0	0	0	20
	8:15 AM	9	7	0	0	0	10	0	0	0	0	1	0	0	0	0	0	27
	8:30 AM	4	9	0	0	0	3	0	0	0	0	1	0	0	0	0	0	17
	8:45 AM	1	8	0	0	0	5	1	0	0	0	1	0	0	0	0	0	16
TOTAL VOLUMES :		NL 19	NT 73	NR 2	NU 0	SL 1	ST 56	SR 1	SU 0	EL 0	ET 0	ER 4	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 156
APPROACH %'s :		20.21%	77.66%	2.13%	0.00%	1.72%	96.55%	1.72%	0.00%	0.00%	0.00%	100.00%	0.00%					
PEAK HR :		07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :		13	42	2	0	0	30	0	0	0	0	1	0	0	0	0	0	88
PEAK HR FACTOR :		0.361	0.656	0.250	0.000	0.000	0.750	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.815
		0.750				0.750				0.250								
PM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
	2:00 PM	1	7	2	0	0	8	1	0	0	0	3	0	0	0	0	0	22
	2:15 PM	0	9	0	0	0	3	0	0	0	0	1	0	0	0	0	0	13
	2:30 PM	0	8	0	0	0	8	0	0	1	0	2	0	0	0	0	0	19
	2:45 PM	0	5	0	0	1	10	0	0	0	0	3	0	0	0	0	0	19
	3:00 PM	2	6	0	0	0	11	2	0	0	0	1	0	0	0	0	0	22
	3:15 PM	3	8	0	0	0	9	0	0	0	0	2	0	0	0	0	0	22
	3:30 PM	2	7	0	0	0	2	0	0	0	0	2	0	0	0	0	0	13
	3:45 PM	3	2	0	0	0	8	0	0	0	0	0	0	0	0	0	0	13
	4:00 PM	2	1	0	0	0	8	0	0	1	0	0	0	0	0	0	0	12
	4:15 PM	1	3	1	0	0	8	1	0	0	0	0	0	0	0	0	0	14
	4:30 PM	0	5	0	0	0	4	0	0	0	0	0	0	3	0	0	0	12
	4:45 PM	2	4	0	0	0	6	1	0	0	0	0	0	1	0	0	0	14
	5:00 PM	2	1	0	0	0	3	0	0	0	0	1	0	0	0	0	0	7
	5:15 PM	0	1	0	0	0	6	1	0	0	0	0	0	0	0	0	0	8
	5:30 PM	1	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	7
	5:45 PM	0	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	6
TOTAL VOLUMES :		NL 19	NT 73	NR 3	NU 0	SL 1	ST 100	SR 6	SU 0	EL 2	ET 0	ER 15	EU 0	WL 4	WT 0	WR 0	WU 0	TOTAL 223
APPROACH %'s :		20.00%	76.84%	3.16%	0.00%	0.93%	93.46%	5.61%	0.00%	11.76%	0.00%	88.24%	0.00%	100.00%	0.00%	0.00%	0.00%	
PEAK HR :		04:15 PM - 05:15 PM																TOTAL
PEAK HR VOL :		5	13	1	0	0	21	2	0	0	0	1	0	4	0	0	0	47
PEAK HR FACTOR :		0.625	0.650	0.250	0.000	0.000	0.656	0.500	0.000	0.000	0.000	0.250	0.000	0.333	0.000	0.000	0.000	0.839
		0.792				0.639				0.250				0.333				

National Data & Surveying Services

Intersection Turning Movement Count

Location: Golden Chain Hwy/N Washington St/SR 49 & N Washington St/Columbia Way/W School St/Shaws Flat Rd
City: Sonora
Control: 2-Way top(EB/WB)

Project ID: 25-090066-001
Date: 5/20/2025

Data - Bikes

NS/EW Streets:		Golden Chain Hwy/N Washington St/SR 49				Golden Chain Hwy/N Washington St/SR 49				N Washington St/Columbia Way/W School St/Shaws Flat Rd				N Washington St/Columbia Way/W School St/Shaws Flat Rd				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :		0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2
PEAK HR :		07:30 AM - 08:30 AM								0.00% 0.00% 100.00% 0.00%								TOTAL
PEAK HR VOL :		0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2
PEAK HR FACTOR :		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.250
PM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		1 NL	1 NT	0 NR	0 NU	1 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
	2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	2:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	2:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
	2:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	3:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	3:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	3:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:00 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
	4:15 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2
	5:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
	5:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
TOTAL VOLUMES :		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :		2	0	1	0	0	3	0	0	0	0	2	0	0	0	0	0	8
PEAK HR :		04:15 PM - 05:15 PM								66.67% 0.00% 33.33% 0.00%								TOTAL
PEAK HR VOL :		0	0	1	0	0	2	0	0	0	0	1	0	0	0	0	0	4
PEAK HR FACTOR :		0.000	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.500

National Data & Surveying Services
Intersection Turning Movement Count

Location: Golden Chain Hwy/N Washington St/SR 49 & N Washington St/Colui **Project ID:** 25-090066-001
City: Sonora **Date:** 5/20/2025

Data - Pedestrians (Crosswalks)

NS/EW Streets:	Golden Chain Hwy/N Washington St/SR 49		Golden Chain Hwy/N Washington St/SR 49		N Washington St/Columbia Way/W		N Washington St/Columbia Way/W		
AM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	1	0	0	0	1
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	1	0	0	0	0	1	0	0	2
8:45 AM	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES :	EB 1	WB 0	EB 0	WB 0	NB 1	SB 1	NB 1	SB 0	TOTAL 4
APPROACH %'s :	100.00%	0.00%			50.00%	50.00%	100.00%	0.00%	
PEAK HR :	07:30 AM - 08:30 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
2:00 PM	1	0	0	0	0	0	0	0	1
2:15 PM	0	0	0	0	0	0	0	0	0
2:30 PM	0	0	0	0	0	0	2	0	2
2:45 PM	0	0	0	0	0	0	0	0	0
3:00 PM	0	0	0	0	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	0	0	0
3:30 PM	0	0	0	0	1	1	0	0	2
3:45 PM	0	0	0	0	3	1	1	1	6
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	1	3	4
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	1	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	1	0	0	1
5:45 PM	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES :	EB 1	WB 0	EB 0	WB 0	NB 5	SB 3	NB 5	SB 4	TOTAL 18
APPROACH %'s :	100.00%	0.00%			62.50%	37.50%	55.56%	44.44%	
PEAK HR :	04:15 PM - 05:15 PM								TOTAL
PEAK HR VOL :	0	0	0	0	1	0	1	3	5
PEAK HR FACTOR :					0.250	0.250	0.250	0.250	0.313

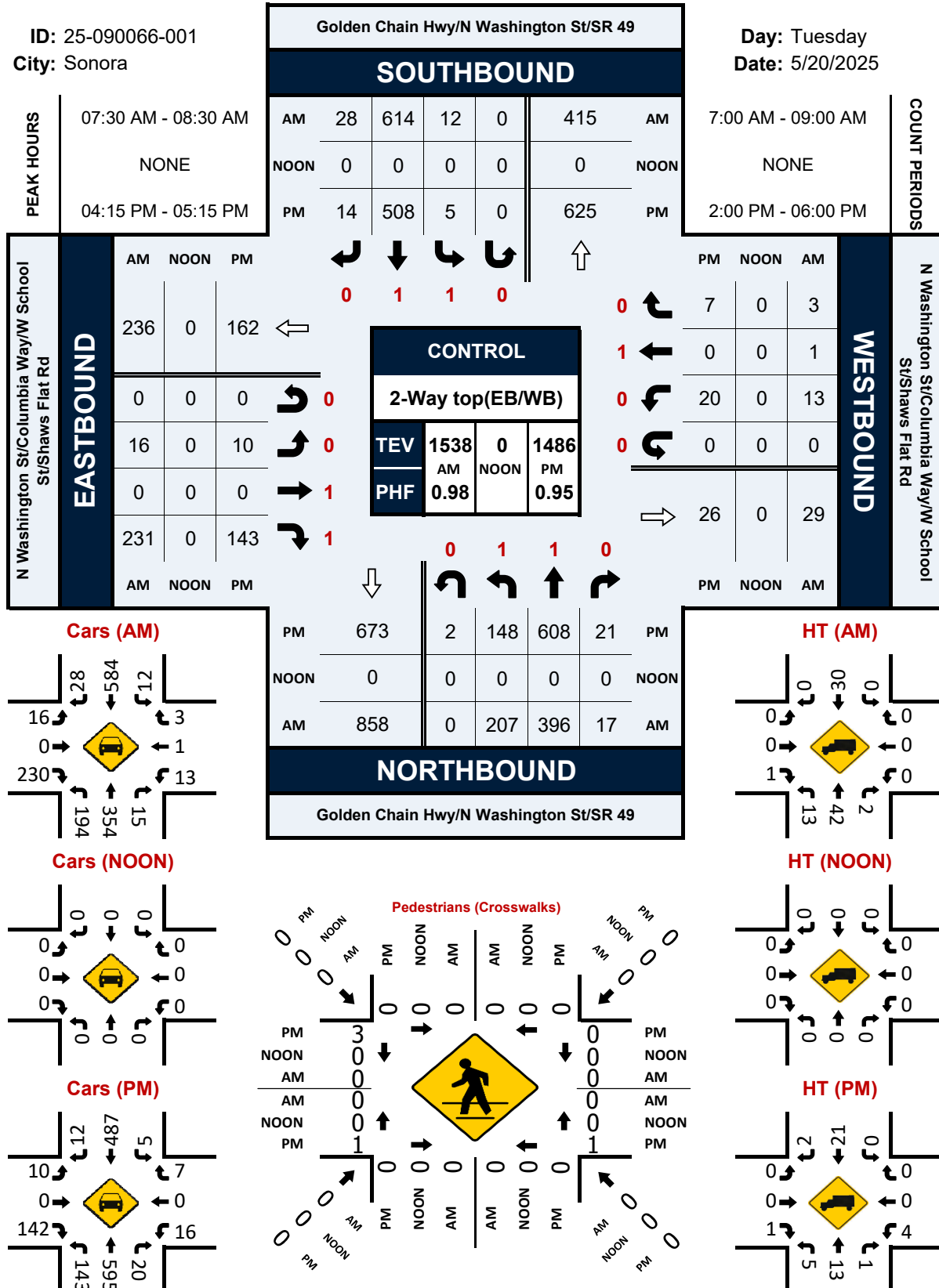
Peak Hour Turning Movement Count

ID: 25-090066-001

City: Sonora

Day: Tuesday

Date: 5/20/2025

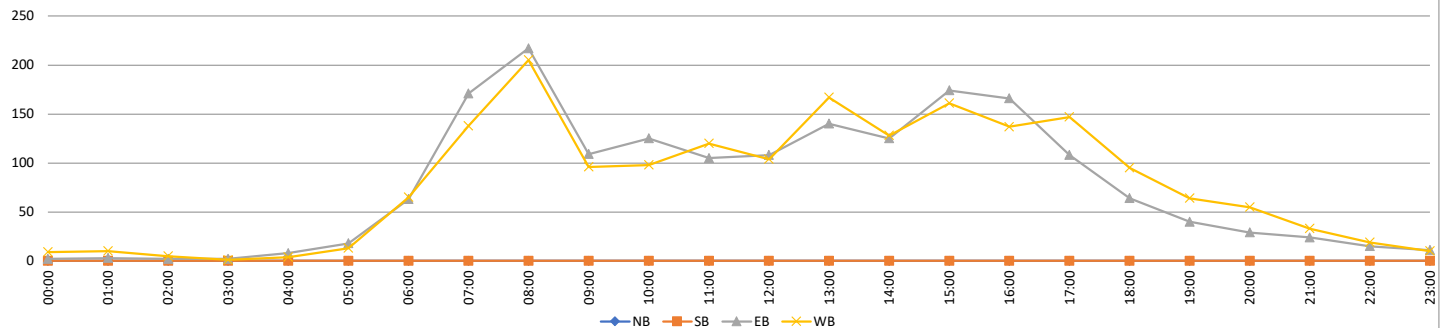


VOLUME**N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49**

Day: Tuesday
Date: 5/20/2025

City: Sonora
Project #: CA25_090067_001

DAILY TOTALS						NB				SB				EB				WB				Total		DAILY TOTALS					
						0				0				1,829				1,884				3,713							
15-Minutes Interval												Hourly Intervals																	
TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL												
0:00			1	3	4	12:00			22	33	55	00:00	01:00		2	9	11												
0:15			1	3	4	12:15			33	20	53	01:00	02:00		3	10	13												
0:30			0	2	2	12:30			24	27	51	02:00	03:00		2	5	7												
0:45			0	1	1	12:45			29	24	53	03:00	04:00		2	1	3												
1:00			0	2	2	13:00			33	45	78	04:00	05:00		8	4	12												
1:15			1	4	5	13:15			55	42	97	05:00	06:00		18	13	31												
1:30			2	4	6	13:30			27	34	61	06:00	07:00		63	65	128												
1:45			0	0	0	13:45			25	46	71	07:00	08:00		171	138	309												
2:00			0	3	3	14:00			23	36	59	08:00	09:00		217	205	422												
2:15			0	2	2	14:15			33	25	58	09:00	10:00		109	96	205												
2:30			2	0	2	14:30			23	32	55	10:00	11:00		125	98	223												
2:45			0	0	0	14:45			46	35	81	11:00	12:00		105	120	225												
3:00			0	0	0	15:00			28	29	57	12:00	13:00		108	104	212												
3:15			1	1	2	15:15			29	33	62	13:00	14:00		140	167	307												
3:30			0	0	0	15:30			52	62	114	14:00	15:00		125	128	253												
3:45			1	0	1	15:45			65	37	102	15:00	16:00		174	161	335												
4:00			0	1	1	16:00			36	27	63	16:00	17:00		166	137	303												
4:15			0	0	0	16:15			40	38	78	17:00	18:00		108	147	255												
4:30			4	3	7	16:30			50	28	78	18:00	19:00		64	95	159												
4:45			4	0	4	16:45			40	44	84	19:00	20:00		40	64	104												
5:00			2	0	2	17:00			21	52	73	20:00	21:00		29	55	84												
5:15			4	3	7	17:15			40	35	75	21:00	22:00		24	33	57												
5:30			6	3	9	17:30			28	35	63	22:00	23:00		15	19	34												
5:45			6	7	13	17:45			19	25	44	23:00	00:00		11	10	21												
6:00			10	8	18	18:00			16	28	44	STATISTICS																	
6:15			10	2	12	18:15			18	25	43		NB	SB	EB	WB	TOTAL												
6:30			23	20	43	18:30			16	15	31	Peak Period	00:00	to	12:00														
6:45			20	35	55	18:45			14	27	41	Volume				825	764	1589											
7:00			35	28	63	19:00			10	17	27	Peak Hour				7:30	7:30	7:30											
7:15			30	26	56	19:15			13	18	31	Peak Volume				241	234	475											
7:30			55	48	103	19:30			8	20	28	Peak Hour Factor				0.803	0.750	0.776											
7:45			51	36	87	19:45			9	9	18																		
8:00			60	72	132	20:00			6	14	20	Peak Period	12:00	to	00:00														
8:15			75	78	153	20:15			6	17	23	Volume				1004	1120	2124											
8:30			52	30	82	20:30			10	18	28	Peak Hour				15:30	13:00	15:30											
8:45			30	25	55	20:45			7	6	13	Peak Volume				193	167	357											
9:00			23	26	49	21:00			6	8	14	Peak Hour Factor				0.742	0.908	0.783											
9:15			23	28	51	21:15			7	8	15																		
9:30			32	24	56	21:30			4	6	10	Peak Period	07:00	to	09:00														
9:45			31	18	49	21:45			7	11	18	Volume				388	343	731											
10:00			24	22	46	22:00			6	6	12	Peak Hour				7:30	7:30	7:30											
10:15			26	22	48	22:15			6	6	12	Peak Volume				241	234	475											
10:30			51	23	74	22:30			2	3	5	Peak Hour Factor				0.803	0.750	0.776											
10:45			24	31	55	22:45			1	4	5																		
11:00			16	34	50	23:00			1	2	3	Peak Period	16:00	to	18:00														
11:15			23	24	47	23:15			7	3	10	Volume				274	284	558											
11:30			40	28	68	23:30			3	3	6	Peak Hour				16:00	16:45	16:15											
11:45			26	34	60	23:45			0	2	2	Peak Volume				166	166	313											
TOTALS	0	0	825	764	1589	TOTALS	0	0	1004	1120	2124	Peak Hour Factor				0.830	0.798	0.932											
SPLIT %	0%	0%	52%	48%	43%	SPLIT %	0%	0%	47%	53%	57%																		



VOLUME**N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49**

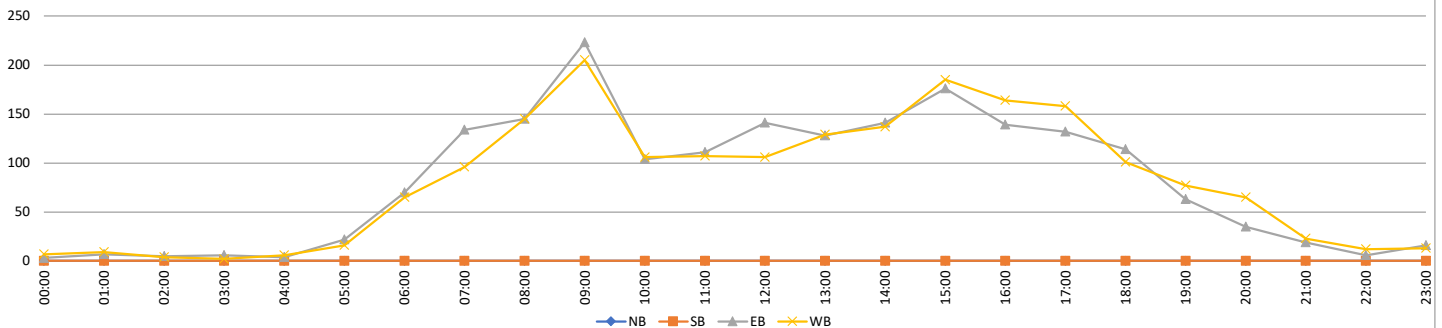
Day: Wednesday

City: Sonora

Date: 5/21/2025

Project #: CA25_090067_001

DAILY TOTALS						NB				SB				EB				WB				Total		DAILY TOTALS					
						0		0		1,944		1,938		3,882															
15-Minutes Interval												Hourly Intervals																	
TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL												
0:00			1	3	4	12:00			42	25	67	00:00	01:00		3	7	10												
0:15			1	1	2	12:15			20	26	46	01:00	02:00		7	9	16												
0:30			0	0	0	12:30			48	26	74	02:00	03:00		5	4	9												
0:45			1	3	4	12:45			31	29	60	03:00	04:00		6	2	8												
1:00			2	4	6	13:00			29	35	64	04:00	05:00		4	6	10												
1:15			2	0	2	13:15			36	30	66	05:00	06:00		22	16	38												
1:30			1	4	5	13:30			34	26	60	06:00	07:00		70	65	135												
1:45			2	1	3	13:45			29	38	67	07:00	08:00		134	96	230												
2:00			0	2	2	14:00			34	34	68	08:00	09:00		145	145	290												
2:15			1	1	2	14:15			41	37	78	09:00	10:00		223	205	428												
2:30			1	1	2	14:30			27	26	53	10:00	11:00		104	106	210												
2:45			3	0	3	14:45			39	40	79	11:00	12:00		111	107	218												
3:00			1	1	2	15:00			23	27	50	12:00	13:00		141	106	247												
3:15			0	1	1	15:15			35	54	89	13:00	14:00		128	129	257												
3:30			2	0	2	15:30			49	59	108	14:00	15:00		141	137	278												
3:45			3	0	3	15:45			69	45	114	15:00	16:00		176	185	361												
4:00			1	1	2	16:00			44	42	86	16:00	17:00		139	164	303												
4:15			0	2	2	16:15			26	47	73	17:00	18:00		132	158	290												
4:30			1	2	3	16:30			41	42	83	18:00	19:00		114	101	215												
4:45			2	1	3	16:45			28	33	61	19:00	20:00		63	77	140												
5:00			3	1	4	17:00			27	40	67	20:00	21:00		35	65	100												
5:15			3	0	3	17:15			32	46	78	21:00	22:00		19	23	42												
5:30			6	4	10	17:30			42	41	83	22:00	23:00		6	12	18												
5:45			10	11	21	17:45			31	31	62	23:00	00:00		16	13	29												
6:00			4	7	11	18:00			43	34	77	STATISTICS																	
6:15			12	8	20	18:15			30	28	58		NB	SB	EB	WB	TOTAL												
6:30			27	21	48	18:30			18	24	42	Peak Period	00:00	to 12:00															
6:45			27	29	56	18:45			23	15	38	Volume			834	768	1602												
7:00			22	23	45	19:00			16	18	34	Peak Hour			9:00	8:45	9:00												
7:15			24	15	39	19:15			13	25	38	Peak Volume			223	213	428												
7:30			46	22	68	19:30			14	16	30	Peak Hour Factor			0.613	0.612	0.601												
7:45			42	36	78	19:45			20	18	38																		
8:00			32	37	69	20:00			6	18	24	Peak Period	12:00	to 00:00															
8:15			44	45	89	20:15			9	23	32	Volume			1110	1170	2280												
8:30			32	28	60	20:30			13	11	24	Peak Hour			15:15	15:15	15:15												
8:45			37	35	72	20:45			7	13	20	Peak Volume			197	200	397												
9:00			38	37	75	21:00			6	10	16	Peak Hour Factor			0.714	0.847	0.871												
9:15			44	54	98	21:15			7	3	10																		
9:30			91	87	178	21:30			3	6	9	Peak Period	07:00	to 09:00															
9:45			50	27	77	21:45			3	4	7	Volume			279	241	520												
10:00			20	35	55	22:00			4	2	6	Peak Hour			7:30	7:45	7:30												
10:15			26	29	55	22:15			0	5	5	Peak Volume			164	146	304												
10:30			38	25	63	22:30			0	4	4	Peak Hour Factor			0.891	0.811	0.854												
10:45			20	17	37	22:45			2	1	3																		
11:00			27	22	49	23:00			2	0	2	Peak Period	16:00	to 18:00															
11:15			28	21	49	23:15			7	2	9	Volume			271	322	593												
11:30			34	30	64	23:30			3	6	9	Peak Hour			16:00	16:00	16:00												
11:45			22	34	56	23:45			4	5	9	Peak Volume			139	164	303												
TOTALS	0	0	834	768	1602	TOTALS	0	0	1110	1170	2280	Peak Hour Factor			0.790	0.872	0.881												
SPLIT %	0%	0%	52%	48%	41%	SPLIT %	0%	0%	49%	51%	59%																		

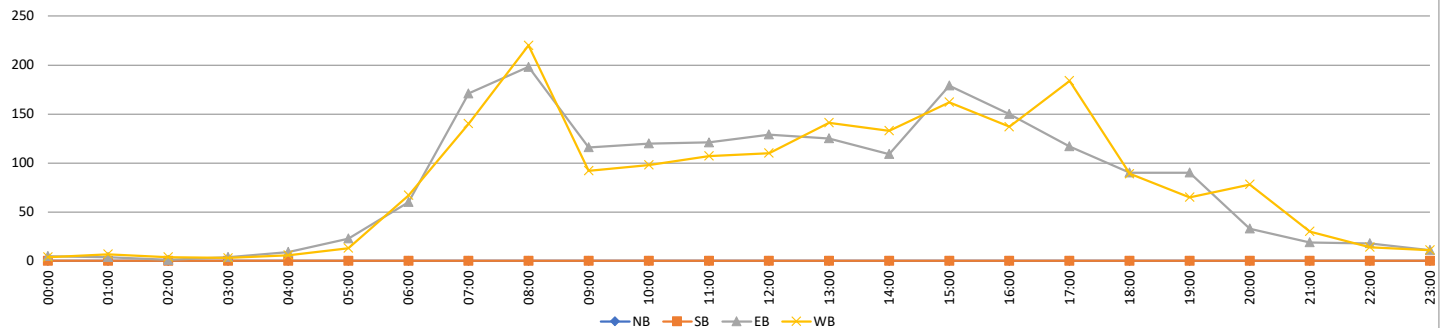


VOLUME**N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49**

Day: Thursday
Date: 5/22/2025

City: Sonora
Project #: CA25_090067_001

DAILY TOTALS						NB				SB				EB				WB				Total		DAILY TOTALS					
						0				0				1,902				1,915				3,817							
15-Minutes Interval												Hourly Intervals																	
TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL												
0:00			2	2	4	12:00			26	24	50	00:00	01:00		5	4	9												
0:15			3	1	4	12:15			26	28	54	01:00	02:00		4	7	11												
0:30			0	0	0	12:30			36	31	67	02:00	03:00		1	4	5												
0:45			0	1	1	12:45			41	27	68	03:00	04:00		4	3	7												
1:00			1	2	3	13:00			26	24	50	04:00	05:00		9	6	15												
1:15			1	2	3	13:15			37	42	79	05:00	06:00		23	13	36												
1:30			2	3	5	13:30			32	32	64	06:00	07:00		60	67	127												
1:45			0	0	0	13:45			30	43	73	07:00	08:00		171	140	311												
2:00			0	3	3	14:00			29	29	58	08:00	09:00		198	220	418												
2:15			0	0	0	14:15			22	34	56	09:00	10:00		116	92	208												
2:30			0	0	0	14:30			21	33	54	10:00	11:00		120	98	218												
2:45			1	1	2	14:45			37	37	74	11:00	12:00		121	107	228												
3:00			0	1	1	15:00			28	26	54	12:00	13:00		129	110	239												
3:15			0	0	0	15:15			48	37	85	13:00	14:00		125	141	266												
3:30			2	0	2	15:30			41	63	104	14:00	15:00		109	133	242												
3:45			2	2	4	15:45			62	36	98	15:00	16:00		179	162	341												
4:00			2	0	2	16:00			43	33	76	16:00	17:00		150	137	287												
4:15			1	2	3	16:15			29	38	67	17:00	18:00		117	184	301												
4:30			1	3	4	16:30			44	33	77	18:00	19:00		90	89	179												
4:45			5	1	6	16:45			34	33	67	19:00	20:00		90	65	155												
5:00			3	3	6	17:00			37	40	77	20:00	21:00		33	78	111												
5:15			4	2	6	17:15			31	42	73	21:00	22:00		19	30	49												
5:30			8	2	10	17:30			24	46	70	22:00	23:00		18	14	32												
5:45			8	6	14	17:45			25	56	81	23:00	00:00		11	11	22												
6:00			6	8	14	18:00			42	30	72	STATISTICS																	
6:15			11	10	21	18:15			22	19	41		NB	SB	EB	WB	TOTAL												
6:30			23	13	36	18:30			15	14	29	Peak Period	00:00	to	12:00														
6:45			20	36	56	18:45			11	26	37	Volume				832	761	1593											
7:00			33	22	55	19:00			10	18	28	Peak Hour				7:45	7:30	7:30											
7:15			32	27	59	19:15			32	12	44	Peak Volume				220	238	453											
7:30			51	49	100	19:30			29	16	45	Peak Hour Factor				0.917	0.753	0.815											
7:45			55	42	97	19:45			19	19	38																		
8:00			49	68	117	20:00			15	31	46	Peak Period	12:00	to	00:00														
8:15			60	79	139	20:15			8	16	24	Volume				1070	1154	2224											
8:30			56	44	100	20:30			8	21	29	Peak Hour				15:15	17:00	15:15											
8:45			33	29	62	20:45			2	10	12	Peak Volume				194	184	363											
9:00			42	23	65	21:00			3	9	12	Peak Hour Factor				0.782	0.821	0.873											
9:15			25	23	48	21:15			2	6	8																		
9:30			25	25	50	21:30			7	9	16	Peak Period	07:00	to	09:00														
9:45			24	21	45	21:45			7	6	13	Volume				369	360	729											
10:00			19	15	34	22:00			4	1	5	Peak Hour				7:45	7:30	7:30											
10:15			31	26	57	22:15			5	4	9	Peak Volume				220	238	453											
10:30			39	32	71	22:30			5	5	10	Peak Hour Factor				0.917	0.753	0.815											
10:45			31	25	56	22:45			4	4	8																		
11:00			35	29	64	23:00			1	0	1	Peak Period	16:00	to	18:00														
11:15			22	21	43	23:15			8	4	12	Volume				267	321	588											
11:30			39	27	66	23:30			2	5	7	Peak Hour				16:00	17:00	17:00											
11:45			25	30	55	23:45			0	2	2	Peak Volume				150	184	301											
TOTALS	0	0	832	761	1593	TOTALS	0	0	1070	1154	2224	Peak Hour Factor				0.852	0.821	0.929											
SPLIT %	0%	0%	52%	48%	42%	SPLIT %	0%	0%	48%	52%	58%																		



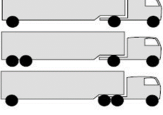
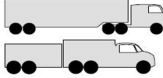
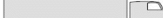
CLASSIFICATION

N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49

Day: Tuesday
Date: 5/20/2025

City: Sonora
Project #: CAZ5_090067_001

HOURLY BREAKDOWN	Time	EASTBOUND													Total	WESTBOUND													Total	TOTALS													Total
	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total			
	0:00	0	2	0	0	0	0	0	0	0	0	0	0	0	2	0	7	2	0	0	0	0	0	0	0	0	9	0	9	2	0	0	0	0	0	0	0	0	0	0	0	11	
	1:00	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	9	1	0	0	0	0	0	0	0	0	10	0	11	2	0	0	0	0	0	0	0	0	0	0	13		
	2:00	0	0	2	0	0	0	0	0	0	0	0	0	0	2	0	3	2	0	0	0	0	0	0	0	0	5	0	3	4	0	0	0	0	0	0	0	0	0	0	7		
	3:00	0	2	0	0	0	0	0	0	0	0	0	0	0	2	0	0	1	0	0	0	0	0	0	0	0	1	0	2	1	0	0	0	0	0	0	0	0	0	0	3		
	4:00	0	5	3	0	0	0	0	0	0	0	0	0	0	8	0	3	1	0	0	0	0	0	0	0	0	4	0	8	4	0	0	0	0	0	0	0	0	0	0	12		
	5:00	0	6	11	1	0	0	0	0	0	0	0	0	0	18	0	8	5	0	0	0	0	0	0	0	0	13	0	14	16	1	0	0	0	0	0	0	0	0	0	31		
	6:00	2	33	19	9	0	0	0	0	0	0	0	0	0	63	1	43	21	0	0	0	0	0	0	0	0	65	3	76	40	9	0	0	0	0	0	0	0	0	0	128		
	7:00	1	121	48	1	0	0	0	0	0	0	0	0	0	171	0	99	35	1	3	0	0	0	0	0	0	138	1	220	83	2	3	0	0	0	0	0	0	0	0	309		
	8:00	2	170	42	1	2	0	0	0	0	0	0	0	0	217	2	140	47	14	2	0	0	0	0	0	0	205	4	310	89	15	4	0	0	0	0	0	0	0	0	422		
	9:00	1	68	38	2	0	0	0	0	0	0	0	0	0	109	0	55	36	1	4	0	0	0	0	0	0	96	1	123	74	3	4	0	0	0	0	0	0	0	0	205		
	10:00	0	80	36	6	3	0	0	0	0	0	0	0	0	125	0	57	38	1	2	0	0	0	0	0	0	98	0	137	74	7	5	0	0	0	0	0	0	0	0	223		
	11:00	1	77	25	0	2	0	0	0	0	0	0	0	0	105	1	75	41	3	0	0	0	0	0	0	0	120	2	152	66	3	2	0	0	0	0	0	0	0	0	225		
	12:00	1	72	34	1	0	0	0	0	0	0	0	0	0	108	1	67	34	2	0	0	0	0	0	0	0	104	2	139	68	3	0	0	0	0	0	0	0	0	0	212		
	13:00	0	96	36	6	2	0	0	0	0	0	0	0	0	140	0	110	52	2	3	0	0	0	0	0	0	167	0	206	88	8	5	0	0	0	0	0	0	0	0	307		
	14:00	0	74	42	5	4	0	0	0	0	0	0	0	0	125	0	86	40	2	0	0	0	0	0	0	0	128	0	160	82	7	4	0	0	0	0	0	0	0	0	253		
	15:00	0	112	56	3	3	0	0	0	0	0	0	0	0	174	3	101	48	5	4	0	0	0	0	0	0	161	3	213	104	8	7	0	0	0	0	0	0	0	0	335		
	16:00	0	114	51	0	1	0	0	0	0	0	0	0	0	166	1	97	32	5	2	0	0	0	0	0	0	137	1	211	83	5	3	0	0	0	0	0	0	0	0	303		
	17:00	0	84	23	0	1	0	0	0	0	0	0	0	0	108	0	109	34	4	0	0	0	0	0	0	0	147	0	193	57	4	1	0	0	0	0	0	0	0	0	255		
	18:00	1	43	19	1	0	0	0	0	0	0	0	0	0	64	3	64	27	0	1	0	0	0	0	0	0	95	4	107	46	1	1	0	0	0	0	0	0	0	0	159		
	19:00	0	32	8	0	0	0	0	0	0	0	0	0	0	40	0	48	16	0	0	0	0	0	0	0	0	64	0	80	24	0	0	0	0	0	0	0	0	0	0	104		
	20:00	0	19	9	0	1	0	0	0	0	0	0	0	0	29	0	38	17	0	0	0	0	0	0	0	0	55	0	57	26	0	1	0	0	0	0	0	0	0	0	84		
	21:00	0	18	6	0	0	0	0	0	0	0	0	0	0	24	0	28	5	0	0	0	0	0	0	0	0	33	0	46	11	0	0	0	0	0	0	0	0	0	0	57		
	22:00	0	13	2	0	0	0	0	0	0	0	0	0	0	15	0	15	3	0	1	0	0	0	0	0	0	19	0	28	5	0	1	0	0	0	0	0	0	0	0	34		
	23:00	0	6	5	0	0	0	0	0	0	0	0	0	0	11	0	7	3	0	0	0	0	0	0	0	0	10	0	13	8	0	0	0	0	0	0	0	0	0	0	21		
	Totals	9	1,249	516	36	19	0	0	0	0	0	0	0	0	1,829	12	1,269	541	40	22	0	0	0	0	0	0	1,884	21	2,518	1,057	76	41	0	0	0	0	0	0	0	0	0	3,713	
	% of Totals	0%	68%	28%	2%	1%									100%	1%	67%	29%	2%	1%							100%	1%	68%	28%	2%	1%											100%

FHWA	CLASSIFICATION DEFINITIONS						
	 #1 Motorcycles	 #2 Passenger Cars	 #3 2-Axle, 4-Tire Single Unit	 #4 Buses	 #5 2-Axle, 6-Tire Single Units	 #6 3-Axle Single Units	 #7 >=4-Axle Single Units
FHWA	 #8 <=4-Axle Single Trailers	 #9 5-Axle Single Trailers	 #10 >=6-Axle Single Trailers	 #11 <=5-Axle Multi-Trailers	 #12 6-Axle Multi-Trailers	ANY 7 OR MORE AXLE #13 >=7-Axle Multi-Trailers	

STATISTICS	00:00 - 12:00	7	566	225	20	7	0	0	0	0	0	0	0	0	825	4	499	230	20	11	0	0	0	0	0	0	0	764	11	1065	455	40	18	0	0	0	0	0	0	0	0	1589	
	%	0	31%	12%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	45%	0	26%	12%	1%	1%	0%	0%	0%	0%	0%	0%	0%	41%	0	29%	12%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	730	
	Peak Hour	5:45	7:30	7:30	6:15	10:15	0:00	0:00	0:00	0:00	0:00	0:00	0:00	0:00	7:30	8:00	7:30	7:30	7:45	8:45	0:00	0:00	0:00	0:00	0:00	0:00	0:00	7:30	8:00	7:30	7:30	7:45	8:45	0:00	0:00	0:00	0:00	0:00	0:00	0:00	0:00	0:00	7:30
	Peak Volume	2	188	51	10	4	0	0	0	0	0	0	0	0	241	2	167	54	14	5	0	0	0	0	0	0	0	234	4	355	105	15	6	0	0	0	0	0	0	0	0	475	
	12:00 - 24:00	2	683	291	16	12	0	0	0	0	0	0	0	0	1004	8	770	311	20	11	0	0	0	0	0	0	0	1120	10	1453	602	36	23	0	0	0	0	0	0	0	2124		
	%	0	37%	16%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	55%	0	41%	17%	1%	1%	0%	0%	0%	0%	0%	0%	0%	59%	0	39%	16%	1%	1%	0%	0%	0%	0%	0%	0%	0%	57%		
	Peak Hour	12:00	15:45	15:30	13:00	13:45	12:00	12:00	12:00	12:00	12:00	12:00	12:00	12:00	15:30	15:15	16:45	13:00	16:45	14:30	12:00	12:00	12:00	12:00	12:00	12:00	13:00	15:15	15:30	15:30	14:45	14:30	12:00	12:00	12:00	12:00	12:00	12:00	12:00	12:00	12:00	15:30	
	Peak Volume	1	132	59	6	4	0	0	0	0	0	0	0	0	193	4	122	52	6	4	0	0	0	0	0	0	0	167	4	237	108	10	8	0	0	0	0	0	0	0	357		
	07:00 - 09:00	3	291	90	2	2	0	0	0	0	0	0	0	0	388	2	239	82	15	5	0	0	0	0	0	0	0	0	343	5	530	172	17	7	0	0	0	0	0	0	0	731	
	%	0	16%	5%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	21%	0	13%	4%	1%	0%	0%	0%	0%	0%	0%	0%	0%	18%	0	14%	5%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	20%	
Peak Hour	8:00	7:30	7:30	7:00	8:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:30	8:00	7:30	7:30	7:45	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:30	8:00	7:30	7:30	7:45	8:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:30
Peak Volume	2	188	51	1	2	0	0	0	0	0	0	0	0	241	2	167	54	14	3	0	0	0	0	0	0	0	0	234	4	355	105	15	4	0	0	0	0	0	0	0	0	475	
16:00 - 18:00	0	198	74	0	2	0	0	0	0	0	0	0	0	284	0	206	16	9	2	0	0	0	0	0	0	0	0	284	1	404	150	14	0	0	0	0	0	0	0	0	580		
%	0	11%	4%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	15%	0	11%	4%	0%	0%	0%	0%	0%	0%	0%	0%	0%	15%	0	11%	4%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	15%	
Peak Hour	16:00	16:00	16:00	16:00	16:15	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:45	16:45	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:15	16:00	16:45	16:15	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:15	
Peak Volume	0	114	51	0	2	0	0	0	0	0	0	0	0	166	1	122	37	6	2	0	0	0	0	0	0	0	166	1	223	83	6	4	0	0	0	0	0	0	0	0	313		

CLASSIFICATION

N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49

Day: Tuesday
Date: 5/20/2025

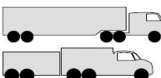
City: Sonora
Project #: CA25_090067_001

	Time	EASTBOUND													Total	WESTBOUND													Total	TOTALS													Total
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	
15-MINUTE BREAKDOWN	0:00	0	1	0	0	0	0	0	0	0	0	0	0	1	0	3	0	0	0	0	0	0	0	0	0	0	3	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	4
	0:15	0	1	0	0	0	0	0	0	0	0	0	0	1	0	3	0	0	0	0	0	0	0	0	0	0	3	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4	
	0:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2	0	1	1	0	0	0	0	0	0	0	0	0	0	2		
	0:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1		
	1:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	2		
	1:15	0	1	0	0	0	0	0	0	0	0	0	0	1	0	3	1	0	0	0	0	0	0	0	0	0	4	0	4	1	0	0	0	0	0	0	0	0	0	0	5		
	1:30	0	1	1	0	0	0	0	0	0	0	0	0	2	0	4	0	0	0	0	0	0	0	0	0	0	4	0	5	1	0	0	0	0	0	0	0	0	0	0	6		
	1:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	2:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3	
	2:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2	
	2:30	0	0	2	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2	
	2:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	3:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	3:15	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2	
	3:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	3:45	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	
	4:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	
	4:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	4:30	0	2	2	0	0	0	0	0	0	0	0	0	4	0	3	0	0	0	0	0	0	0	0	0	0	0	3	0	5	2	0	0	0	0	0	0	0	0	0	0	7	
	4:45	0	3	1	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	1	0	0	0	0	0	0	0	0	0	0	4	
	5:00	0	1	1	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2	
	5:15	0	3	1	0	0	0	0	0	0	0	0	0	4	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	5	2	0	0	0	0	0	0	0	0	0	0	7	
	5:30	0	1	4	1	0	0	0	0	0	0	0	0	6	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	3	5	1	0	0	0	0	0	0	0	0	0	9	
	5:45	0	1	5	0	0	0	0	0	0	0	0	0	6	0	4	3	0	0	0	0	0	0	0	0	0	0	7	0	5	8	0	0	0	0	0	0	0	0	0	13		
	6:00	1	3	6	0	0	0	0	0	0	0	0	0	10	1	5	2	0	0	0	0	0	0	0	0	0	0	8	2	8	8	0	0	0	0	0	0	0	0	0	0	18	
	6:15	0	5	3	2	0	0	0	0	0	0	0	0	10	0	1	1	0	0	0	0	0	0	0	0	0	0	2	0	6	4	2	0	0	0	0	0	0	0	0	0	12	
	6:30	1	13	4	5	0	0	0	0	0	0	0	0	23	0	12	8	0	0	0	0	0	0	0	0	0	0	20	1	25	12	5	0	0	0	0	0	0	0	0	43		
	6:45	0	12	6	2	0	0	0	0	0	0	0	0	20	0	25	10	0	0	0	0	0	0	0	0	0	0	35	0	37	16	2	0	0	0	0	0	0	0	0	0	55	
	7:00	1	24	9	1	0	0	0	0	0	0	0	0	35	0	19	9	0	0	0	0	0	0	0	0	0	0	28	1	43	18	1	0	0	0	0	0	0	0	0	63		
	7:15	0	20	10	0	0	0	0	0	0	0	0	0	30	0	17	8	0	1	0	0	0	0	0	0	0	0	26	0	37	18	0	1	0	0	0	0	0	0	0	56		
	7:30	0	41	14	0	0	0	0	0	0	0	0	0	55	0	35	10	1	2	0	0	0	0	0	0	0	0	48	0	76	24	1	2	0	0	0	0	0	0	0	103		
	7:45	0	36	15	0	0	0	0	0	0	0	0	0	51	0	28	8	0	0	0	0	0	0	0	0	0	0	36	0	64	23	0	0	0	0	0	0	0	0	0	87		
	8:00	1	51	8	0	0	0	0	0	0	0	0	0	60	0	54	17	1	0	0	0	0	0	0	0	0	0	72	1	105	25	1	0	0	0	0	0	0	0	0	132		
	8:15	0	60	14	1	0	0	0	0	0	0	0	0	75	0	50	19	9	0	0	0	0	0	0	0	0	0	78	0	110	33	10	0	0	0	0	0	0	0	0	153		
	8:30	0	37	14	0	1	0	0	0	0	0	0	0	52	0	22	4	4	0	0	0	0	0	0	0	0	0	30	0	59	18	4	1	0	0	0	0	0	0	0	82		
	8:45	1	22	6	0	1	0	0	0	0	0	0	0	30	2	14	7	0	2	0	0	0	0	0	0	0	0	25	3	36	13	0	3	0	0	0	0	0	0	0	55		
	9:00	1	16	6	0	0	0	0	0	0	0	0	0	23	0	16	9	1	0	0	0	0	0	0	0	0	0	26	1	32	15	1	0	0	0	0	0	0	0	0	49		
	9:15	0	14	8	1	0	0	0	0	0	0	0	0	23	0	12	15	0	1	0	0	0	0	0	0	0	0	28	0	26	23	1	1	0	0	0	0	0	0	0	51		
	9:30	0	20	12	0	0	0	0	0	0	0	0	0	32	0	13	9	0	2	0	0	0	0	0	0	0	0	24	0	33	21	0	2	0	0	0	0	0	0	0	56		
	9:45	0	18	12	1	0	0	0	0	0	0	0	0	31	0	14	3	0	1	0	0	0	0	0	0	0	0	18	0	32	15	1	1	0	0	0	0	0	0	0	49		
	10:00	0	17	6	1	0	0	0	0	0	0	0	0	24	0	13	8	1	0	0	0	0	0	0	0	0	0	22	0	30	14	2	0	0	0	0	0	0	0	0	46		
10:15	0	14	11	0	1	0	0	0	0	0	0	0	26	0	11	11	0	0	0	0	0	0	0	0	0	0	22	0	25	22	0	1	0	0	0	0	0	0	0	48			
10:30	0	35	9	5	2	0	0	0	0	0	0	0	51	0	18	4	0	1	0	0	0	0	0	0	0	0	23	0	53	13	5	3	0	0	0	0	0	0	0	74			
10																																											

[illegible]

Day: Wednesday
Date: 5/21/2025

HOURLY BREAKDOWN	Time	EASTBOUND													Total	WESTBOUND													Total	TOTALS													Total
	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total			
	0:00	0	3	0	0	0	0	0	0	0	0	0	0	0	3	0	6	1	0	0	0	0	0	0	0	0	0	7	0	9	1	0	0	0	0	0	0	0	0	0	0	10	
	1:00	0	6	1	0	0	0	0	0	0	0	0	0	0	7	0	7	2	0	0	0	0	0	0	0	0	0	9	0	13	3	0	0	0	0	0	0	0	0	0	16		
	2:00	0	4	1	0	0	0	0	0	0	0	0	0	0	5	0	3	1	0	0	0	0	0	0	0	0	0	4	0	7	2	0	0	0	0	0	0	0	0	0	9		
	3:00	0	4	2	0	0	0	0	0	0	0	0	0	0	6	0	1	1	0	0	0	0	0	0	0	0	0	2	0	5	3	0	0	0	0	0	0	0	0	0	8		
	4:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4	0	4	2	0	0	0	0	0	0	0	0	0	6	0	8	2	0	0	0	0	0	0	0	0	0	10		
	5:00	0	12	8	2	0	0	0	0	0	0	0	0	0	22	0	7	9	0	0	0	0	0	0	0	0	0	16	0	19	17	2	0	0	0	0	0	0	0	0	0	38	
	6:00	3	39	18	8	0	2	0	0	0	0	0	0	0	70	3	47	14	0	0	1	0	0	0	0	0	0	65	6	86	32	8	0	3	0	0	0	0	0	0	135		
	7:00	0	99	29	2	4	0	0	0	0	0	0	0	0	134	0	61	23	6	5	0	0	1	0	0	0	0	96	0	160	52	8	9	0	0	1	0	0	0	0	230		
	8:00	1	86	47	6	4	1	0	0	0	0	0	0	0	145	0	83	53	5	2	1	0	1	0	0	0	0	145	1	169	100	11	6	2	0	1	0	0	0	0	290		
	9:00	0	159	59	1	2	2	0	0	0	0	0	0	0	223	0	140	52	10	3	0	0	0	0	0	0	0	205	0	299	111	11	5	2	0	0	0	0	0	0	428		
	10:00	0	58	37	5	3	0	0	1	0	0	0	0	0	104	0	67	35	2	1	0	0	1	0	0	0	0	106	0	125	72	7	4	0	0	2	0	0	0	0	210		
	11:00	1	73	35	1	1	0	0	0	0	0	0	0	0	111	0	67	34	4	2	0	0	0	0	0	0	0	107	1	140	69	5	3	0	0	0	0	0	0	0	218		
	12:00	0	95	45	1	0	0	0	0	0	0	0	0	0	141	0	66	36	2	1	0	0	1	0	0	0	0	106	0	161	81	3	1	0	0	1	0	0	0	0	247		
	13:00	0	81	39	7	0	0	0	1	0	0	0	0	0	128	0	83	45	1	0	0	0	0	0	0	0	0	129	0	164	84	8	0	0	0	1	0	0	0	0	257		
	14:00	0	81	44	7	7	0	0	2	0	0	0	0	0	141	0	91	39	3	3	0	0	1	0	0	0	0	137	0	172	83	10	10	0	0	3	0	0	0	0	278		
	15:00	0	130	44	2	0	0	0	0	0	0	0	0	0	176	3	120	57	4	1	0	0	0	0	0	0	0	185	3	250	101	6	1	0	0	0	0	0	0	0	361		
	16:00	0	100	38	0	0	1	0	0	0	0	0	0	0	139	1	115	42	5	0	1	0	0	0	0	0	0	164	1	215	80	5	0	2	0	0	0	0	0	0	303		
	17:00	0	94	36	0	2	0	0	0	0	0	0	0	0	132	1	109	44	3	1	0	0	0	0	0	0	0	158	1	203	80	3	3	0	0	0	0	0	0	0	290		
18:00	1	74	37	1	0	0	0	1	0	0	0	0	0	114	2	69	29	0	1	0	0	0	0	0	0	0	101	3	143	66	1	1	0	0	1	0	0	0	0	215			
19:00	1	45	15	0	1	0	0	1	0	0	0	0	0	63	0	55	22	0	0	0	0	0	0	0	0	0	77	1	100	37	0	1	0	0	1	0	0	0	0	140			
20:00	0	26	9	0	0	0	0	0	0	0	0	0	0	35	1	44	19	0	1	0	0	0	0	0	0	0	65	1	70	28	0	1	0	0	0	0	0	0	0	100			
21:00	0	15	4	0	0	0	0	0	0	0	0	0	0	19	0	20	3	0	0	0	0	0	0	0	0	0	23	0	35	7	0	0	0	0	0	0	0	0	0	42			
22:00	0	5	1	0	0	0	0	0	0	0	0	0	0	6	0	8	3	0	1	0	0	0	0	0	0	0	12	0	13	4	0	1	0	0	0	0	0	0	0	18			
23:00	0	11	5	0	0	0	0	0	0	0	0	0	0	16	0	10	3	0	0	0	0	0	0	0	0	0	13	0	21	8	0	0	0	0	0	0	0	0	0	29			
Totals		7	1,304	554	43	24	6	0	6	0	0	0	0	1,944	11	1,283	569	45	22	3	0	5	0	0	0	0	1,938	18	2,587	1,123	88	46	9	0	11	0	0	0	0	3,882			
% of Totals		0%	67%	28%	2%	1%	0%	0%	0%	0%	0%	0%	0%	100%	1%	66%	29%	2%	1%	0%	0%	0%	0%	0%	0%	0%	100%	0%	67%	29%	2%	1%	0%	0%	11%	0%	0%	0%	0%	100%			

FHWA	CLASSIFICATION DEFINITIONS						
	 #1 Motorcycles	 #2 Passenger Cars	 #3 2-Axle, 4-Tire Single Unit	 #4 Buses	 #5 2-Axle, 6-Tire Single Units	 #6 3-Axle Single Units	 #7 >=4-Axle Single Units
	 #8 <=4-Axle Single Trailers	 #9 5-Axle Single Trailers	 #10 >>6-Axle Single Trailers	 #11 <=5-Axle Multi-Trailers	 #12 6-Axle Multi-Trailers	ANY 7 OR MORE AXLE #13 >>7-Axle Multi-Trailers	

[illegible]

City: Sonora
Project #: CA25_090067_001

[illegible]

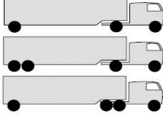
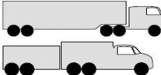
Time		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total	
15-MINUTE BREAKDOWN	12:00	0	25	17	0	0	0	0	0	0	0	0	0	0	42	0	17	7	0	0	0	0	1	0	0	0	0	0	25	0	42	24	0	0	0	0	1	0	0	0	0	0	0	67
	12:15	0	14	5	1	0	0	0	0	0	0	0	0	0	20	0	17	8	1	0	0	0	0	0	0	0	0	0	26	0	31	13	2	0	0	0	0	0	0	0	0	0	46	
	12:30	0	35	13	0	0	0	0	0	0	0	0	0	0	48	0	14	11	0	1	0	0	0	0	0	0	0	0	26	0	49	24	0	1	0	0	0	0	0	0	0	0	74	
	12:45	0	21	10	0	0	0	0	0	0	0	0	0	0	31	0	18	10	1	0	0	0	0	0	0	0	0	0	29	0	39	20	1	0	0	0	0	0	0	0	0	0	60	
	13:00	0	20	8	1	0	0	0	0	0	0	0	0	0	29	0	22	12	1	0	0	0	0	0	0	0	0	0	35	0	42	20	2	0	0	0	0	0	0	0	0	0	64	
	13:15	0	24	8	4	0	0	0	0	0	0	0	0	0	36	0	18	12	0	0	0	0	0	0	0	0	0	0	30	0	42	20	4	0	0	0	0	0	0	0	0	0	66	
	13:30	0	20	13	0	0	0	0	1	0	0	0	0	0	34	0	17	9	0	0	0	0	0	0	0	0	0	0	26	0	37	22	0	0	0	1	0	0	0	0	0	60		
	13:45	0	17	10	2	0	0	0	0	0	0	0	0	0	29	0	26	12	0	0	0	0	0	0	0	0	0	0	38	0	43	22	2	0	0	0	0	0	0	0	0	0	67	
	14:00	0	19	12	1	1	0	0	0	1	0	0	0	0	34	0	18	13	2	1	0	0	0	0	0	0	0	0	34	0	37	25	3	2	0	0	1	0	0	0	0	0	68	
	14:15	0	25	11	2	3	0	0	0	0	0	0	0	0	41	0	23	11	1	1	0	0	1	0	0	0	0	0	37	0	48	22	3	4	0	0	1	0	0	0	0	0	78	
	14:30	0	13	12	0	1	0	0	0	1	0	0	0	0	27	0	16	9	0	0	1	0	0	0	0	0	0	0	26	0	29	21	0	2	0	0	1	0	0	0	0	0	53	
	14:45	0	24	9	4	2	0	0	0	0	0	0	0	0	39	0	34	6	0	0	0	0	0	0	0	0	0	0	40	0	58	15	4	2	0	0	0	0	0	0	0	0	79	
	15:00	0	17	6	0	0	0	0	0	0	0	0	0	0	23	1	19	6	1	0	0	0	0	0	0	0	0	0	27	1	36	12	1	0	0	0	0	0	0	0	0	0	50	
	15:15	0	24	10	1	0	0	0	0	0	0	0	0	0	35	0	37	15	2	0	0	0	0	0	0	0	0	0	54	0	61	25	3	0	0	0	0							

CLASSIFICATION

N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49

Day: Thursday
Date: 5/22/2025City: Sonora
Project #: CA25_090067_001

HOURLY BREAKDOWN	Time	EASTBOUND													Total	WESTBOUND													Total	TOTALS													Total
	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total			
	0:00	0	2	3	0	0	0	0	0	0	0	0	0	0	5	0	3	1	0	0	0	0	0	0	0	0	4	0	5	4	0	0	0	0	0	0	0	0	0	0	0	9	
	1:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4	0	6	1	0	0	0	0	0	0	0	0	7	0	10	1	0	0	0	0	0	0	0	0	0	0	11		
	2:00	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	3	1	0	0	0	0	0	0	0	0	4	0	3	2	0	0	0	0	0	0	0	0	0	0	5		
	3:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4	0	0	2	0	1	0	0	0	0	0	0	3	0	4	2	0	1	0	0	0	0	0	0	0	0	7		
	4:00	0	5	3	0	1	0	0	0	0	0	0	0	0	9	0	2	4	0	0	0	0	0	0	0	0	6	0	7	7	0	1	0	0	0	0	0	0	0	15			
	5:00	0	10	10	1	1	1	0	0	0	0	0	0	0	23	0	8	3	0	1	1	0	0	0	0	0	13	0	18	13	1	2	2	0	0	0	0	0	0	36			
	6:00	1	31	19	9	0	0	0	0	0	0	0	0	0	60	0	44	23	0	0	0	0	0	0	0	0	67	1	75	42	9	0	0	0	0	0	0	0	0	127			
	7:00	0	119	50	1	1	0	0	0	0	0	0	0	0	171	1	99	36	3	1	0	0	0	0	0	0	140	1	218	86	4	2	0	0	0	0	0	0	0	311			
	8:00	1	153	41	1	2	0	0	0	0	0	0	0	0	198	0	145	58	8	7	0	0	2	0	0	0	0	220	1	298	99	9	9	0	0	2	0	0	0	0	418		
	9:00	0	68	46	1	1	0	0	0	0	0	0	0	0	116	0	54	34	1	3	0	0	0	0	0	0	0	92	0	122	80	2	4	0	0	0	0	0	0	0	208		
	10:00	1	77	34	5	2	0	0	1	0	0	0	0	0	120	1	61	32	3	0	0	0	1	0	0	0	0	98	2	138	66	8	2	0	0	2	0	0	0	0	218		
	11:00	0	79	35	4	3	0	0	0	0	0	0	0	0	121	1	61	41	3	1	0	0	0	0	0	0	0	107	1	140	76	7	4	0	0	0	0	0	0	0	228		
	12:00	1	76	41	5	5	1	0	0	0	0	0	0	0	129	1	68	36	2	3	0	0	0	0	0	0	0	110	2	144	77	7	8	1	0	0	0	0	0	0	239		
	13:00	0	84	37	2	1	0	0	1	0	0	0	0	0	125	0	93	43	3	1	0	0	1	0	0	0	0	141	0	177	80	5	2	0	0	2	0	0	0	0	266		
	14:00	0	72	30	6	1	0	0	0	0	0	0	0	0	109	0	92	34	5	2	0	0	0	0	0	0	0	133	0	164	64	11	3	0	0	0	0	0	0	0	242		
	15:00	0	122	48	5	4	0	0	0	0	0	0	0	0	179	1	118	39	4	0	0	0	0	0	0	0	0	162	1	240	87	9	4	0	0	0	0	0	0	0	341		
	16:00	0	108	41	0	1	0	0	0	0	0	0	0	0	150	0	99	32	5	1	0	0	0	0	0	0	0	137	0	207	73	5	2	0	0	0	0	0	0	0	287		
	17:00	2	79	35	0	1	0	0	0	0	0	0	0	0	117	1	142	38	3	0	0	0	0	0	0	0	0	184	3	221	73	3	1	0	0	0	0	0	0	0	301		
	18:00	1	66	22	1	0	0	0	0	0	0	0	0	0	90	1	69	19	0	0	0	0	0	0	0	0	0	89	2	135	41	1	0	0	0	0	0	0	0	0	179		
	19:00	0	59	30	0	1	0	0	0	0	0	0	0	0	90	0	46	19	0	0	0	0	0	0	0	0	0	65	0	105	49	0	1	0	0	0	0	0	0	0	155		
	20:00	0	25	8	0	0	0	0	0	0	0	0	0	0	33	0	57	20	0	1	0	0	0	0	0	0	0	78	0	82	28	0	1	0	0	0	0	0	0	0	111		
	21:00	0	16	3	0	0	0	0	0	0	0	0	0	0	19	0	24	6	0	0	0	0	0	0	0	0	0	30	0	40	9	0	0	0	0	0	0	0	0	0	49		
22:00	0	15	3	0	0	0	0	0	0	0	0	0	0	18	0	10	3	0	1	0	0	0	0	0	0	0	14	0	25	6	0	1	0	0	0	0	0	0	0	32			
23:00	0	6	5	0	0	0	0	0	0	0	0	0	0	11	0	10	1	0	0	0	0	0	0	0	0	0	11	0	16	6	0	0	0	0	0	0	0	0	0	22			
Totals	7	1,280	545	41	25	2	0	2	0	0	0	0	0	1,902	7	1,314	526	40	23	1	0	4	0	0	0	0	1,915	14	2,594	1,071	81	48	3	0	6	0	0	0	0	0	3,517		
% of Totals	0%	67%	29%	2%	1%	0%	0%	0%	0%	0%	0%	0%	0%	100%	0%	69%	27%	2%	1%	0%	0%	0%	0%	0%	0%	100%	0%	68%	28%	2%	1%	0%	0%	0%	0%	0%	0%	0%	0%	100%			

FHWA	CLASSIFICATION DEFINITIONS						
	#1 Motorcycles	#2 Passenger Cars	#3 2-Axle, 4-Tire Single Unit	#4 Buses	#5 2-Axle, 6-Tire Single Units	#6 3-Axle Single Units	#7 >=4-Axle Single Units
							
	#8 <=4-Axle Single Trailers	#9 5-Axle Single Trailers	#10 >=6-Axle Single Trailers	#11 <=5-Axle Multi-Trailers	#12 6-Axle Multi-Trailers	ANY 7 OR MORE AXLE	
							

STATISTICS	00:00 - 12:00	%	3	552	242	22	11	1	0	1	0	0	0	0	0	832	3	486	236	18	14	1	0	3	0	0	0	0	0	761	6	1038	478	40	25	2	0	4	0	0	0	0	1593	
		%	0%	29%	13%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	44%	0%	25%	12%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	40%	0%	27%	13%	1%	1%	0%	0%	0%	0%	0%	0%	0%	42%	
	Peak Hour	5:15	7:45	8:45	6:15	11:30	5:00	0:00	10:00	0:00	0:00	0:00	0:00	0:00	0:00	7:45	11:30	7:30	7:45	8:15	7:45	4:45	0:00	7:45	0:00	0:00	0:00	0:00	0:00	7:30	9:15	7:30	7:15	8:15	8:15	5:00	0:00	7:45	0:00	0:00	0:00	0:00	0:00	7:30
	Peak Volume	1	177	52	10	6	1	0	1	0	0	0	0	0	0	220	2	169	58	9	7	1	0	2	0	0	0	0	0	238	2	339	99	11	10	2	0	2	0	0	0	0	453	
	12:00 - 24:00	%	4	728	303	19	14	1	0	1	0	0	0	0	0	1070	4	828	290	22	9	0	0	1	0	0	0	0	0	1154	8	1556	593	41	23	1	0	2	0	0	0	0	2224	
		%	0%	38%	15%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	56%	0%	38%	15%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	43%	0%	38%	15%	1%	0%	0%	0%	0%	0%	0%	0%	60%		
	Peak Hour	17:15	15:15	15:15	14:45	12:00	12:00	12:00	12:45	12:00	12:00	12:00	12:00	12:00	12:00	15:15	17:15	17:00	13:15	16:15	12:15	12:00	12:00	12:00	12:00	12:00	12:00	12:00	12:00	17:00	17:15	15:15	15:15	14:45	12:00	12:00	12:45	12:00	12:00	12:00	12:00	12:00	15:15	
	Peak Volume	3	137	50	9	5	1	0	1	0	0	0	0	0	0	194	2	142	45	6	4	0	0	1	0	0	0	0	0	184	5	257	93	13	8	1	0	2	0	0	0	0	363	
	07:00 - 09:00	%	1	272	91	2	3	0	0	0	0	0	0	0	0	0	369	1	244	94	11	8	0	0	2	0	0	0	0	0	360	2	516	185	13	11	0	0	2	0	0	0	0	729
	Peak Hour	8:00	7:45	7:00	7:00	7:45	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	19%	7:45	7:00	7:30	7:45	7:30	7:45	7:00	7:45	7:00	7:00	7:00	7:00	7:00	19%	7:00	7:30	7:15	7:30	7:45	7:00	7:45	7:00	7:00	7:00	7:00	7:00	7:30	19%
16:00 - 18:00	%	1	177	50	1	2	0	0	0	0	0	0	0	0	0	220	1	169	58	8	7	0	0	2	0	0	0	0	0	238	1	339	99	9	9	0	0	2	0	0	0	0	453	
Peak Hour	17:00	16:00	16:30	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16%	17:00	17:00	17:00	16:15	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16%	17:00	17:00	16:30	16:15	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	17:00	
Peak Volume	2	108	44	0	1	0	0	0	0	0	0	0	0	0	150	1	142	38	6	1	0	0	0	0	0	0	0	0	184	3	221	76	6	2	0	0	0	0	0	0	0	301		

CLASSIFICATION

N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49

Day: Thursday
Date: 5/22/2025

City: Sonora
Project #: CA25_090067_001

	Time	EASTBOUND													Total	WESTBOUND													Total	TOTALS													Total	
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		
15-MINUTE BREAKDOWN	0:00	0	1	1	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	2	0	3	1	0	0	0	0	0	0	0	0	0	0	0	0	4	
	0:15	0	1	2	0	0	0	0	0	0	0	0	0	3	0	0	1	0	0	0	0	0	0	0	0	0	1	0	1	3	0	0	0	0	0	0	0	0	0	0	0	4		
	0:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	0:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	1			
	1:00	0	1	0	0	0	0	0	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	2	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3		
	1:15	0	1	0	0	0	0	0	0	0	0	0	0	1	0	2	0	0	0	0	0	0	0	0	0	0	2	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3		
	1:30	0	2	0	0	0	0	0	0	0	0	0	0	2	0	3	0	0	0	0	0	0	0	0	0	0	3	0	5	0	0	0	0	0	0	0	0	0	0	0	5			
	1:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	2:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3		
	2:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	2:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	2:45	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	
	3:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	
	3:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	3:30	0	2	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	
	3:45	0	2	0	0	0	0	0	0	0	0	0	0	2	0	0	1	0	1	0	0	0	0	0	0	0	0	2	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4	
	4:00	0	1	0	0	1	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	2	
	4:15	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	2	0	0	0	0	0	0	0	0	0	0	2	0	0	3	0	0	0	0	0	0	0	0	0	0	0	3	
	4:30	0	1	0	0	0	0	0	0	0	0	0	0	1	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4	
	4:45	0	3	2	0	0	0	0	0	0	0	0	0	5	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	3	3	0	0	0	0	0	0	0	0	0	0	0	6	
	5:00	0	3	0	0	0	0	0	0	0	0	0	0	3	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6	
	5:15	0	0	3	1	0	0	0	0	0	0	0	0	4	0	0	1	0	1	0	0	0	0	0	0	0	0	2	0	0	4	1	1	0	0	0	0	0	0	0	0	6		
	5:30	0	3	5	0	0	0	0	0	0	0	0	0	8	0	1	0	0	0	1	0	0	0	0	0	0	0	2	0	4	5	0	0	1	0	0	0	0	0	0	0	0	10	
	5:45	0	4	2	0	1	1	0	0	0	0	0	0	8	0	5	1	0	0	0	0	0	0	0	0	0	0	6	0	9	3	0	1	1	0	0	0	0	0	0	0	0	14	
	6:00	1	3	2	0	0	0	0	0	0	0	0	0	6	0	5	3	0	0	0	0	0	0	0	0	0	0	8	1	8	5	0	0	0	0	0	0	0	0	0	0	0	14	
	6:15	0	8	1	2	0	0	0	0	0	0	0	0	11	0	5	5	0	0	0	0	0	0	0	0	0	0	10	0	13	6	2	0	0	0	0	0	0	0	0	0	0	21	
	6:30	0	11	9	3	0	0	0	0	0	0	0	0	23	0	5	8	0	0	0	0	0	0	0	0	0	0	13	0	16	17	3	0	0	0	0	0	0	0	0	0	0	0	36
	6:45	0	9	7	4	0	0	0	0	0	0	0	0	20	0	29	7	0	0	0	0	0	0	0	0	0	0	36	0	38	14	4	0	0	0	0	0	0	0	0	0	0	0	56
	7:00	0	20	11	1	1	0	0	0	0	0	0	0	33	1	12	8	0	1	0	0	0	0	0	0	0	0	22	1	32	19	1	2	0	0	0	0	0	0	0	0	0	0	55
	7:15	0	15	17	0	0	0	0	0	0	0	0	0	32	0	16	11	0	0	0	0	0	0	0	0	0	0	27	0	31	28	0	0	0	0	0	0	0	0	0	0	0	0	59
	7:30	0	40	11	0	0	0	0	0	0	0	0	0	51	0	39	7	3	0	0	0	0	0	0	0	0	0	49	0	79	18	3	0	0	0	0	0	0	0	0	0	0	0	100
	7:45	0	44	11	0	0	0	0	0	0	0	0	0	55	0	32	10	0	0	0	0	0	0	0	0	0	0	42	0	76	21	0	0	0	0	0	0	0	0	0	0	0	0	97
	8:00	0	38	11	0	0	0	0	0	0	0	0	0	49	0	46	21	0	1	0	0	0	0	0	0	0	0	68	0	84	32	0	1	0	0	0	0	0	0	0	0	0	0	117
	8:15	0	48	10	1	1	0	0	0	0	0	0	0	60	0	52	18	5	3	0	0	1	0	0	0	0	0	79	0	100	28	6	4	0	0	1	0	0	0	0	0	0	139	
	8:30	0	47	8	0	1	0	0	0	0	0	0	0	56	0	28	9	3	3	0	0	1	0	0	0	0	0	44	0	75	17	3	4	0	0	1	0	0	0	0	0	0	100	
	8:45	1	20	12	0	0	0	0	0	0	0	0	0	33	0	19	10	0	0	0	0	0	0	0	0	0	29	1	39	22	0	0	0	0	0	0	0	0	0	0	0	0	62	
	9:00	0	19	21	1	1	0	0	0	0	0	0	0	42	0	13	8	1	1	0	0	0	0	0	0	0	0	23	0	32	29	2	2	0	0	0	0	0	0	0	0	0	0	65
	9:15	0	17	8	0	0	0	0	0	0	0	0	0	25	0	14	9	0	0	0	0	0	0	0	0	0	0	23	0	31	17	0	0	0	0	0	0	0	0	0	0	0	48	
	9:30	0	14	11	0	0	0	0	0	0	0	0	0	25	0	17	8	0	0	0	0	0	0	0	0	0	0	25	0	31	19	0	0	0	0	0	0	0	0	0	0	0	0	50
	9:45	0	18	6	0	0	0	0	0	0	0	0	0	24	0	10	9	0	2	0	0	0	0	0	0	0	0	21	0	28	15	0	2	0	0	0	0	0	0	0	0	0	0	45
10:00	1	9	7	1	1	0	0	0	0	0	0	0	19	1	8	6	0	0	0	0	0	0	0	0	0	0	15	2	17	13	1	1	0	0	0	0	0	0	0	0	0	0	34	
10:15	0	19	11	0	1	0	0	0	0	0	0	0	31	0	17	9	0	0	0	0	0	0	0	0	0	0	26																	

CLASSIFICATION

N Washington St/Shaws Flat Rd W/O Golden Chain Hwy/N Washington St/SR 49

Day: Thursday
Date: 5/22/2025

City: Sonora
Project #: CA25_090067_001

	Time	EASTBOUND													Total	WESTBOUND													Total	TOTALS													Total
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	
15-MINUTE BREAKDOWN	12:00	0	16	6	2	2	0	0	0	0	0	0	0	26	0	16	8	0	0	0	0	0	0	0	0	0	24	0	32	14	2	2	0	0	0	0	0	0	0	0	0	50	
	12:15	0	14	9	2	1	0	0	0	0	0	0	0	26	1	17	8	0	2	0	0	0	0	0	0	0	28	1	31	17	2	3	0	0	0	0	0	0	0	0	54		
	12:30	1	20	12	1	1	1	0	0	0	0	0	0	36	0	17	11	2	1	0	0	0	0	0	0	0	31	1	37	23	3	2	1	0	0	0	0	0	0	0	67		
	12:45	0	26	14	0	1	0	0	0	0	0	0	0	41	0	18	9	0	0	0	0	0	0	0	0	0	27	0	44	23	0	1	0	0	0	0	0	0	0	0	68		
	13:00	0	19	7	0	0	0	0	0	0	0	0	0	26	0	16	5	2	1	0	0	0	0	0	0	0	24	0	35	12	2	1	0	0	0	0	0	0	0	0	50		
	13:15	0	26	10	1	0	0	0	0	0	0	0	0	37	0	27	13	1	0	0	0	1	0	0	0	0	42	0	53	23	2	0	0	0	1	0	0	0	0	0	79		
	13:30	0	18	13	0	0	0	0	1	0	0	0	0	32	0	21	11	0	0	0	0	0	0	0	0	0	32	0	39	24	0	0	0	0	1	0	0	0	0	0	64		
	13:45	0	21	7	1	1	0	0	0	0	0	0	0	30	0	29	14	0	0	0	0	0	0	0	0	0	43	0	50	21	1	1	0	0	0	0	0	0	0	0	73		
	14:00	0	19	8	2	0	0	0	0	0	0	0	0	29	0	19	7	3	0	0	0	0	0	0	0	0	29	0	38	15	5	0	0	0	0	0	0	0	0	0	58		
	14:15	0	17	4	0	1	0	0	0	0	0	0	0	22	0	25	8	0	1	0	0	0	0	0	0	0	0	34	0	42	12	0	2	0	0	0	0	0	0	0	0	56	
	14:30	0	12	9	0	0	0	0	0	0	0	0	0	21	0	24	7	2	0	0	0	0	0	0	0	0	0	33	0	36	16	2	0	0	0	0	0	0	0	0	0	54	
	14:45	0	24	9	4	0	0	0	0	0	0	0	0	37	0	24	12	0	1	0	0	0	0	0	0	0	0	37	0	48	21	4	1	0	0	0	0	0	0	0	0	74	
	15:00	0	16	10	0	2	0	0	0	0	0	0	0	28	0	20	5	1	0	0	0	0	0	0	0	0	0	26	0	36	15	1	2	0	0	0	0	0	0	0	0	54	
	15:15	0	33	10	4	1	0	0	0	0	0	0	0	48	1	26	9	1	0	0	0	0	0	0	0	0	0	37	1	59	19	5	1	0	0	0	0	0	0	0	0	85	
	15:30	0	30	10	1	0	0	0	0	0	0	0	0	41	0	46	15	2	0	0	0	0	0	0	0	0	0	63	0	76	25	3	0	0	0	0	0	0	0	0	0	104	
	15:45	0	43	18	0	1	0	0	0	0	0	0	0	62	0	26	10	0	0	0	0	0	0	0	0	0	0	36	0	69	28	0	1	0	0	0	0	0	0	0	0	98	
	16:00	0	31	12	0	0	0	0	0	0	0	0	0	43	0	22	9	1	1	0	0	0	0	0	0	0	0	33	0	53	21	1	1	0	0	0	0	0	0	0	0	76	
	16:15	0	22	6	0	1	0	0	0	0	0	0	0	29	0	28	10	0	0	0	0	0	0	0	0	0	0	38	0	50	16	0	1	0	0	0	0	0	0	0	0	67	
	16:30	0	30	14	0	0	0	0	0	0	0	0	0	44	0	24	7	2	0	0	0	0	0	0	0	0	0	33	0	54	21	2	0	0	0	0	0	0	0	0	0	77	
	16:45	0	25	9	0	0	0	0	0	0	0	0	0	34	0	25	6	2	0	0	0	0	0	0	0	0	0	33	0	50	15	2	0	0	0	0	0	0	0	0	0	67	
	17:00	0	29	8	0	0	0	0	0	0	0	0	0	37	0	28	10	2	0	0	0	0	0	0	0	0	0	40	0	57	18	2	0	0	0	0	0	0	0	0	0	77	
	17:15	0	18	13	0	0	0	0	0	0	0	0	0	31	0	33	9	0	0	0	0	0	0	0	0	0	0	42	0	51	22	0	0	0	0	0	0	0	0	0	0	73	
	17:30	1	19	4	0	0	0	0	0	0	0	0	0	24	0	37	8	1	0	0	0	0	0	0	0	0	0	46	1	56	12	1	0	0	0	0	0	0	0	0	0	70	
	17:45	1	13	10	0	1	0	0	0	0	0	0	0	25	1	44	11	0	0	0	0	0	0	0	0	0	0	56	2	57	21	0	1	0	0	0	0	0	0	0	0	81	
	18:00	1	33	8	0	0	0	0	0	0	0	0	0	42	1	24	5	0	0	0	0	0	0	0	0	0	0	30	2	57	13	0	0	0	0	0	0	0	0	0	0	72	
	18:15	0	14	7	1	0	0	0	0	0	0	0	0	22	0	12	7	0	0	0	0	0	0	0	0	0	0	19	0	26	14	1	0	0	0	0	0	0	0	0	0	41	
	18:30	0	9	6	0	0	0	0	0	0	0	0	0	15	0	12	2	0	0	0	0	0	0	0	0	0	0	14	0	21	8	0	0	0	0	0	0	0	0	0	0	29	
	18:45	0	10	1	0	0	0	0	0	0	0	0	0	11	0	21	5	0	0	0	0	0	0	0	0	0	0	26	0	31	6	0	0	0	0	0	0	0	0	0	0	37	
	19:00	0	7	3	0	0	0	0	0	0	0	0	0	10	0	11	7	0	0	0	0	0	0	0	0	0	0	18	0	18	10	0	0	0	0	0	0	0	0	0	0	28	
	19:15	0	20	12	0	0	0	0	0	0	0	0	0	32	0	9	3	0	0	0	0	0	0	0	0	0	0	12	0	29	15	0	0	0	0	0	0	0	0	0	0	44	
	19:30	0	20	9	0	0	0	0	0	0	0	0	0	29	0	11	5	0	0	0	0	0	0	0	0	0	0	16	0	31	14	0	0	0	0	0	0	0	0	0	0	45	
	19:45	0	12	6	0	1	0	0	0	0	0	0	0	19	0	15	4	0	0	0	0	0	0	0	0	0	0	19	0	27	10	0	1	0	0	0	0	0	0	0	0	38	
	20:00	0	12	3	0	0	0	0	0	0	0	0	0	15	0	21	9	0	1	0	0	0	0	0	0	0	0	31	0	33	12	0	1	0	0	0	0	0	0	0	0	0	46
	20:15	0	4	4	0	0	0	0	0	0	0	0	0	8	0	14	2	0	0	0	0	0	0	0	0	0	0	16	0	18	6	0	0	0	0	0	0	0	0	0	0	24	
	20:30	0	7	1	0	0	0	0	0	0	0	0	0	8	0	14	7	0	0	0	0	0	0	0	0	0	0	21	0	21	8	0	0	0	0	0	0	0	0	0	0	29	
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	21:15	0	1	1	0	0	0	0	0	0	0	0	0	2	0	4	2	0	0	0	0	0	0	0	0	0	0	6	0	5	3	0	0	0	0	0	0	0	0	0	0	0	8
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	22:00	0	3	1	0	0	0	0	0	0	0	0	0	4	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	4	1	0	0	0	0	0	0	0	0	0	0	0	5
	22:15	0	4	1	0	0	0	0	0	0	0	0	0	5	0	2	2	0	0	0	0	0	0	0	0	0	0	4	0	6	3	0	0										

VOLUME

Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd

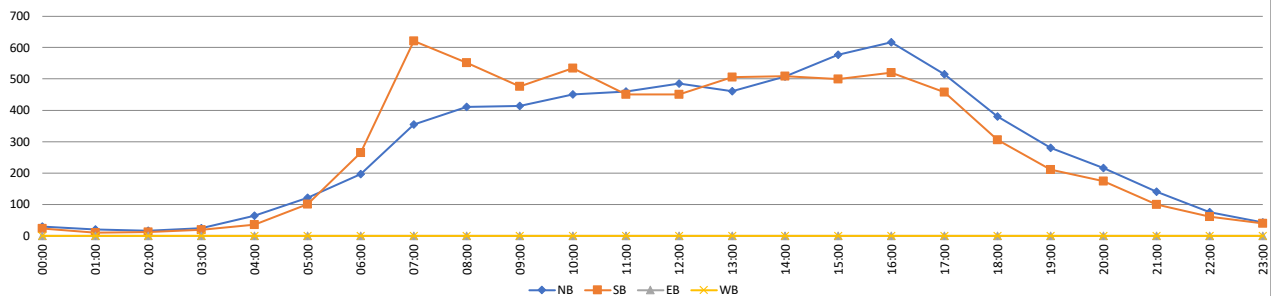
Day: Tuesday

City: Sonora

Date: 5/20/2025

Project #: CA25_090067_002

DAILY TOTALS						NB		SB		EB		WB		Total		DAILY TOTALS					
						6,858		6,933		0		0		13,791							
15-Minutes Interval												Hourly Intervals									
TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL	TIME		NB	SB	EB	WB	TOTAL			
0:00	9	4			13	12:00	125	106			231	00:00	01:00	29	23			52			
0:15	9	5			14	12:15	108	102			210	01:00	02:00	20	10			30			
0:30	8	4			12	12:30	129	119			248	02:00	03:00	16	12			28			
0:45	3	10			13	12:45	123	124			247	03:00	04:00	24	19			43			
1:00	7	2			9	13:00	96	125			221	04:00	05:00	64	35			99			
1:15	2	3			5	13:15	121	117			238	05:00	06:00	121	101			222			
1:30	8	3			11	13:30	105	130			235	06:00	07:00	197	265			462			
1:45	3	2			5	13:45	139	134			273	07:00	08:00	355	621			976			
2:00	3	4			7	14:00	125	123			248	08:00	09:00	411	552			963			
2:15	3	2			5	14:15	117	126			243	09:00	10:00	414	476			890			
2:30	4	3			7	14:30	141	123			264	10:00	11:00	451	534			985			
2:45	6	3			9	14:45	125	137			262	11:00	12:00	460	451			911			
3:00	3	1			4	15:00	159	154			313	12:00	13:00	485	451			936			
3:15	6	6			12	15:15	122	117			239	13:00	14:00	461	506			967			
3:30	8	7			15	15:30	146	144			290	14:00	15:00	508	509			1017			
3:45	7	5			12	15:45	150	84			234	15:00	16:00	577	499			1076			
4:00	14	3			17	16:00	141	124			265	16:00	17:00	617	520			1137			
4:15	15	10			25	16:15	178	124			302	17:00	18:00	515	458			973			
4:30	16	9			25	16:30	135	131			266	18:00	19:00	380	306			686			
4:45	19	13			32	16:45	163	141			304	19:00	20:00	280	211			491			
5:00	16	19			35	17:00	150	132			282	20:00	21:00	216	174			390			
5:15	22	25			47	17:15	135	109			244	21:00	22:00	140	100			240			
5:30	37	34			71	17:30	130	114			244	22:00	23:00	75	61			136			
5:45	46	23			69	17:45	100	103			203	23:00	00:00	42	39			81			
6:00	38	39			77	18:00	116	82			198	STATISTICS									
6:15	39	61			100	18:15	98	70			168		NB	SB	EB	WB	TOTAL				
6:30	55	76			131	18:30	86	80			166	Peak Period	00:00	to	12:00						
6:45	65	89			154	18:45	80	74			154	Volume	2562	3099			5661				
7:00	72	125			197	19:00	71	51			122	Peak Hour	11:00	7:30			7:30				
7:15	77	139			216	19:15	73	50			123	Peak Volume	460	654			1071				
7:30	105	173			278	19:30	68	59			127	Peak Hour Factor	0.898	0.889			0.939				
7:45	101	184			285	19:45	68	51			119	Peak Period	12:00	to	00:00						
8:00	104	146			250	20:00	64	63			127	Volume	4296	3834			8130				
8:15	107	151			258	20:15	47	49			96	Peak Hour	16:15	14:45			16:15				
8:30	106	129			235	20:30	52	30			82	Peak Volume	626	552			1154				
8:45	94	126			220	20:45	53	32			85	Peak Hour Factor	0.879	0.896			0.949				
9:00	103	105			208	21:00	49	25			74	Peak Period	07:00	to	09:00						
9:15	99	125			224	21:15	35	29			64	Volume	766	1173			1939				
9:30	105	100			205	21:30	27	27			54	Peak Hour	7:45	7:30			7:30				
9:45	107	146			253	21:45	29	19			48	Peak Volume	418	654			1071				
10:00	117	123			240	22:00	17	13			30	Peak Hour Factor	0.977	0.889			0.939				
10:15	102	136			238	22:15	15	14			29	Peak Period	16:00	to	18:00						
10:30	111	127			238	22:30	22	19			41	Volume	1132	978			2110				
10:45	121	148			269	22:45	21	15			36	Peak Hour	16:15	16:15			16:15				
11:00	85	117			202	23:00	11	13			24	Peak Volume	626	528			1154				
11:15	128	91			219	23:15	12	6			18	Peak Hour Factor	0.879	0.936			0.949				
11:30	119	111			230	23:30	11	11			22										
11:45	128	132			260	23:45	8	9			17										
TOTALS	2562	3099	0	0	5661	TOTALS	4296	3834	0	0	8130										
SPLIT %	45%	55%	0%	0%	41%	SPLIT %	53%	47%	0%	0%	59%										



VOLUME**Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd**

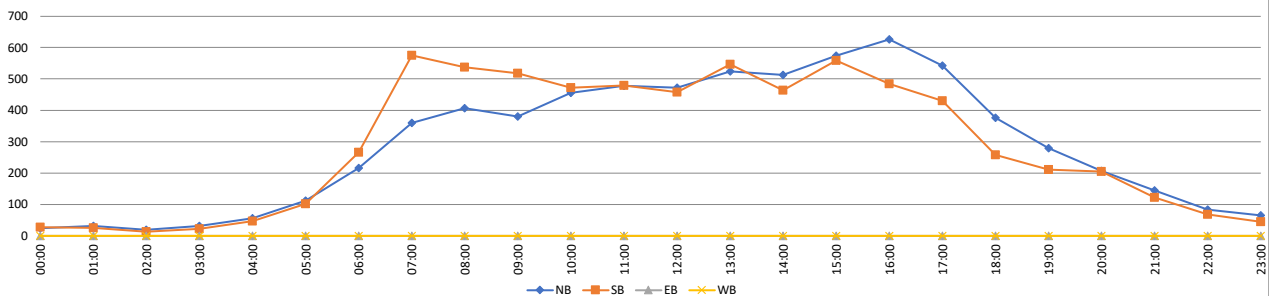
Day: Wednesday

City: Sonora

Date: 5/21/2025

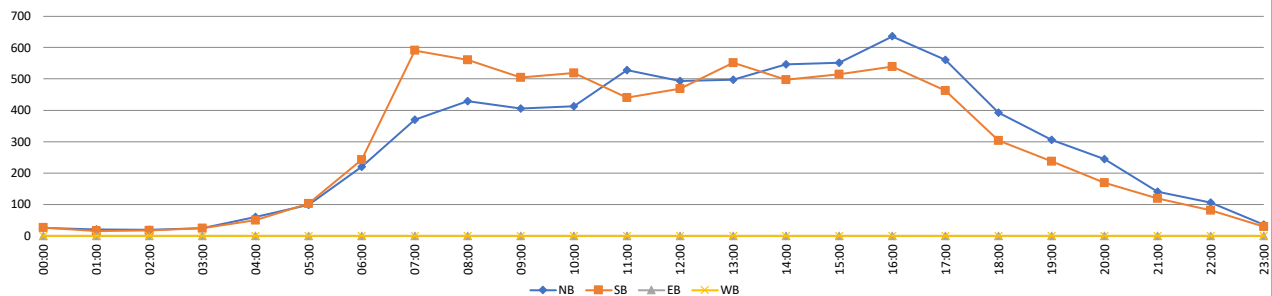
Project #: CA25_090067_002

DAILY TOTALS						NB		SB		EB		WB		Total		DAILY TOTALS					
						6,975		6,933		0		0		13,908							
15-Minutes Interval												Hourly Intervals									
TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL				
0:00	4	6			10	12:00	124	98			222	00:00 01:00	24	27			51				
0:15	5	8			13	12:15	116	99			215	01:00 02:00	31	25			56				
0:30	9	7			16	12:30	119	127			246	02:00 03:00	19	13			32				
0:45	6	6			12	12:45	113	134			247	03:00 04:00	31	22			53				
1:00	10	7			17	13:00	130	134			264	04:00 05:00	56	47			103				
1:15	11	6			17	13:15	131	120			251	05:00 06:00	112	102			214				
1:30	3	5			8	13:30	133	147			280	06:00 07:00	216	266			482				
1:45	7	7			14	13:45	130	145			275	07:00 08:00	360	575			935				
2:00	3	5			8	14:00	114	107			221	08:00 09:00	407	537			944				
2:15	6	1			7	14:15	112	105			217	09:00 10:00	380	518			898				
2:30	3	2			5	14:30	148	119			267	10:00 11:00	456	472			928				
2:45	7	5			12	14:45	139	133			272	11:00 12:00	478	479			957				
3:00	7	5			12	15:00	113	167			280	12:00 13:00	472	458			930				
3:15	3	3			6	15:15	162	154			316	13:00 14:00	524	546			1070				
3:30	10	6			16	15:30	124	132			256	14:00 15:00	513	464			977				
3:45	11	8			19	15:45	175	106			281	15:00 16:00	574	559			1133				
4:00	13	9			22	16:00	142	124			266	16:00 17:00	626	484			1110				
4:15	17	11			28	16:15	173	122			295	17:00 18:00	542	430			972				
4:30	14	12			26	16:30	148	126			274	18:00 19:00	376	258			634				
4:45	12	15			27	16:45	163	112			275	19:00 20:00	279	211			490				
5:00	14	15			29	17:00	138	115			253	20:00 21:00	207	205			412				
5:15	17	24			41	17:15	173	125			298	21:00 22:00	144	122			266				
5:30	37	29			66	17:30	131	87			218	22:00 23:00	83	68			151				
5:45	44	34			78	17:45	100	103			203	23:00 00:00	65	45			110				
6:00	33	36			69	18:00	110	85			195	STATISTICS									
6:15	65	54			119	18:15	103	65			168		NB	SB	EB	WB	TOTAL				
6:30	56	73			129	18:30	81	58			139	Peak Period	00:00	to	12:00						
6:45	62	103			165	18:45	82	50			132	Volume	2570	3083			5653				
7:00	61	96			157	19:00	62	66			128	Peak Hour	11:00	7:30			7:30				
7:15	81	138			219	19:15	63	53			116	Peak Volume	478	612			1043				
7:30	102	162			264	19:30	74	48			122	Peak Hour Factor	0.879	0.855			0.884				
7:45	116	179			295	19:45	80	44			124	Peak Period	12:00	to	00:00						
8:00	119	131			250	20:00	70	56			126	Volume	4405	3850			8255				
8:15	94	140			234	20:15	56	57			113	Peak Hour	15:45	14:45			14:30				
8:30	88	144			232	20:30	38	52			90	Peak Volume	638	586			1135				
8:45	106	122			228	20:45	43	40			83	Peak Hour Factor	0.911	0.877			0.898				
9:00	104	131			235	21:00	45	33			78	Peak Period	07:00	to	09:00						
9:15	93	137			230	21:15	37	33			70	Volume	767	1112			1879				
9:30	102	142			244	21:30	34	29			63	Peak Hour	7:30	7:30			7:30				
9:45	81	108			189	21:45	28	27			55	Peak Volume	431	612			1043				
10:00	98	129			227	22:00	27	18			45	Peak Hour Factor	0.905	0.855			0.884				
10:15	116	96			212	22:15	25	15			40	Peak Period	16:00	to	18:00						
10:30	115	123			238	22:30	15	21			36	Volume	1168	914			2082				
10:45	127	124			251	22:45	16	14			30	Peak Hour	16:00	16:00			16:00				
11:00	96	111			207	23:00	15	13			28	Peak Volume	626	484			1110				
11:15	117	125			242	23:15	15	7			22	Peak Hour Factor	0.905	0.960			0.941				
11:30	129	116			245	23:30	20	15			35										
11:45	136	127			263	23:45	15	10			25										
TOTALS	2570	3083	0	0	5653	TOTALS	4405	3850	0	0	8255										
SPLIT %	45%	55%	0%	0%	41%	SPLIT %	53%	47%	0%	0%	59%										



VOLUME**Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd**Day: Thursday
Date: 5/22/2025City: Sonora
Project #: CA25_090067_002

DAILY TOTALS						NB		SB		EB		WB		Total	DAILY TOTALS					
						7,121		7,067		0		0		14,188						
15-Minutes Interval											Hourly Intervals									
TIME	NB	SB	EB	WB	TOTAL	TIME	NB	SB	EB	WB	TOTAL	TIME		NB	SB	EB	WB	TOTAL		
0:00	7	12			19	12:00	127	130			257	00:00	01:00	25	26			51		
0:15	9	3			12	12:15	143	107			250	01:00	02:00	20	15			35		
0:30	6	4			10	12:30	106	106			212	02:00	03:00	19	17			36		
0:45	3	7			10	12:45	117	126			243	03:00	04:00	24	24			48		
1:00	8	7			15	13:00	116	146			262	04:00	05:00	60	50			110		
1:15	4	5			9	13:15	144	147			291	05:00	06:00	99	103			202		
1:30	2	2			4	13:30	129	140			269	06:00	07:00	220	242			462		
1:45	6	1			7	13:45	108	119			227	07:00	08:00	370	591			961		
2:00	2	4			6	14:00	123	125			248	08:00	09:00	429	561			990		
2:15	4	3			7	14:15	132	122			254	09:00	10:00	406	505			911		
2:30	7	2			9	14:30	138	122			260	10:00	11:00	413	519			932		
2:45	6	8			14	14:45	153	128			281	11:00	12:00	528	440			968		
3:00	5	3			8	15:00	126	141			267	12:00	13:00	493	469			962		
3:15	4	7			11	15:15	128	140			268	13:00	14:00	497	552			1049		
3:30	9	5			14	15:30	151	142			293	14:00	15:00	546	497			1043		
3:45	6	9			15	15:45	147	92			239	15:00	16:00	552	515			1067		
4:00	14	9			23	16:00	178	139			317	16:00	17:00	636	539			1175		
4:15	14	7			21	16:15	136	120			256	17:00	18:00	561	463			1024		
4:30	17	15			32	16:30	154	140			294	18:00	19:00	392	304			696		
4:45	15	19			34	16:45	168	140			308	19:00	20:00	306	237			543		
5:00	16	14			30	17:00	144	125			269	20:00	21:00	244	169			413		
5:15	21	29			50	17:15	170	108			278	21:00	22:00	140	119			259		
5:30	31	23			54	17:30	130	116			246	22:00	23:00	106	81			187		
5:45	31	37			68	17:45	117	114			231	23:00	00:00	35	29			64		
6:00	34	38			72	18:00	108	82			190	STATISTICS								
6:15	46	47			93	18:15	95	73			168		NB	SB	EB	WB	TOTAL			
6:30	64	56			120	18:30	87	62			149	Peak Period	00:00	to	12:00					
6:45	76	101			177	18:45	102	87			189	Volume	2613	3093			5706			
7:00	70	92			162	19:00	83	60			143	Peak Hour	11:00	7:30			7:30			
7:15	92	144			236	19:15	68	67			135	Peak Volume	528	645			1079			
7:30	94	169			263	19:30	78	58			136	Peak Hour Factor	0.892	0.867			0.899			
7:45	114	186			300	19:45	77	52			129									
8:00	134	140			274	20:00	76	45			121	Peak Period	12:00	to	00:00					
8:15	92	150			242	20:15	57	35			92	Volume	4508	3974			8482			
8:30	97	146			243	20:30	60	45			105	Peak Hour	16:00	12:45			16:00			
8:45	106	125			231	20:45	51	44			95	Peak Volume	636	559			1175			
9:00	90	112			202	21:00	42	32			74	Peak Hour Factor	0.893	0.951			0.927			
9:15	111	130			241	21:15	43	29			72									
9:30	98	150			248	21:30	21	32			53	Peak Period	07:00	to	09:00					
9:45	107	113			220	21:45	34	26			60	Volume	799	1152			1951			
10:00	90	145			235	22:00	36	24			60	Peak Hour	7:45	7:30			7:30			
10:15	114	114			228	22:15	23	22			45	Peak Volume	437	645			1079			
10:30	106	132			238	22:30	19	22			41	Peak Hour Factor	0.815	0.867			0.899			
10:45	103	128			231	22:45	28	13			41									
11:00	148	101			249	23:00	7	7			14	Peak Period	16:00	to	18:00					
11:15	141	110			251	23:15	14	8			22	Volume	1197	1002			2199			
11:30	124	127			251	23:30	7	11			18	Peak Hour	16:00	16:00			16:00			
11:45	115	102			217	23:45	7	3			10	Peak Volume	636	539			1175			
TOTALS	2613	3093	0	0	5706	TOTALS	4508	3974	0	0	8482	Peak Hour Factor	0.893	0.963			0.927			
SPLIT %	46%	54%	0%	0%	40%	SPLIT %	53%	47%	0%	0%	60%									

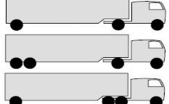
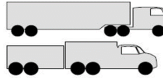
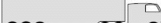
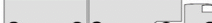
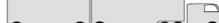


CLASSIFICATION

Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd

Day: Tuesday
Date: 5/20/2025City: Sonora
Project #: CA25_090067_002

HOURLY BREAKDOWN	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total	
	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total				
	0:00	0	21	6	0	1	0	0	0	1	0	0	0	0	29	0	16	7	0	0	0	0	0	0	0	0	0	23	0	37	13	0	1	0	0	1	0	0	0	0	52			
	1:00	0	13	3	0	1	3	0	0	0	0	0	0	0	20	0	9	1	0	0	0	0	0	0	0	0	0	10	0	22	4	0	1	3	0	0	0	0	0	0	30			
	2:00	0	7	4	0	1	4	0	0	0	0	0	0	0	16	0	8	2	0	1	0	0	0	1	0	0	0	12	0	15	6	0	2	4	0	0	1	0	0	0	28			
	3:00	0	8	5	0	0	11	0	0	0	0	0	0	0	24	0	14	5	0	0	0	0	0	0	0	0	0	19	0	22	10	0	0	11	0	0	0	0	0	43				
	4:00	0	23	22	0	5	12	0	0	0	0	1	1	0	64	0	20	13	0	1	0	0	0	1	0	0	0	35	0	43	35	0	6	12	0	0	1	1	1	0	99			
	5:00	0	54	56	0	9	1	0	0	0	0	0	1	0	121	1	65	30	0	1	0	0	1	3	0	0	0	101	1	119	86	0	10	1	0	1	3	0	1	0	222			
	6:00	1	107	63	2	7	7	2	3	5	0	0	0	0	197	2	163	81	2	2	0	0	4	10	0	1	0	265	3	270	144	4	9	7	2	7	15	0	1	0	462			
	7:00	2	209	107	3	10	13	0	4	6	1	0	0	0	355	0	442	153	2	8	0	0	2	13	0	1	0	621	2	651	260	5	18	13	0	6	19	1	1	0	0	976		
	8:00	1	268	114	1	8	12	0	2	3	2	0	0	0	411	2	386	139	5	8	3	0	3	6	0	0	0	552	3	654	253	6	16	15	0	5	9	2	0	0	0	963		
	9:00	0	264	132	2	7	4	0	1	2	2	0	0	0	414	3	296	143	1	21	1	1	3	7	0	0	0	476	3	560	275	3	28	5	1	4	9	2	0	0	0	890		
	10:00	3	292	131	3	7	11	0	1	3	0	0	0	0	451	6	354	154	1	4	2	0	1	12	0	0	0	534	9	646	285	4	11	13	0	2	15	0	0	0	0	985		
	11:00	3	318	115	4	4	9	0	3	3	0	1	0	0	460	2	320	104	3	13	4	0	0	4	1	0	0	0	451	5	638	219	7	17	13	0	3	7	1	1	0	0	911	
	12:00	5	319	141	1	8	7	0	0	3	0	1	0	0	485	3	279	144	3	8	3	0	1	7	2	1	0	0	451	8	598	285	4	16	10	0	1	10	2	2	0	0	936	
	13:00	4	305	125	1	13	6	0	0	6	1	0	0	0	461	3	317	151	1	10	4	0	3	13	3	1	0	0	506	7	622	276	2	23	10	0	3	19	4	1	0	0	967	
	14:00	2	346	137	2	9	8	0	2	1	1	0	0	0	508	4	329	151	4	7	2	0	2	8	2	0	0	0	509	6	675	288	6	16	10	0	4	9	3	0	0	0	1017	
	15:00	6	393	156	4	11	2	0	3	2	0	0	0	0	577	4	311	160	3	9	1	0	1	7	3	0	0	0	499	10	704	316	7	20	3	0	4	9	3	0	0	0	1076	
	16:00	2	454	151	0	5	1	0	1	2	1	0	0	0	617	2	344	151	2	12	0	0	4	4	1	0	0	0	520	4	798	302	2	17	1	0	5	6	2	0	0	0	1137	
	17:00	5	384	118	0	6	1	0	0	1	1	0	0	0	515	5	324	116	2	8	2	0	0	1	0	0	0	0	458	10	708	234	2	14	3	0	0	0	2	0	0	0	0	973
	18:00	1	271	102	0	4	0	0	1	1	0	0	0	0	380	3	207	90	0	6	0	0	0	0	0	0	0	0	306	4	478	192	0	10	0	0	1	1	0	0	0	0	686	
	19:00	5	197	71	0	7	0	0	0	0	0	0	0	0	280	4	148	52	0	5	0	0	2	0	0	0	0	0	211	9	345	123	0	12	0	0	2	0	0	0	0	0	491	
	20:00	1	156	56	0	2	0	0	1	0	0	0	0	0	216	1	138	35	0	0	0	0	0	0	0	0	0	0	174	2	294	91	0	2	0	0	1	0	0	0	0	0	390	
	21:00	1	112	27	0	0	0	0	0	0	0	0	0	0	140	1	75	22	0	2	0	0	0	0	0	0	0	0	100	2	187	49	0	2	0	0	0	0	0	0	0	0	240	
22:00	0	61	12	0	2	0	0	0	0	0	0	0	0	75	0	41	17	0	2	0	0	0	0	1	0	0	61	0	102	29	0	4	0	0	0	1	0	0	0	0	0	136		
23:00	0	35	6	0	0	0	0	0	0	1	0	0	0	42	0	30	9	0	0	0	0	0	0	0	0	0	0	39	0	65	15	0	0	0	0	0	1	0	0	0	0	81		
Totals		42	4,617	1,860	23	127	112	2	22	40	9	4	0	0	6,858	46	4,636	1,930	29	128	22	1	27	98	12	4	0	0	6,933	88	9,253	3,790	52	255	134	3	49	138	21	8	0	0	13,791	
% of Totals		1%	67%	27%	0%	2%	2%	0%	0%	1%	0%	0%	0%	100%	1%	67%	28%	0%	2%	0%	0%	0%	0%	1%	0%	0%	0%	100%	1%	67%	27%	0%	2%	1%	0%	0%	1%	0%	0%	0%	0%	0%	100%	

FHWA	CLASSIFICATION DEFINITIONS						
	 #1 Motorcycles	 #2 Passenger Cars	 #3 2-Axle, 4-Tire Single Unit	 #4 Buses	 #5 2-Axle, 6-Tire Single Units	 #6 3-Axle Single Units	 #7 >=4-Axle Single Units
	 #8 <=4-Axle Single Trailers	 #9 5-Axle Single Trailers	 #10 >=6-Axle Single Trailers	 #11 <=5-Axle Multi-Trailers	 #12 6-Axle Multi-Trailers	ANY 7 OR MORE AXLE #13 >=7-Axle Multi-Trailers	

STATISTICS	00:00 - 12:00												12:00 - 24:00												07:00 - 09:00												16:00 - 18:00											
	10	1584	758	15	60	87	2	14	23	6	3	0	0	2562	16	2093	832	14	59	10	1	14	57	1	2	0	0	3099	26	3677	1590	29	119	97	3	28	80	7	5	0	0	5661						
	%	0%	23%	11%	0%	1%	1%	0%	0%	0%	0%	0%	0%	37%	0%	30%	12%	0%	1%	0%	0%	0%	1%	0%	0%	0%	45%	0%	27%	12%	0%	1%	1%	0%	0%	1%	0%	0%	0%	0%	41%							
	Peak Hour	6:30	11:15	11:45	10:30	5:15	7:30	5:30	6:30	6:45	8:00	11:30	0:00	0:00	11:15	10:00	7:15	7:30	11:45	9:00	11:00	9:00	6:00	6:45	11:15	6:15	0:00	0:00	7:30	10:00	7:30	7:30	11:45	9:00	7:30	5:30	6:30	6:45	8:00	11:30	0:00	0:00	7:30					
	Peak Volume	3	351	137	5	12	18	2	5	6	2	2	0	0	500	6	463	167	6	21	4	1	4	16	2	2	0	0	654	9	722	288	8	28	19	2	9	22	2	3	0	0	1071					
	%	0%	44%	16%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	63%	0%	37%	16%	0%	1%	0%	0%	0%	1%	0%	0%	0%	55%	0%	40%	16%	0%	1%	0%	0%	0%	0%	0%	0%	0%	59%							
	Peak Hour	12:30	16:15	15:45	14:30	13:00	12:15	12:00	14:30	12:45	13:00	12:00	12:00	12:00	16:15	16:15	16:15	14:45	14:15	14:30	12:45	12:00	13:15	13:00	14:15	12:00	12:00	14:45	14:45	16:15	15:30	14:30	13:00	12:15	12:00	15:30	12:45	14:15	12:00	12:00	12:00	16:15						
	Peak Volume	6	468	167	4	13	8	0	3	7	1	1	0	0	626	6	361	169	5	12	5	0	4	13	5	1	0	0	552	10	829	322	9	23	11	0	6	19	6	2	0	0	1154					
	%	0%	47%	22%	4	18	25	0	6	9	3	0	0	0	76%	2	828	292	7	16	3	0	5	19	0	1	0	0	117%	5	1305	513	11	34	28	0	11	28	3	1	0	0	1939					
	Peak Hour	7:00	7:45	7:30	7:00	7:00	7:30	7:00	7:00	7:00	8:00	7:00	7:00	7:00	7:45	8:00	7:15	7:30	7:45	7:00	8:00	7:00	8:00	7:00	7:00	7:00	7:00	7:00	7:30	8:00	7:30	7:30	7:00	7:00	7:00	7:00	8:00	7:00	7:00	7:00	7:00	7:00	14%					
Peak Volume	2	276	121	3	10	18	0	4	6	2	0	0	0	418	2	463	167	5	8	3	0	3	13	0	1	0	0	654	3	722	288	7	18	19	0	6	19	2	1	0	0	1071						
%	0%	44%	16%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	63%	0%	37%	16%	0%	1%	0%	0%	0%	1%	0%	0%	0%	55%	0%	40%	16%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	59%							
Peak Hour	17:00	16:15	16:00	16:00	16:45	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:15	16:15	16:15	16:00	16:00	16:00	16:00	16:30	16:00	16:00	16:00	16:00	16:00	16:00	16:15	17:00	16:15	16:00	16:00	16:45	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:15						
Peak Volume	5	468	151	0	6	1	0	1	2	1	2	1	0	0	626	6	361	151	2	12	2	0	4	4	1	0	0	528	10	829	302	2	17	3	0	5	6	2	0	0	0	0	1154					

CLASSIFICATION

Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd

Day: Tuesday
Date: 5/20/2025City: Sonora
Project #: CA25_090067_002

	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	
15-MINUTE BREAKDOWN	0:00	0	7	1	0	0	0	0	0	1	0	0	0	0	9	0	3	1	0	0	0	0	0	0	0	0	0	4	0	10	2	0	0	0	0	0	1	0	0	0	0	0	13
	0:15	0	7	1	0	1	0	0	0	0	0	0	0	0	9	0	4	1	0	0	0	0	0	0	0	0	0	5	0	11	2	0	1	0	0	0	0	0	0	0	0	14	
	0:30	0	5	3	0	0	0	0	0	0	0	0	0	0	8	0	2	2	0	0	0	0	0	0	0	0	0	4	0	7	5	0	0	0	0	0	0	0	0	0	0	12	
	0:45	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	7	3	0	0	0	0	0	0	0	0	0	10	0	9	4	0	0	0	0	0	0	0	0	0	0	13	
	1:00	0	3	2	0	1	1	0	0	0	0	0	0	0	7	0	2	0	0	0	0	0	0	0	0	0	0	2	0	5	2	0	1	1	0	0	0	0	0	0	0	9	
	1:15	0	2	0	0	0	0	0	0	0	0	0	0	0	2	0	2	1	0	0	0	0	0	0	0	0	0	3	0	4	1	0	0	0	0	0	0	0	0	0	0	5	
	1:30	0	6	0	0	0	2	0	0	0	0	0	0	0	8	0	3	0	0	0	0	0	0	0	0	0	0	3	0	9	0	0	0	2	0	0	0	0	0	0	0	11	
	1:45	0	2	1	0	0	0	0	0	0	0	0	0	0	3	0	2	0	0	0	0	0	0	0	0	0	0	2	0	4	1	0	0	0	0	0	0	0	0	0	0	5	
	2:00	0	2	0	0	0	1	0	0	0	0	0	0	0	3	0	2	0	0	1	0	0	0	1	0	0	0	4	0	4	0	0	1	1	0	0	1	0	0	0	7		
	2:15	0	2	0	0	0	1	0	0	0	0	0	0	0	3	0	2	0	0	0	0	0	0	0	0	0	0	2	0	4	0	0	0	1	0	0	0	0	0	0	0	5	
	2:30	0	3	1	0	0	0	0	0	0	0	0	0	0	4	0	3	0	0	0	0	0	0	0	0	0	0	3	0	6	1	0	0	0	0	0	0	0	0	0	0	7	
	2:45	0	0	3	0	1	2	0	0	0	0	0	0	0	6	0	1	2	0	0	0	0	0	0	0	0	0	3	0	1	5	0	1	2	0	0	0	0	0	0	0	9	
	3:00	0	1	1	0	0	1	0	0	0	0	0	0	0	3	0	0	1	0	0	0	0	0	0	0	0	0	1	0	1	2	0	0	1	0	0	0	0	0	0	0	4	
	3:15	0	3	0	0	0	3	0	0	0	0	0	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0	6	0	9	0	0	0	3	0	0	0	0	0	0	0	12	
	3:30	0	2	2	0	0	4	0	0	0	0	0	0	0	8	0	4	3	0	0	0	0	0	0	0	0	0	7	0	6	5	0	0	4	0	0	0	0	0	0	0	15	
	3:45	0	2	2	0	0	3	0	0	0	0	0	0	0	7	0	4	1	0	0	0	0	0	0	0	0	0	5	0	6	3	0	0	3	0	0	0	0	0	0	0	12	
	4:00	0	4	4	0	1	5	0	0	0	0	0	0	0	14	0	0	3	0	0	0	0	0	0	0	0	0	3	0	4	7	0	1	5	0	0	0	0	0	0	17		
	4:15	0	5	7	0	0	2	0	0	0	0	0	1	0	15	0	5	3	0	1	0	0	0	1	0	0	0	10	0	10	10	0	1	2	0	0	1	0	1	0	0	25	
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9:45	0	69	33	0	1	2	0	0	0	0	0	0	0	107	1	92	44	0	6	1	1	1	0	0	0	0	0	146	1	161	77	0	7	3	1	1	0	2	0	0	0	253	
10:00	1	76	31	0	4	4	0	1	0	0	0	0	0	117	0	76	42	1	1	1	0	0	2	0	0	0	0	123	1	152	73	1	5	5	0	1	2	0	0	0	0	240	
10:15	2	68	28	0	0	3	0	0	1	0	0	0	0	102	3	90	37	0	2	1	0	0	3	0	0	0	0	136	5	158	65	0											

CLASSIFICATION

Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd

Day: Tuesday
Date: 5/20/2025

City: Sonora
Project #: CA25_090067_002

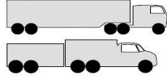
	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	
15-MINUTE BREAKDOWN	12:00	1	89	32	0	0	2	0	0	1	0	0	0	0	125	1	60	37	0	4	1	0	0	2	1	0	0	0	106	2	149	69	0	4	3	0	0	3	1	0	0	0	231
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	12:30	1	89	32	1	4	1	0	0	1	0	0	0	0	129	1	80	30	2	2	1	0	1	2	0	0	0	0	119	2	169	62	3	6	2	0	1	3	0	0	0	248	
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	13:45	2	90	42	0	2	2	0	0	0	0	1	0	0	139	1	89	37	1	2	0	0	0	3	1	0	0	0	134	3	179	79	1	4	2	0	0	3	2	0	0	0	273
	14:00	0	79	40	0	3	2	0	1	0	0	0	0	0	125	1	75	38	1	3	1	0	2	2	0	0	0	0	123	1	154	78	1	6	3	0	3	2	0	0	0	0	248
	14:15	1	79	33	1	3	0	0	0	0	0	0	0	0	117	1	90	34	0	0	0	0	0	1	0	0	0	0	126	2	169	67	1	3	0	0	0	1	0	0	0	0	243
	14:30	1	105	27	0	2	4	0	1	1	0	0	0	0	141	1	72	42	2	3	0	0	0	2	1	0	0	0	123	2	177	69	2	5	4	0	1	3	1	0	0	0	264
	14:45	0	83	37	1	1	2	0	0	0	0	1	0	0	125	1	92	37	1	1	1	0	0	3	1	0	0	0	137	1	175	74	2	2	3	0	0	3	2	0	0	0	262
	15:00	1	109	42	1	4	0	0	0	2	0	0	0	0	159	2	97	45	2	4	0	0	0	1	3	0	0	0	154	3	206	87	3	8	0	0	0	3	3	0	0	0	313
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	15:30	3	100	38	0	2	2	0	1	0	0	0	0	0	146	2	86	55	0	0	0	0	1	0	0	0	0	0	144	5	186	93	0	2	2	0	2	0	0	0	0	0	290
	15:45	1	102	45	1	1	0	0	0	0	0	0	0	0	150	0	51	28	1	1	1	0	0	2	0	0	0	0	84	1	153	73	2	2	1	0	0	2	0	0	0	0	234
	16:00	0	102	37	0	1	1	0	0	0	0	0	0	0	141	0	74	43	1	3	0	0	1	2	0	0	0	0	124	0	176	80	1	4	1	0	1	2	0	0	0	0	265
	16:15	1	130	45	0	0	0	0	1	1	0	0	0	0	178	0	84	31	1	4	0	0	2	2	0	0	0	0	124	1	214	76	1	4	0	0	3	3	0	0	0	0	302
	16:30	1	92	40	0	2	0	0	0	0	0	0	0	0	135	2	94	32	0	2	0	0	0	1	0	0	0	0	131	3	186	72	0	4	0	0	1	0	0	0	0	0	266
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	17:15	1	99	35	0	0	0	0	0	0	0	0	0	0	135	0	81	23	1	2	1	0	0	1	0	0	0	0	109	1	180	58	1	2	1	0	0	1	0	0	0	0	244
	17:30	0	96	30	0	3	1	0	0	0	0	0	0	0	130	0	84	26	1	3	0	0	0	0	0	0	0	0	114	0	180	56	1	6	1	0	0	0	0	0	0	0	244
	17:45	3	73	21	0	2	0	0	0	1	0	0	0	0	100	1	68	33	0	1	0	0	0	0	0	0	0	0	103	4	141	54	0	3	0	0	0	1	0	0	0	0	203
	18:00	1	81	32	0	0	0	0	1	1	0	0	0	0	116	1	51	28	0	2	0	0	0	0	0	0	0	0	82	2	132	60	0	2	0	0	1	1	0	0	0	0	198
	18:15	0	75	21	0	2	0	0	0	0	0	0	0	0	98	0	47	22	0	1	0	0	0	0	0	0	0	0	70	0	122	43	0	3	0	0	0	0	0	0	0	0	168
	18:30	0	64	22	0	0	0	0	0	0	0	0	0	0	86	1	52	25	0	2	0	0	0	0	0	0	0	0	80	1	116	47	0	2	0	0	0	0	0	0	0	0	166
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	19:00	0	52	19	0	0	0	0	0	0	0	0	0	0	71	1	36	12	0	2	0	0	0	0	0	0	0	0	51	1	88	31	0	2	0	0	0	0	0	0	0	0	122
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	20:00	0	46	16	0	1	0	0	1	0	0	0	0	0	64	1	53	9	0	0	0	0	0	0	0	0	0	0	63	1	99	25	0	1	0	0	1	0	0	0	0	0	127
	20:15	1	33	12	0	1	0	0	0	0	0	0	0	0	47	0	38	11	0	0	0	0	0	0	0	0	0	0	49	1	71	23	0	1	0	0	0	0	0	0	0	0	96
	20:30	0	35	17	0	0	0	0	0	0	0	0	0	0	52	0	23	7	0	0	0	0	0	0	0	0	0	0	30	0	58	24	0	0	0	0	0	0	0	0	0	0	82
	20:45	0	42	11	0	0	0	0	0	0	0	0	0	0	53	0	24	8	0	0	0	0	0	0	0	0	0	0	32	0	66	19	0	0	0	0	0	0	0	0	0	0	85
	21:00	1	36	12	0	0	0	0	0	0	0	0	0	0	49	0	20	5	0	0	0	0	0	0	0	0	0	0	25	1	56	17	0	0	0	0	0	0	0	0	0	0	74
	21:15	0	31	4	0	0	0	0	0	0	0	0	0	0	35	1	19	9	0	0	0	0	0	0	0	0	0	0	29	1	50	13	0	0	0	0	0	0	0	0	0	0	64
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	22:00	0	14																																								

CLASSIFICATION

Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd

Day: Wednesday
Date: 5/21/2025City: Sonora
Project #: CA25_090067_002

Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total
	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	
0:00	0	18	5	0	1	0	0	0	0	0	0	0	0	24	0	21	5	0	0	0	0	1	0	0	0	0	0	27	0	39	10	0	1	0	0	1	0	0	0	0	0	51
1:00	0	20	8	0	1	2	0	0	0	0	0	0	0	31	0	19	6	0	0	0	0	0	0	0	0	0	0	25	0	39	14	0	1	2	0	0	0	0	0	0	56	
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3:00	0	16	4	0	3	7	0	0	1	0	0	0	0	31	0	14	6	0	1	0	0	1	0	0	0	0	0	22	0	30	10	0	4	7	0	1	1	0	0	0	53	
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5:00	0	42	55	0	4	3	0	1	6	0	1	0	0	112	1	62	35	0	1	1	0	1	1	0	0	0	0	102	1	104	90	0	5	4	0	2	7	0	1	0	0	214
6:00	2	104	80	2	9	8	1	3	7	0	0	0	0	216	1	162	78	2	6	1	0	2	12	1	1	0	0	266	3	266	158	4	15	9	1	5	19	1	1	0	0	482
7:00	3	213	110	3	8	12	0	4	3	2	2	0	0	360	3	407	139	2	9	0	0	1	11	2	1	0	0	575	6	620	249	5	17	12	0	5	14	4	3	0	0	935
8:00	2	239	139	1	6	11	0	2	5	1	1	0	0	407	1	364	154	2	3	2	1	0	10	0	0	0	0	537	3	603	293	3	9	13	1	2	15	1	1	0	0	944
9:00	5	225	129	1	8	5	0	3	4	0	0	0	0	380	5	334	148	2	15	2	0	1	9	0	2	0	0	518	10	559	277	3	23	7	0	4	13	0	2	0	0	898
10:00	2	292	144	1	4	5	0	3	5	0	0	0	0	456	2	314	137	1	4	4	0	2	7	1	0	0	0	472	4	606	281	2	8	9	0	5	12	1	0	0	0	928
11:00	3	317	134	1	7	8	0	3	5	0	0	0	0	478	5	303	142	3	11	2	0	2	9	2	0	0	0	479	8	620	276	4	18	10	0	5	14	2	0	0	0	957
12:00	2	300	155	0	6	4	0	0	4	0	1	0	0	472	5	314	118	0	4	4	0	2	11	0	0	0	0	458	7	614	273	0	10	8	0	2	15	0	1	0	0	930
13:00	4	341	150	4	8	5	0	5	6	1	0	0	0	524	5	359	161	2	4	3	0	0	8	3	1	0	0	546	9	700	311	6	12	8	0	5	14	4	1	0	0	1070
14:00	2	360	137	1	5	2	0	1	5	0	0	0	0	513	13	296	130	4	12	1	0	1	6	0	1	0	0	464	15	656	267	5	17	3	0	2	11	0	1	0	0	977
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16:00	6	418	192	1	8	0	0	0	1	0	0	0	0	626	1	320	141	2	9	1	0	4	5	0	1	0	0	484	7	738	333	3	17	1	0	4	6	0	1	0	0	1110
17:00	5	407	121	0	5	3	0	0	1	0	0	0	0	542	2	292	130	0	6	0	0	0	0	0	0	0	0	430	7	699	251	0	11	3	0	0	1	0	0	0	0	972
18:00	3	277	93	0	2	0	0	0	1	0	0	0	0	376	4	181	68	0	4	0	0	0	1	0	0	0	0	258	7	458	161	0	6	0	0	0	2	0	0	0	0	634
19:00	1	199	71	0	7	1	0	0	0	0	0	0	0	279	2	142	62	0	3	0	0	0	1	1	0	0	0	211	3	341	133	0	10	1	0	1	1	0	0	0	0	490
20:00	0	144	61	0	2	0	0	0	0	0	0	0	0	207	2	146	56	0	0	0	0	1	0	0	0	0	0	205	2	290	117	0	2	0	0	1	0	0	0	0	0	412
21:00	0	115	29	0	0	0	0	0	0	0	0	0	0	144	1	84	37	0	0	0	0	0	0	0	0	0	122	1	199	66	0	0	0	0	0	0	0	0	0	0	266	
22:00	0	71	12	0	0	0	0	0	0	0	0	0	0	83	0	54	14	0	0	0	0	0	0	0	0	0	68	0	125	26	0	0	0	0	0	0	0	0	0	0	151	
23:00	0	44	19	0	1	0	0	0	1	0	0	0	0	65	0	30	15	0	0	0	0	0	0	0	0	0	0	45	0	74	34	0	1	0	0	0	1	0	0	0	0	110
Totals	47	4,602	2,019	20	106	88	1	25	57	4	6	0	0	6,975	57	4,616	1,970	24	106	24	1	21	98	9	7	0	0	6,933	104	9,218	3,989	44	212	112	2	46	155	13	13	0	0	13,908
% of Totals	1%	66%	29%	0%	2%	1%	0%	0%	1%	0%	0%	0%	0%	100%	1%	67%	28%	0%	2%	0%	0%	1%	0%	0%	0%	0%	100%	1%	66%	29%	0%	2%	1%	0%	0%	1%	0%	0%	0%	0%	100%	

FHWA	CLASSIFICATION DEFINITIONS						
	 #1 Motorcycles	 #2 Passenger Cars	 #3 2-Axle, 4-Tire Single Unit	 #4 Buses	 #5 2-Axle, 6-Tire Single Units	 #6 3-Axle Single Units	 #7 >=4-Axle Single Units
	 #8 <=4-Axle Single Trailers	 #9 5-Axle Single Trailers	 #10 >=6-Axle Single Trailers	 #11 <=5-Axle Multi-Trailers	 #12 6-Axle Multi-Trailers	ANY 7 OR MORE AXLE #13 >=7-Axle Multi-Trailers	

STATISTICS	00:00 - 12:00																										12:00 - 24:00																										07:00 - 09:00																										16:00 - 18:00																									
	17	1514	840	9	57	69	1	19	37	3	4	0	0	2570	18	2042	866	12	50	13	1	11	60	6	4	0	0	3083	35	3556	1706	21	107	82	2	30	97	9	8	0	0	5653																																																														
	0%	22%	12%	0%	1%	1%	0%	0%	1%	0%	0%	0%	0%	37%	0%	29%	12%	0%	1%	0%	0%	0%	0%	1%	0%	0%	0%	44%	0%	26%	12%	0%	1%	1%	0%	0%	1%	0%	0%	0%	41%																																																															
	Peak Hour	8:15	11:15	11:30	6:30	7:30	6:30	5:15	6:45	5:30	6:45	6:15	0:00	0:00	11:15	9:00	7:15	8:30	6:45	9:00	9:15	7:15	10:15	6:15	6:15	8:30	0:00	0:00	7:30	9:00	7:15	8:30	6:45	9:00	6:30	5:15	6:45	6:15	6:45	6:15	0:00	0:00	7:30																																																													
	Peak Volume	5	337	160	3	10	14	1	4	7	2	2	0	0	506	5	428	163	4	15	4	1	3	15	3	2	0	0	612	10	690	304	7	23	15	1	6	21	4	3	0	0	1043																																																													
	%	0%	44%	17%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	63%	1%	37%	16%	0%	1%	0%	0%	0%	1%	0%	0%	0%	0%	56%	0%	41%	16%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	59%																																																													
	Peak Hour	15:15	15:45	16:00	15:00	12:45	12:15	12:00	12:45	12:30	12:30	12:00	12:00	12:00	15:45	14:00	14:45	14:45	14:15	14:15	12:00	12:00	15:15	12:00	13:00	13:45	12:00	12:00	14:45	14:00	15:00	16:00	15:00	14:15	12:15	12:00	12:45	12:15	13:00	13:45	12:00	12:00	14:30																																																													
	Peak Volume	8	443	192	5	10	7	0	5	9	1	1	0	0	638	13	370	179	6	17	4	0	5	11	3	2	0	0	586	15	768	333	9	21	11	0	5	18	4	2	0	0	1135																																																													
	%	0%	45%	24%	4	14	23	0	6	8	3	3	0	0	767	4	771	293	4	12	2	1	1	21	2	1	0	0	1112	9	1223	542	8	26	25	1	7	29	5	4	0	0	1879																																																													
	Peak Hour	7:00	7:15	8:00	7:00	7:30	7:00	7:00	7:00	8:00	7:00	7:00	7:00	7:00	7:30	7:30	7:15	7:45	7:30	7:00	7:30	7:15	7:00	7:15	7:00	7:00	7:00	7:30	7:00	7:15	7:45	7:30	7:30	7:15	7:00	7:15	7:00	7:00	7:00	7:00	7:00	7:00	7:00	7:00	14%																																																											
Peak Volume	3	262	139	3	10	12	0	4	5	2	2	0	0	431	4	428	160	3	9	2	1	1	13	2	1	0	0	612	6	690	295	6	17	14	1	5	16	4	3	0	0	1043																																																														
%	0%	44%	17%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	63%	1%	37%	16%	0%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	56%	0%	41%	16%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	59%																																																												
Peak Hour	16:15	16:30	16:00	16:00	16:00	17:00	16:00	16:00	16:00	16:30	16:00	16:00	16:00	16:00	16:15	16:15	16:15	16:00	16:00	16:30	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:15	16:30	16:00	16:00	16:30	17:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00	16:00																																																												
Peak Volume	7	431	192	1	9	3	0	0	2	10	0	0	0	0	626	3	327	141	2	10	1	0	4	5	0	1	0	0	484	10	756	333	3	19	3	0	4	6	0	1	0	0	1110																																																													

CLASSIFICATION

Golden Chain Hwy/SR 49 N/O N Washington St/Columbia Way/Shaws Flat Rd

Day: Wednesday
Date: 5/21/2025City: Sonora
Project #: CA25_090067_002

	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	
15-MINUTE BREAKDOWN	0:00	0	3	1	0	0	0	0	0	0	0	0	0	4	0	5	1	0	0	0	0	0	0	0	0	0	6	0	8	2	0	0	0	0	0	0	0	0	0	0	0	10	
	0:15	0	3	2	0	0	0	0	0	0	0	0	0	5	0	5	2	0	0	0	0	1	0	0	0	0	8	0	8	4	0	0	0	0	1	0	0	0	0	0	13		
	0:30	0	7	2	0	0	0	0	0	0	0	0	0	9	0	6	1	0	0	0	0	0	0	0	0	0	7	0	13	3	0	0	0	0	0	0	0	0	0	0	16		
	0:45	0	5	0	0	1	0	0	0	0	0	0	0	6	0	5	1	0	0	0	0	0	0	0	0	0	6	0	10	1	0	1	0	0	0	0	0	0	0	0	12		
	1:00	0	5	4	0	0	1	0	0	0	0	0	0	10	0	5	2	0	0	0	0	0	0	0	0	0	7	0	10	6	0	0	1	0	0	0	0	0	0	0	17		
	1:15	0	10	1	0	0	0	0	0	0	0	0	0	11	0	6	0	0	0	0	0	0	0	0	0	0	6	0	16	1	0	0	0	0	0	0	0	0	0	0	0	17	
	1:30	0	2	1	0	0	0	0	0	0	0	0	0	3	0	4	1	0	0	0	0	0	0	0	0	0	5	0	6	2	0	0	0	0	0	0	0	0	0	0	8		
	1:45	0	3	2	0	1	1	0	0	0	0	0	0	7	0	4	3	0	0	0	0	0	0	0	0	0	7	0	7	5	0	1	1	0	0	0	0	0	0	0	14		
	2:00	0	1	2	0	0	0	0	0	0	0	0	0	3	0	4	1	0	0	0	0	0	0	0	0	0	5	0	5	3	0	0	0	0	0	0	0	0	0	0	8		
	2:15	0	5	0	0	0	1	0	0	0	0	0	0	6	0	1	0	0	0	0	0	0	0	0	0	0	1	0	6	0	0	0	1	0	0	0	0	0	0	0	0	7	
	2:30	0	1	2	0	0	0	0	0	0	0	0	0	3	0	2	0	0	0	0	0	0	0	0	0	0	2	0	3	2	0	0	0	0	0	0	0	0	0	0	0	5	
	2:45	0	3	2	0	1	1	0	0	0	0	0	0	7	0	3	2	0	0	0	0	0	0	0	0	0	0	5	0	6	4	0	1	1	0	0	0	0	0	0	0	12	
	3:00	0	3	1	0	0	2	0	0	1	0	0	0	7	0	3	2	0	0	0	0	0	0	0	0	0	0	5	0	6	3	0	0	2	0	0	1	0	0	0	0	12	
	3:15	0	3	0	0	0	0	0	0	0	0	0	0	3	0	2	0	0	1	0	0	0	0	0	0	0	0	3	0	5	0	0	1	0	0	0	0	0	0	0	0	6	
	3:30	0	4	1	0	3	2	0	0	0	0	0	0	10	0	3	3	0	0	0	0	0	0	0	0	0	0	6	0	7	4	0	3	2	0	0	0	0	0	0	0	16	
	3:45	0	6	2	0	0	3	0	0	0	0	0	0	11	0	6	1	0	0	0	0	1	0	0	0	0	0	8	0	12	3	0	0	3	0	1	0	0	0	0	0	19	
	4:00	0	6	6	0	1	0	0	0	0	0	0	0	13	0	5	3	0	0	1	0	0	0	0	0	0	0	9	0	11	9	0	1	1	0	0	0	0	0	0	0	22	
	4:15	0	3	9	0	2	3	0	0	0	0	0	0	17	0	8	3	0	0	0	0	0	0	0	0	0	0	11	0	11	12	0	2	3	0	0	0	0	0	0	0	28	
	4:30	0	7	4	0	1	2	0	0	0	0	0	0	14	0	7	4	0	0	0	0	0	1	0	0	0	0	12	0	14	8	0	1	2	0	0	1	0	0	0	0	26	
	4:45	0	2	7	0	1	1	0	0	1	0	0	0	12	0	12	3	0	0	0	0	0	0	0	0	0	0	15	0	14	10	0	1	1	0	0	1	0	0	0	0	27	
	5:00	0	5	7	0	0	0	0	0	2	0	0	0	14	0	9	5	0	0	0	0	0	1	0	0	0	0	15	0	14	12	0	0	0	0	0	3	0	0	0	0	29	
	5:15	0	10	4	0	0	0	0	1	2	0	0	0	17	0	16	8	0	0	0	0	0	0	0	0	0	0	24	0	26	12	0	0	0	0	1	2	0	0	0	0	41	
	5:30	0	14	18	0	1	3	0	0	1	0	0	0	37	0	18	9	0	0	1	0	1	0	0	0	0	0	29	0	32	27	0	1	4	0	1	1	0	0	0	0	66	
	5:45	0	13	26	0	3	0	0	0	1	0	0	1	0	44	1	19	13	0	1	0	0	0	0	0	0	0	34	1	32	39	0	4	0	0	0	1	0	1	0	0	78	
	6:00	0	16	11	0	3	0	0	1	1	1	0	0	0	33	0	19	16	0	0	0	0	0	1	0	0	0	36	0	35	27	0	3	0	1	1	2	0	0	0	0	69	
	6:15	1	26	28	0	2	2	0	2	4	0	0	0	0	65	0	26	20	0	1	0	0	1	6	0	0	0	54	1	52	48	0	3	2	0	3	10	0	0	0	0	119	
	6:30	1	27	24	1	0	3	0	0	0	0	0	0	0	56	1	46	21	0	1	1	0	0	2	1	0	0	73	2	73	45	1	1	4	0	0	2	1	0	0	0	129	
	6:45	0	35	17	1	4	3	0	0	2	0	0	0	0	62	0	71	21	2	4	0	0	1	3	0	1	0	103	0	106	38	3	8	3	0	1	5	0	1	0	0	165	
	7:00	1	31	20	0	2	5	0	0	0	0	2	0	0	61	0	61	25	1	3	0	0	0	4	2	0	0	96	1	92	45	1	5	5	0	0	4	2	2	0	0	157	
	7:15	1	53	21	1	0	3	0	1	1	0	0	0	0	81	0	100	32	0	2	0	0	0	4	0	0	0	138	1	153	53	1	2	3	0	1	5	0	0	0	0	219	
	7:30	1	54	34	1	3	2	0	3	2	2	0	0	0	102	2	117	38	1	2	0	0	1	1	0	0	0	162	3	171	72	2	5	2	0	4	3	2	0	0	0	264	
	7:45	0	75	35	1	3	2	0	0	0	0	0	0	0	116	1	129	44	0	2	0	0	0	2	0	1	0	179	1	204	79	1	5	2	0	0	2	0	1	0	0	295	
	8:00	0	80	35	0	0	4	0	0	0	0	0	0	0	119	0	82	40	0	2	0	1	0	6	0	0	0	131	0	162	75	0	2	4	1	0	6	0	0	0	0	250	
	8:15	1	48	32	1	4	4	0	1	2	0	1	0	0	94	1	98	34	2	0	2	0	0	3	0	0	0	140	2	146	66	3	4	6	0	1	5	0	1	0	0	234	
	8:30	1	51	33	0	1	1	0	0	1	0	0	0	0	88	0	100	42	0	1	0	0	0	1	0	0	0	144	1	151	75	0	2	1	0	0	2	0	0	0	0	232	
	8:45	0	60	39	0	1	2	0	1	2	1	0	0	0	106	0	84	38	0	0	0	0	0	0	0	0	0	122	0	144	77	0	1	2	0	1	2	1	0	0	0	228	
	9:00	3	63	30	0	3	2	0	2	1	0	0	0	0	104	1	74	48	0	4	0	0	1	2	0	1	0	131	4	137	78	0	7	2	0	3	3	0	1	0	0	235	
	9:15	0	49	39	0	0	2	0	1	2	0	0	0	0	93	1	91	35	1	6	1	0	0	1	0	1	0	137	1	140	74	1	6	3	0	1	3	0	1	0	0	230	
	9:30	1	62	35	1	2	0	0	0	1	0	0	0	0	102	1	94	37	1	4	1	0	0	4	0	0	0	142	2	156	72	2	6	1	0	0	5	0	0	0	0	244	
	9:45	1	51	25	0	3	1	0	0	0	0	0	0	0	81	2	75	28	0	1	0	0	0	2	0	0	0	108	3	126	53	0	4	1	0	0	2	0	0	0	0	189	
10:00	0	62	32	0	0	2	0	2	0	0	0	0	0	98	0	88	36	0	0	2	0	0	3	0	0	0	129	0	150	68	0	0	4	0	2	3	0	0	0	0	227		
10:15	1	73	33	1	2	1	0	1	4	0	0	0	0	116	2	60	30	0	0	1	0	2	0	1	0	0	96	3	133	63	1	2	2	0	3	4	1	0	0	0	212		
10:30																																											

City: Sonora
Project #: CA25_090067_002

	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total	
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		
15-MINUTE BREAKDOWN	12:00	0	85	39	0	0	0	0	0	0	0	0	0	124	0	65	28	0	2	1	0	0	2	4	0	0	0	98	0	150	67	0	2	1	0	0	2	0	0	0	0	222		
	12:15	1	74	38	0	1	1	0	0	0	0	1	0	116	1	72	19	0	1	1	0	0	1	4	0	0	0	99	2	146	57	0	2	2	0	1	4	0	1	0	0	215		
	12:30	1	73	37	0	3	3	0	0	2	0	0	0	119	3	81	36	0	0	1	0	1	5	0	0	0	0	127	4	154	73	0	3	4	0	1	7	0	0	0	246			
	12:45	0	68	41	0	2	0	0	0	2	0	0	0	113	1	96	35	0	1	1	0	0	0	0	0	0	0	134	1	164	76	0	3	1	0	0	2	0	0	0	247			
	13:00	0	88	31	4	1	3	0	0	3	0	0	0	130	3	84	39	1	2	1	0	0	0	2	2	0	0	134	3	172	70	5	3	4	0	0	5	2	0	0	264			
	13:15	0	85	38	0	3	0	0	2	2	1	0	0	131	0	79	38	0	1	1	0	0	1	0	0	0	0	120	0	164	76	0	4	1	0	2	3	1	0	0	251			
	13:30	2	80	42	0	4	1	0	3	1	0	0	0	133	2	95	45	1	0	1	0	0	3	0	0	0	0	147	4	175	87	1	4	2	0	3	4	0	0	0	280			
	13:45	2	88	39	0	0	1	0	0	0	0	0	0	130	0	101	39	0	1	0	0	0	0	2	1	1	0	145	2	189	78	0	1	1	0	0	2	1	1	0	0	275		
	14:00	0	79	32	0	2	0	0	0	0	1	0	0	0	114	3	70	32	1	0	0	0	0	1	0	0	0	107	3	149	64	1	2	0	0	0	2	0	0	0	0	221		
	14:15	1	75	31	0	1	1	0	0	3	0	0	0	0	112	3	74	21	0	5	0	0	0	2	0	0	0	105	4	149	52	0	6	1	0	0	5	0	0	0	0	217		
	14:30	1	108	35	0	2	0	0	1	1	1	0	0	0	148	3	69	38	2	4	1	0	0	1	0	1	0	0	119	4	177	73	2	6	1	0	1	2	0	1	0	0	267	
	14:45	0	98	39	1	0	1	0	0	0	0	0	0	0	139	4	83	39	1	3	0	0	1	2	0	0	0	0	133	4	181	78	2	3	1	0	1	2	0	0	0	0	272	
	15:00	0	82	30	0	1	0	0	0	0	0	0	0	0	113	0	111	44	3	5	0	0	0	4	0	0	0	0	167	0	193	74	3	6	0	0	0	4	0	0	0	0	280	
	15:15	5	109	44	1	2	1	0	0	0	0	0	0	0	162	1	92	54	0	4	1	0	1	1	0	0	0	0	154	6	201	98	1	6	2	0	1	1	0	0	0	0	316	
	15:30	1	95	22	1	1	2	0	0	1	0	1	0	0	124	1	84	42	1	2	1	0	0	1	0	0	0	0	132	2	179	64	2	3	3	0	0	2	0	1	0	0	256	
	15:45	1	126	43	3	1	1	0	0	0	0	0	0	0	175	2	69	32	0	3	0	0	0	0	0	0	0	0	106	3	195	75	3	4	1	0	0	0	0	0	0	0	281	
	16:00	1	99	40	0	2	0	0	0	0	0	0	0	0	142	0	78	38	1	1	1	0	4	1	0	0	0	0	124	1	177	78	1	3	1	0	4	1	0	0	0	0	266	
	16:15	1	122	49	0	1	0	0	0	0	0	0	0	0	173	0	87	29	1	2	0	0	0	3	0	0	0	0	122	1	209	78	1	3	0	0	0	3	0	0	0	0	295	
	16:30	2	96	49	0	1	0	0	0	0	0	0	0	0	148	0	86	35	0	4	0	0	0	1	0	0	0	0	126	2	182	84	0	5	0	0	0	1	0	0	0	0	274	
	16:45	2	101	54	1	4	0	0	0	0	1	0	0	0	163	1	69	39	0	2	0	0	0	0	0	0	1	0	112	3	170	93	1	6	0	0	0	1	0	1	0	0	0	275
	17:00	2	100	34	0	1	1	0	0	0	0	0	0	0	138	2	85	27	0	1	0	0	0	0	0	0	0	0	115	4	185	61	0	2	1	0	0	0	0	0	0	0	253	
	17:15	1	134	33	0	3	1	0	0	1	0	0	0	0	173	0	85	37	0	3	0	0	0	0	0	0	0	0	125	1	219	70	0	6	1	0	0	1	0	0	0	0	298	
	17:30	2	96	32	0	1	0	0	0	0	0	0	0	0	131	0	57	29	0	1	0	0	0	0	0	0	0	0	87	2	153	61	0	2	0	0	0	0	0	0	0	0	218	
	17:45	0	77	22	0	0	1	0	0	0	0	0	0	0	100	0	65	37	0	1	0	0	0	0	0	0	0	0	103	0	142	59	0	1	1	0	0	0	0	0	0	0	203	
	18:00	0	87	23	0	0	0	0	0	0	0	0	0	0	110	0	56	26	0	2	0	0	0	0	1	0	0	0	85	0	143	49	0	2	0	0	0	0	1	0	0	0	0	195
	18:15	1	84	18	0	0	0	0	0	0	0	0	0	0	103	2	44	19	0	0	0	0	0	0	0	0	0	0	65	3	128	37	0	0	0	0	0	0	0	0	0	0	168	
	18:30	1	50	29	0	1	0	0	0	0	0	0	0	0	81	1	42	14	0	1	0	0	0	0	0	0	0	0	58	2	92	43	0	2	0	0	0	0	0	0	0	0	139	
	18:45	1	56	23	0	1	0	0	0	0	1	0	0	0	82	1	39	9	0	1	0	0	0	0	0	0	0	0	50	2	95	32	0	2	0	0	0	0	1	0	0	0	0	132
	19:00	0	44	17	0	1	0	0	0	0	0	0	0	0	62	0	45	19	0	1	0	0	1	0	0	0	0	0	66	0	89	36	0	2	0	0	0	1	0	0	0	0	0	128
	19:15	0	45	16	0	2	0	0	0	0	0	0	0	0	63	1	33	18	0	1	0	0	0	0	0	0	0	0	53	1	78	34	0	3	0	0	0	0	0	0	0	0	0	116
	19:30	1	55	15	0	3	0	0	0	0	0	0	0	0	74	0	38	10	0	0	0	0	0	0	0	0	0	0	48	1	93	25	0	3	0	0	0	0	0	0	0	0	0	122
	19:45	0	55	23	0	1	1	0	0	0	0	0	0	0	80	1	26	15	0	1	0	0	0	1	0	0	0	0	44	1	81	38	0	2	1	0	0	0	1	0	0	0	0	124
20:00	0	45	23	0	2	0	0	0	0	0	0	0	0	70	1	40	15	0	0	0	0	0	0	0	0	0	0	56	1	85	38	0	2	0	0	0	0	0	0	0	0	0	126	
20:15	0	39	17	0	0	0	0	0	0	0	0	0	0	56	1	42	13	0	0	0	0	1	0	0	0	0	0	57	1	81	30	0	0	0	0	0	1	0	0	0	0	0	113	
20:30	0	33	5	0	0	0	0	0	0	0	0	0	0	38	0	34	18	0	0	0	0	0	0	0	0	0	0	52	0	67	23	0	0	0	0	0	0	0	0	0	0	0	90	
20:45	0	27	16	0	0	0	0	0	0	0	0	0	0	43	0	30	10	0	0	0	0	0	0	0	0	0	0	40	0	57	26	0	0	0	0	0	0	0	0	0	0	0	83	
21:00	0	35	10	0	0	0	0	0	0	0	0	0	0	45	0	21	12	0	0	0	0	0	0	0	0	0	0	33	0	56	22	0	0	0	0	0	0	0	0	0	0	0	78	
21:15	0	30	7	0	0	0	0	0	0	0	0	0	0	37	0	26	7	0	0	0	0	0	0	0	0	0	0	33	0	56	14	0	0	0	0	0	0	0	0	0	0	0	70	
21:30	0	28	6	0	0	0	0	0	0	0	0	0	0	34	0	20	9	0	0	0	0	0	0	0	0	0	0	29	0	48	15	0	0	0	0	0	0	0	0	0	0	0	63	
21:45	0	22	6	0	0	0	0	0	0	0	0	0	0	28	1	17	9	0	0	0	0	0	0	0	0	0	0	27	1	39	15	0	0	0	0	0	0	0	0	0	0	0	55	
22:00	0	25	2	0	0	0	0	0	0	0	0	0	0	27	0	14	4	0	0	0	0	0	0	0	0	0	0	18	0	39	6	0	0	0	0	0	0	0	0	0	0	0	45	
22:15	0	19	6	0	0	0	0	0	0	0	0	0	0	25	0	12	3	0	0	0	0	0	0	0	0	0	0	15	0	31	9	0	0	0	0	0	0	0	0	0	0	0	40	
22:30	0	13	2	0	0	0	0	0	0	0	0	0	0	15	0	19	2	0	0	0	0	0	0	0	0	0	0	21	0	32	4	0	0	0	0	0	0	0	0	0	0	0	36	
22:45	0	14	2	0	0	0	0	0	0	0	0	0	0																															

City: Sonora
Project #: CA25_090067_002

HOURLY BREAKDOWN	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total	
	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13					
	0:00	0	21	4	0	0	0	0	0	0	0	0	0	0	25	0	20	5	0	1	0	0	0	0	0	0	0	26	0	41	9	0	1	0	0	0	0	0	0	0	51			
	1:00	0	12	5	0	1	2	0	0	0	0	0	0	0	20	1	8	6	0	0	0	0	0	0	0	0	0	15	1	20	11	0	1	2	0	0	0	0	0	0	35			
	2:00	0	10	7	0	1	1	0	0	0	0	0	0	0	19	0	11	5	0	1	0	0	0	0	0	0	0	17	0	21	12	0	2	1	0	0	0	0	0	0	36			
	3:00	0	6	8	0	5	5	0	0	0	0	0	0	0	24	0	18	6	0	0	0	0	0	0	0	0	0	24	0	24	14	0	5	5	0	0	0	0	0	0	48			
	4:00	0	20	30	0	4	5	0	1	0	0	0	0	0	60	0	27	20	0	3	0	0	0	0	0	0	0	50	0	47	50	0	7	5	0	1	0	0	0	0	110			
	5:00	0	47	45	0	3	0	0	1	3	0	0	0	0	99	2	57	36	0	2	0	0	1	5	0	0	0	0	103	2	104	81	0	5	0	0	2	8	0	0	0	202		
	6:00	2	111	72	1	11	12	0	3	6	0	2	0	0	220	0	153	71	1	4	0	0	3	10	0	0	0	0	242	2	264	143	2	15	12	0	6	16	0	2	0	0	462	
	7:00	1	230	106	3	6	14	0	4	5	1	0	0	0	370	3	413	153	1	2	2	0	2	11	2	2	0	0	591	4	643	259	4	8	16	0	6	16	3	2	0	0	961	
	8:00	1	254	148	2	4	11	0	6	2	1	0	0	0	429	3	363	174	5	7	0	0	1	8	0	0	0	0	561	4	617	322	7	11	11	0	7	10	1	0	0	0	990	
	9:00	2	251	124	6	9	9	1	3	1	0	0	0	0	406	1	323	153	0	16	3	0	4	5	0	0	0	0	505	3	574	277	6	25	12	1	7	6	0	0	0	0	911	
	10:00	0	264	132	1	3	8	0	2	3	0	0	0	0	413	6	326	166	1	3	3	1	0	11	2	0	0	0	519	6	590	298	2	6	11	1	2	14	2	0	0	0	932	
	11:00	2	323	179	2	8	7	0	4	3	0	0	0	0	528	2	289	125	4	8	1	0	2	7	1	1	0	0	440	4	612	304	6	16	8	0	6	10	1	1	0	0	0	968
	12:00	5	302	167	1	6	3	0	2	6	0	1	0	0	493	5	313	128	2	8	1	1	3	7	0	1	0	0	469	10	615	295	3	14	4	1	5	13	0	2	0	0	0	962
	13:00	10	315	156	3	5	5	0	1	2	0	0	0	0	497	3	359	148	5	10	4	0	4	14	3	2	0	0	552	13	674	304	8	15	9	0	5	16	3	2	0	0	0	1049
	14:00	1	379	147	0	8	2	0	0	9	0	0	0	0	546	7	325	138	2	10	3	0	2	10	0	0	0	0	497	8	704	285	2	18	5	0	2	19	0	0	0	0	0	1043
	15:00	5	365	160	4	9	3	0	4	2	0	0	0	0	552	4	346	150	2	6	0	0	1	6	0	0	0	0	515	9	711	310	6	15	3	0	5	8	0	0	0	0	0	1067
	16:00	2	425	196	0	9	2	0	1	1	0	0	0	0	636	6	346	164	2	11	1	0	3	6	0	0	0	0	539	8	771	360	2	20	3	0	4	7	0	0	0	0	0	1175
	17:00	5	415	128	0	10	1	0	0	2	0	0	0	0	561	3	321	131	0	8	0	0	0	0	0	0	0	0	463	8	736	259	0	18	1	0	0	2	0	0	0	0	0	1024
18:00	1	282	100	0	5	0	0	1	3	0	0	0	0	392	3	216	76	0	8	0	0	0	1	0	0	0	0	304	4	498	176	0	13	0	0	1	4	0	0	0	0	0	696	
19:00	1	227	74	1	3	0	0	0	0	0	0	0	0	306	3	159	71	0	0	0	0	2	1	1	0	0	0	237	4	386	145	1	3	0	0	2	1	1	0	0	0	0	543	
20:00	2	167	74	0	1	0	0	0	0	0	0	0	0	244	2	126	41	0	0	0	0	0	0	0	0	0	0	169	4	293	115	0	1	0	0	0	0	0	0	0	0	0	413	
21:00	0	112	28	0	0	0	0	0	0	0	0	0	0	140	1	82	34	0	1	0	0	1	0	0	0	0	0	119	1	194	62	0	1	0	0	1	0	0	0	0	0	0	259	
22:00	0	75	30	0	0	0	0	0	1	0	0	0	0	106	0	55	26	0	0	0	0	0	0	0	0	0	0	81	0	130	56	0	0	0	0	0	1	0	0	0	0	0	187	
23:00	0	27	8	0	0	0	0	0	0	0	0	0	0	35	0	23	6	0	0	0	0	0	0	0	0	0	0	29	0	50	14	0	0	0	0	0	0	0	0	0	0	0	64	
Totals	40	4,640	2,128	24	111	90	1	33	49	2	3	0	0	7,121	55	4,679	2,033	25	109	18	2	29	102	9	6	0	0	7,067	95	9,319	4,161	49	220	108	3	62	151	11	9	0	0	0	0	14,188
% of Totals	1%	65%	30%	0%	2%	1%	0%	0%	1%	0%	0%	0%	0%	100%	1%	66%	29%	0%	2%	0%	0%	0%	1%	0%	0%	0%	100%	1%	63%	29%	0%	2%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	100%	

FHWA	CLASSIFICATION DEFINITIONS						
	 #1 Motorcycles	 #2 Passenger Cars	 #3 2-Axis, 4-Tire Single Unit	 #4 Buses	 #5 2-Axis, 6-Tire Single Units	 #6 3-Axis Single Units	 #7 ≥4-Axis Single Units
	 #8 ≤4-Axis Single Trailers	 #9 5-Axis Single Trailers	 #10 ≥6-Axis Single Trailers	 #11 ≤5-Axis Multi-Trailers	 #12 6-Axis Multi-Trailers	ANY 7 OR MORE AXLE #13 ≥7-Axis Multi-Trailers	

[illegible]

City: Sonora
Project #: CA25_090067_002

[illegible]

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	Time	NORTHBOUND													Total	SOUTHBOUND													Total	TOTALS													Total		
		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13		#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13			
15-MINUTE BREAKDOWN	12:00	2	76	47	0	0	0	0	0	1	0	1	0	0	127	2	87	37	0	1	1	0	1	1	0	0	0	0	130	4	163	84	0	1	1	0	1	2	0	0	1	0	0	257	
	12:15	0	95	43	0	2	2	0	1	0	0	0	0	0	143	1	74	26	0	4	0	0	1	1	0	0	0	0	107	1	169	69	0	6	2	0	2	1	0	0	0	0	250		
	12:30	0	66	32	1	2	0	0	0	5	0	0	0	0	106	1	68	34	1	1	0	0	0	1	0	0	0	0	106	1	134	66	2	3	0	0	0	6	0	0	0	0	212		
	12:45	3	65	45	0	2	1	0	1	0	0	0	0	0	117	1	84	31	1	2	0	1	1	4	0	1	0	0	126	4	149	76	1	4	1	1	2	4	0	0	1	0	0	243	
	13:00	1	76	35	0	2	1	0	0	0	1	0	0	0	116	0	95	40	2	4	1	0	1	0	3	0	0	0	146	1	171	75	2	6	2	0	1	1	3	0	0	0	0	262	
	13:15	5	83	50	2	2	1	0	0	1	0	0	0	0	144	2	98	31	3	1	2	0	1	7	0	2	0	0	147	7	181	81	5	3	3	0	1	8	0	2	0	0	0	291	
	13:30	1	80	45	0	1	1	0	1	0	0	0	0	0	129	0	97	37	0	3	0	0	0	3	0	0	0	0	140	1	177	82	0	4	1	0	1	3	0	0	0	0	0	269	
	13:45	3	76	26	1	0	2	0	0	0	0	0	0	0	108	1	69	40	0	2	1	0	2	4	0	0	0	0	119	4	145	66	1	2	3	0	2	4	0	0	0	0	0	227	
	14:00	0	83	36	0	3	0	0	0	0	1	0	0	0	123	2	82	34	2	1	1	0	0	3	0	0	0	0	125	2	165	70	2	4	1	0	0	0	4	0	0	0	0	0	248
	14:15	0	93	33	0	1	1	0	0	4	0	0	0	0	132	1	78	30	0	6	1	0	2	4	0	0	0	0	122	1	171	63	0	7	2	0	2	8	0	0	0	0	0	254	
	14:30	1	91	44	0	1	0	0	0	1	0	0	0	0	138	2	79	39	0	0	1	0	0	1	0	0	0	0	122	3	170	83	0	1	1	0	0	2	0	0	0	0	0	260	
	14:45	0	112	34	0	3	1	0	0	3	0	0	0	0	153	2	86	35	0	3	0	0	0	2	0	0	0	0	128	2	198	69	0	6	1	0	0	5	0	0	0	0	0	281	
	15:00	1	74	43	1	3	2	0	0	2	0	0	0	0	126	0	107	34	0	0	0	0	0	0	0	0	0	0	141	1	181	77	1	3	2	0	0	2	0	0	0	0	0	0	267
	15:15	2	92	30	0	2	1	0	1	0	0	0	0	0	128	1	91	40	1	4	0	0	0	3	0	0	0	0	140	3	183	70	1	6	1	0	1	3	0	0	0	0	0	0	268
	15:30	2	104	41	2	1	0	0	1	0	0	0	0	0	151	3	92	44	1	1	0	0	0	1	0	0	0	0	142	5	196	85	3	2	0	0	1	1	0	0	0	0	0	293	
	15:45	0	95	46	1	3	0	0	2	0	0	0	0	0	147	0	56	32	0	1	0	0	1	2	0	0	0	0	92	0	151	78	1	4	0	0	3	2	0	0	0	0	0	239	
	16:00	0	110	64	0	1	2	0	1	0	0	0	0	0	178	3	89	40	1	4	0	0	1	1	0	0	0	0	139	3	199	104	1	5	2	0	2	1	0	0	0	0	0	317	
	16:15	1	96	36	0	3	0	0	0	0	0	0	0	0	136	0	67	45	0	5	1	0	0	2	0	0	0	0	120	1	163	81	0	8	1	0	0	2	0	0	0	0	0	256	
	16:30	1	106	43	0	3	0	0	0	1	0	0	0	0	154	3	99	36	1	0	0	0	1	0	0	0	0	0	140	4	205	79	1	3	0	0	1	1	0	0	0	0	0	294	
	16:45	0	113	53	0	2	0	0	0	0	0	0	0	0	168	0	91	43	0	2	0	0	1	3	0	0	0	0	140	0	204	96	0	4	0	0	1	3	0	0	0	0	0	308	
	17:00	3	104	32	0	4	0	0	0	1	0	0	0	0	144	2	80	43	0	0	0	0	0	0	0	0	0	0	125	5	184	75	0	4	0	0	0	1	0	0	0	0	0	0	269
	17:15	1	119	44	0	4	1	0	0	1	0	0	0	0	170	0	75	29	0	4	0	0	0	0	0	0	0	0	108	1	194	73	0	8	1	0	0	1	0	0	0	0	0	278	
	17:30	0	107	23	0	0	0	0	0	0	0	0	0	0	130	0	89	24	0	3	0	0	0	0	0	0	0	116	0	196	47	0	3	0	0	0	0	0	0	0	0	0	246		
	17:45	1	85	29	0	2	0	0	0	0	0	0	0	0	117	1	77	35	0	1	0	0	0	0	0	0	0	0	114	2	162	64	0	3	0	0	0	0	0	0	0	0	0	231	
	18:00	0	76	28	0	3	0	0	0	1	0	0	0	0	108	2	56	20	0	3	0	0	0	1	0	0	0	0	82	2	132	48	0	6	0	0	0	2	0	0	0	0	0	190	
	18:15	1	69	24	0	0	0	0	1	0	0	0	0	0	95	0	57	14	0	2	0	0	0	0	0	0	0	0	73	1	126	38	0	2	0	0	1	0	0	0	0	0	0	168	
	18:30	0	65	20	0	1	0	0	0	1	0	0	0	0	87	1	43	17	0	1	0	0	0	0	0	0	0	0	62	1	108	37	0	2	0	0	0	1	0	0	0	0	0	149	
	18:45	0	72	28	0	1	0	0	0	1	0	0	0	0	102	0	60	25	0	2	0	0	0	0	0	0	0	0	87	0	132	53	0	3	0	0	0	1	0	0	0	0	0	189	
	19:00	1	60	22	0	0	0	0	0	0	0	0	0	0	83	0	41	19	0	0	0	0	0	0	0	0	0	0	60	1	101	41	0	0	0	0	0	0	0	0	0	0	0	143	
	19:15	0	52	15	0	1	0	0	0	0	0	0	0	0	68	0	44	21	0	0	0	0	0	1	1	0	0	0	67	0	96	36	0	1	0	0	0	1	1	0	0	0	0	135	
	19:30	0	54	23	0	1	0	0	0	0	0	0	0	0	78	0	37	19	0	0	0	0	2	0	0	0	0	0	58	0	91	42	0	1	0	0	2	0	0	0	0	0	0	136	
	19:45	0	61	14	1	1	0	0	0	0	0	0	0	0	77	3	37	12	0	0	0	0	0	0	0	0	0	0	52	3	98	26	1	1	0	0	0	0	0	0	0	0	0	129	
20:00	0	54	22	0	0	0	0	0	0	0	0	0	0	76	1	35	9	0	0	0	0	0	0	0	0	0	0	45	1	89	31	0	0	0	0	0	0	0	0	0	0	0	121		
20:15	2	37	18	0	0	0	0	0	0	0	0	0	0	57	0	26	9	0	0	0	0	0	0	0	0	0	0	35	2	63	27	0	0	0	0	0	0	0	0	0	0	0	92		
20:30	0	40	19	0	1	0	0	0	0	0	0	0	0	60	0	33	12	0	0	0	0	0	0	0	0	0	0	45	0	73	31	0	1	0	0	0	0	0	0	0	0	0	105		
20:45	0	36	15	0	0	0	0	0	0	0	0	0	0	51	1	32	11	0	0	0	0	0	0	0	0	0	0	44	1	68	26	0	0	0	0	0	0	0	0	0	0	0	95		
21:00	0	35	7	0	0	0	0	0	0	0	0	0	0	42	1	19	11	0	1	0	0	0	0	0	0	0	0	32	1	54	18	0	1	0	0	0	0	0	0	0	0	0	74		
21:15	0	33	10	0	0	0	0	0	0	0	0	0	0	43	0	21	7	0	0	0	0	1	0	0	0	0	0	29	0	54	17	0	0	0	0	1	0	0	0	0	0	0	72		
21:30	0	17	4	0	0	0	0	0	0	0	0	0	0	21	0	22	10	0	0	0	0	0	0	0	0	0	0	32	0	39	14	0	0	0	0	0	0	0	0	0	0	0	53		
21:45	0	27	7	0	0	0	0	0	0	0	0	0	0	34	0	20	6	0	0	0	0	0	0	0	0	0	0	26	0	47	13	0	0	0	0	0	0	0	0	0	0	0	60		
22:00	0	23	13	0	0	0	0	0	0	0	0	0	0	36	0	16	8	0	0	0	0	0	0	0	0	0	0	24	0	39	21	0	0	0	0	0	0	0	0	0	0	0	60		
22:15	0	20	3	0	0	0	0	0	0	0	0	0	0	23	0	12	10	0	0	0	0	0	0	0	0	0	0	22	0	32	13	0	0	0	0	0	0	0	0	0	0	0	45		
22:30	0	13	5	0	0	0	0	0	1	0	0	0	0	19	0	19																													

Appendix C. Existing (2025) Conditions SimTraffic Outputs

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Denied Del/Veh (s)	0.5	2.4	0.0	0.0	0.0	0.0	0.0	0.0	7.1	4.1	3.2	2.1
Total Del/Veh (s)	98.3	106.7	150.4	34.4	43.1	12.9	1.7	0.4	21.6	10.9	10.7	24.8
Vehicles Exited	15	219	10	1	3	195	423	16	10	593	30	1515
Hourly Exit Rate	15	219	10	1	3	195	423	16	10	593	30	1515
Input Volume	16	231	13	1	3	207	435	17	12	614	28	1577
% of Volume	94	95	77	100	100	94	97	94	83	97	107	96

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	NBR	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	7.8	5.0	3.8	0.1	0.0	2.3
Total Del/Veh (s)	7.8	5.7	10.9	14.9	9.2	7.7	5.4	10.4
Vehicles Exited	17	43	70	580	11	754	82	1557
Hourly Exit Rate	17	43	70	580	11	754	82	1557
Input Volume	17	40	77	603	10	788	84	1619
% of Volume	100	108	91	96	110	96	98	96

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	EBT	NBT	SBR	All
Denied Del/Veh (s)	0.0		0.0	0.1	0.0
Total Del/Veh (s)	0.6		1.5	35.2	10.4
Vehicles Exited	29	0	11	14	54
Hourly Exit Rate	29	0	11	14	54
Input Volume	29	3	10	17	59
% of Volume	100	0	110	82	92

Total Network Performance

Denied Del/Veh (s)	4.1
Total Del/Veh (s)	33.8
Vehicles Exited	1656
Hourly Exit Rate	1656
Input Volume	4920
% of Volume	34

Queuing and Blocking Report

Existing 2025 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	NB	SB	SB
Directions Served	LT	R	LTR	L	TR	L	TR
Maximum Queue (ft)	485	197	54	85	12	35	315
Average Queue (ft)	171	148	17	58	0	4	50
95th Queue (ft)	472	243	47	89	12	21	347
Link Distance (ft)	1155		52	54	54		1130
Upstream Blk Time (%)			11	19			2
Queuing Penalty (veh)			2	58			0
Storage Bay Dist (ft)		130				50	
Storage Blk Time (%)	0	50				0	4
Queuing Penalty (veh)	1	8				0	1

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	57	124	417	117
Average Queue (ft)	29	26	103	54
95th Queue (ft)	51	78	335	120
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)			1	4
Queuing Penalty (veh)			0	32
Storage Bay Dist (ft)		80		
Storage Blk Time (%)		0	9	
Queuing Penalty (veh)		0	7	

Intersection: 3: Columbia Way & Colombia Way

Movement	EB	NB	SB
Directions Served	L	T	R
Maximum Queue (ft)	3	3	45
Average Queue (ft)	0	0	5
95th Queue (ft)	3	3	35
Link Distance (ft)	52	113	664
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 109

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.3	0.6	2.5	0.0	0.0	0.0	0.0	0.0	0.0	3.1	0.4	0.4
Total Del/Veh (s)	38.8	29.2	15.6	69.0	33.7	17.5	8.1	1.3	0.3	4.2	3.0	1.0
Vehicles Exited	14	1	142	9	1	6	138	537	16	4	530	16
Hourly Exit Rate	14	1	142	9	1	6	138	537	16	4	530	16
Input Volume	14	1	143	11	1	5	143	550	15	6	534	15
% of Volume	100	100	99	82	100	120	97	98	107	67	99	107

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.4
Total Del/Veh (s)	4.9
Vehicles Exited	1414
Hourly Exit Rate	1414
Input Volume	1438
% of Volume	98

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	NBR	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.2	0.6	0.6	0.0	0.0	0.4
Total Del/Veh (s)	6.6	5.5	4.7	7.2	4.2	5.9	3.7	6.4
Vehicles Exited	28	34	28	651	8	642	58	1449
Hourly Exit Rate	28	34	28	651	8	642	58	1449
Input Volume	30	33	27	665	9	655	54	1473
% of Volume	93	103	104	98	89	98	107	98

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	NBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.1	0.0
Total Del/Veh (s)	0.6	1.4	0.5	0.7
Vehicles Exited	21	8	16	45
Hourly Exit Rate	21	8	16	45
Input Volume	22	9	17	48
% of Volume	95	89	94	94

Total Network Performance

Denied Del/Veh (s)	0.7
Total Del/Veh (s)	11.9
Vehicles Exited	1504
Hourly Exit Rate	1504
Input Volume	4453
% of Volume	34

Queuing and Blocking Report

Existing 2025 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	SB	SB
Directions Served	LT	R	LTR	L	L	TR
Maximum Queue (ft)	57	133	51	75	21	62
Average Queue (ft)	10	53	13	42	2	5
95th Queue (ft)	44	110	39	73	11	28
Link Distance (ft)	1155		52	54		1130
Upstream Blk Time (%)			3	5		
Queuing Penalty (veh)			1	19		
Storage Bay Dist (ft)		130			50	
Storage Blk Time (%)	0	1				0
Queuing Penalty (veh)	0	0				0

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	59	26	163	103
Average Queue (ft)	29	7	38	32
95th Queue (ft)	51	26	105	86
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				1
Queuing Penalty (veh)				8
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			1	
Queuing Penalty (veh)			0	

Intersection: 3: Columbia Way & Colombia Way

Movement	SB
Directions Served	R
Maximum Queue (ft)	6
Average Queue (ft)	0
95th Queue (ft)	4
Link Distance (ft)	664
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 28

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Denied Del/Veh (s)	0.3	2.5	0.0	0.0	0.0	0.0	0.0	3.0	0.4	0.5	0.4
Total Del/Veh (s)	38.0	10.6	98.4	25.4	7.3	1.3	0.2	7.8	2.8	0.9	5.0
Vehicles Exited	9	139	19	7	144	600	20	3	509	15	1465
Hourly Exit Rate	9	139	19	7	144	600	20	3	509	15	1465
Input Volume	10	143	20	7	151	609	21	5	510	14	1490
% of Volume	90	97	95	100	95	99	95	60	100	107	98

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	NBR	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.0	0.8	0.6	0.0	0.0	0.4
Total Del/Veh (s)	6.3	4.5	4.8	7.9	4.6	5.5	2.5	6.6
Vehicles Exited	24	26	18	739	12	628	42	1489
Hourly Exit Rate	24	26	18	739	12	628	42	1489
Input Volume	25	24	19	755	12	637	39	1511
% of Volume	96	108	95	98	100	99	108	99

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	EBT	NBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.1	0.1
Total Del/Veh (s)	0.5	0.1	1.4	54.9	23.7
Vehicles Exited	24	1	12	27	64
Hourly Exit Rate	24	1	12	27	64
Input Volume	26	1	12	27	66
% of Volume	92	100	100	100	97

Total Network Performance

Movement	EBL	EBT	NBT	SBR	All
Denied Del/Veh (s)				0.8	
Total Del/Veh (s)				13.3	
Vehicles Exited				1526	
Hourly Exit Rate				1526	
Input Volume				4611	
% of Volume				33	

Queuing and Blocking Report

Existing 2025 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	NB	SB	SB
Directions Served	LT	R	LTR	L	TR	L	TR
Maximum Queue (ft)	36	110	67	73	5	18	40
Average Queue (ft)	5	43	30	39	0	1	3
95th Queue (ft)	21	88	67	70	5	10	23
Link Distance (ft)	1155		52	54	54		1130
Upstream Blk Time (%)			22	4	0		
Queuing Penalty (veh)			6	17	0		
Storage Bay Dist (ft)		130				50	
Storage Blk Time (%)		0				0	0
Queuing Penalty (veh)		0				0	0

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	60	28	200	81
Average Queue (ft)	27	4	41	15
95th Queue (ft)	51	20	127	55
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				0
Queuing Penalty (veh)				2
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			2	
Queuing Penalty (veh)			0	

Intersection: 3: Columbia Way & Colombia Way

Movement	EB	NB	SB
Directions Served	L	T	R
Maximum Queue (ft)	3	3	97
Average Queue (ft)	0	0	18
95th Queue (ft)	3	3	100
Link Distance (ft)	52	113	664
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 26

Appendix D. Cumulative with Bypass Conditions Volumes Development

Tuolumne County GP - EX 2015 AM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL
1	198	438	13	11	523	21	14	14	1	231	11	3	9 1,473
2	80	626			679	74	18	18		42			1,519

Tuolumne County GP - EX 2015 PM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL
1	178	570	16	9	554	22	13	13	0	173	9	1	11 1,556
2	19	745			694	43	25	25		24			1,550

Tuolumne County GP - CUML 2040 AM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL
1	159	351	10	8	365	14	17	17	1	273	13	4	11 1,226
2	63	491			479	52	19	19		45			1,149

Tuolumne County GP - CUML 2040 PM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL
1	143	456	13	6	388	15	15	15	0	205	11	1	13 1,266
2	15	584			491	31	27	27		26			1,174

Sonora Downtown Plan - 2012 AM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	199	430	18	14	506	24	18	18	0	220	8	0	8 1,445

Sonora Downtown Plan - 2012 School Peak

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	167	551	29	37	574	30	20	20	1	222	9	1	9 1,650

Tuolumne County GP CUML 2040 AM Total / Sonora Downtown Plan 2012 AM Total**0.848443****Est. 2040 School Peak**

1	142	467	25	31	444	25	17	17	1	188	8	1	8 1,357
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Tuolumne County GP 2015 AM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	198	438	13	11	523	21	14	1	231	11	3	9	1,473
2	80	626	0	0	679	74	18	0	42	0	0	0	1,519

NDS 2025 AM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	207	396	17	12	614	28	16	0	231	13	1	3	1,538
2	77	603	10	0	774	84	17	0	40	0	0	0	1,605

Sonora Downtown Plan 2012 School Peak

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total	
1		167	551	29	37	574	30	20	1	222	9	1	9	1,650
2	29	706		17	15	660	56	32	1	35	0	0	0	1,551

NDS 2025 School Peak

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	151	588	22	19	444	14	13	1	185	11	0	6	1,454
2	30	728	13	0	590	50	33	0	36	0	0	0	1,480

Tuolumne County GP 2015 PM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	178	570	16	9	554	22	13	0	173	9	1	11	1,556
2	19	745	0	0	694	43	25	0	24	0	0	0	1,550

NDS 2025 PM

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	151	608	21	5	508	14	10	0	143	20	0	7	1,487
2	19	755	12	0	632	39	25	0	24	0	0	0	1,506

Sonora Downtown Plan 2012 School Peak

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	167	551	29	37	574	30	20	1	222	9	1	9	1,650
2	29	706	17	15	660	56	32	1	35	0	0	0	1,551

Est. 2040 School Peak

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	142	467	25	31	444	25	17	1	188	8	1	8	1,357
2	25	607	15	0	590	50	27	0	30	0	0	0	1,344

Year	Peak	Int #	Intersection	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
2025 AM		1	SR 49 & Shaws Flat Rd / Columbia Way	207	396	17	12	614	28	16	0	231	13	1	3	1,538
2040 AM		1	SR 49 & Shaws Flat Rd / Columbia Way	159	351	10	8	365	14	17	1	273	13	4	11	1,226
2025 PM		1	SR 49 & Shaws Flat Rd / Columbia Way	151	608	21	5	510	14	10	0	143	20	0	7	1,489
2040 PM		1	SR 49 & Shaws Flat Rd / Columbia Way	143	456	13	6	388	15	15	0	205	11	1	13	1,266
2025 SCHOOL		1	SR 49 & Shaws Flat Rd / Columbia Way	143	537	15	6	534	15	14	1	143	11	1	5	1,425
2040 SCHOOL		1	SR 49 & Shaws Flat Rd / Columbia Way	142	467	25	31	444	25	17	1	188	8	1	8	1,357
2025 AM		2	SR 49 & School St	84	603	0	0	774	84	17	0	44	0	0	0	1,606
2040 AM		2	SR 49 & School St	63	501	0	0	599	52	19	0	45	0	0	0	1,279
2025 PM		2	SR 49 & School St	19	755	12	0	634	39	25	0	24	0	0	0	1,508
2040 PM		2	SR 49 & School St	15	585	0	0	573	31	27	0	26	0	0	0	1,257
2025 SCHOOL		2	SR 49 & School St	27	665	9	0	634	54	30	0	33	0	0	0	1,452
2040 SCHOOL		2	SR 49 & School St	25	607	15	0	590	50	27	0	30	0	0	0	1,344

EX CONFIGURATION --> AWSC / SIGNAL / 4LegRBT

2025 AM

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	207	396	17	12	614	28	16	0	231	13	1	3	1,538
2	84	603	0	0	774	84	17	0	44	0	0	0	1,606

Int 2 - Removal

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1													
2		-84		0									

Int 2 - Reassign

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	84												
2			84				84						

Int 2 - EBT Removal

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2								0				

Int 2 - EBT Reassign

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1			0									
2							0					

Int 2 - SBL Removal

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1					0							
2				0								

Int 2 - SBL Reassign

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1				0								
2												

FUTURE AWSC / SIGNAL CONFIGURATION - 2040 AM

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	84	207	396	17	12	614	28	16	0	231	13	1	3	1,538
2	0	0	687	0	0	774	168	17	0	44	0	0	0	1,690

2025 SCHOOL

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	143	537	15	6	534	15	14	1	143	11	1	5	1,425
2	27	665	9	0	634	54	30	0	33	0	0	0	1,452

Int 2 - NBR Removal

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1													
2		-27		-9									

Int 2 - NBR Reassign

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	27												
2			27				27						

Int 2 - EBT Removal

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2								0				

Int 2 - EBT Reassign

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1			0									
2							0					

Int 2 - SBL Removal													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1						0							
2					0								
Int 2 - SBL Reassign													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1				0									
2													

FUTURE AWSC / SIGNAL CONFIGURATION - 2040 SCHOOL															
INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR		Total
1	27	143	537	15	6	534	15	14	1	143	11	1	5		1,425
2	0	0	692	0	0	634	81	30	0	33	0	0	0		1,470

2025 PM

EX CONFIGURATION													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	151	608	21	5	510	14	10	0	143	20	0	7	1,489
2	19	755	12	0	634	39	25	0	24	0	0	0	1,508

Int 2 - NBR Removal													
INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1													
2		-19		-12									
Int 2 - NBR Reassign													
INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	19												
2			19				19						

Int 2 - EBT Removal													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1													
2								0					
Int 2 - EBT Reassign													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1			0										
2							0						

Int 2 - SBL Removal													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1						0							
2					0								
Int 2 - SBL Reassign													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1				0									
2													

FUTURE AWSC / SIGNAL CONFIGURATION - 2040 PM															
INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR		Total
1	19	151	608	21	5	510	14	10	0	143	20	0	7		1,489
2	0	0	774	0	0	634	58	25	0	24	0	0	0		1,515

EX CONFIGURATION --> 5-LEG RBT

2025 AM

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	207	396	17	12	614	28	16	0	231	13	1	3	1,538
2	84	603	0	0	774	84	17	0	44	0	0	0	1,606

INT 1 SBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	554	60						

INT 1 EBR Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	208	23						

INT 1 WBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	12	1						

INT 2 NBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	201	385	17									
2												

INT 2 EBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	6	11	0									
2												

Int 1

INT ID	NB-SW	NB-NW	NB-N (NBT)	SB-S (SBT)	SB-SW	SB-NW	NEB-NW	NEB-N	NEB-S	SEB-N	SEB-S	SEB-SW	SWB-S	SWB-SW	SWB-NW	SWB-N
1	84	201	385	566	60	28	6	11	44	16	208	23	12	1	1	3

2025 SCHOOL

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	143	537	15	6	534	15	14	1	143	11	1	5	1,425
2	27	665	9	0	634	54	30	0	33	0	0	0	1,452

INT 1 SBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	492	42						

INT 1 EBR Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	132	11						

INT 1 WBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	10	1						

INT 2 NBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	137	513	15									
2												

INT 2 EBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	6	24	0									
2												

Int 1

INT ID	NB-SW	NB-NW	NB-N (NBT)	SB-S (SBT)	SB-SW	SB-NW	NEB-NW	NEB-N	NEB-S	SEB-N	SEB-S	SEB-SW	SWB-S	SWB-SW	SWB-NW	SWB-N
1	27	137	513	498	42	15	6	24	33	14	133	11	10	1	1	5

2025 PM

EX CONFIGURATION

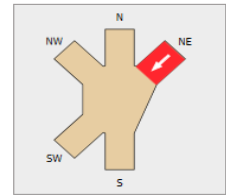
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	151	608	21	5	510	14	10	0	143	20	0	7	1,489
2	19	755	12	0	634	39	25	0	24	0	0	0	1,508

INT 1 SBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	480	30						

INT 1 EBR Proportioning Out

Approach Selector



INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	135	8						

INT 1 WBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	19	1						

INT 2 NBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	146	588	21									
2												

INT 2 EBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	5	20	0									
2												

Int 1

INT ID	NB-SW	NB-NW	NB-N (NBT)	SB-S (SBT)	SB-SW	SB-NW	NEB-NW	NEB-N	NEB-S	SEB-N	SEB-S	SEB-SW	SWB-S	SWB-SW	SWB-NW	SWB-N
1	19	146	588	485	30	14	5	20	24	10	135	8	19	1	0	7

EX CONFIGURATION --> AWSC / SIGNAL / 4LegRBT

2040 AM

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	159	351	10	8	365	14	17	1	273	13	4	11	1,226
2	63	501	0	0	599	52	19	0	45	0	0	0	1,279

Int 2 - Removal

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1													
2		-63		0									

Int 2 - Reassign

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	63												
2			63				63						

Int 2 - EBT Removal

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2								0				

Int 2 - EBT Reassign

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1			0									
2							0					

Int 2 - SBL Removal

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1						0						
2				0								

Int 2 - SBL Reassign

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1				0								
2												

FUTURE AWSC / SIGNAL CONFIGURATION - 2040 AM

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	63	159	351	10	8	365	14	17	1	273	13	4	11	1,226
2	0	0	564	0	0	599	115	19	0	45	0	0	0	1,342

2040 SCHOOL

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	142	467	25	31	444	25	17	1	188	8	1	8	1,357
2	25	607	15	0	590	50	27	0	30	0	0	0	1,344

Int 2 - NBR Removal

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1													
2		-25		-15									

Int 2 - NBR Reassign

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	25												
2			25				25						

Int 2 - EBT Removal

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2								0				

Int 2 - EBT Reassign

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1			0									
2							0					

Int 2 - SBL Removal													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1						0							
2					0								
Int 2 - SBL Reassign													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1				0									
2													

FUTURE AWSC / SIGNAL CONFIGURATION - 2040 SCHOOL

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	25	142	467	25	31	444	25	17	1	188	8	1	8	1,357
2	0	0	632	0	0	590	75	27	0	30	0	0	0	1,354

2040 PM

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	143	456	13	6	388	15	15	0	205	11	1	13	1,266
2	15	585	0	0	573	31	27	0	26	0	0	0	1,257

Int 2 - NBR Removal													
INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1													
2		-15		0									
Int 2 - NBR Reassign													
INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	15												
2			15				15						

Int 2 - EBT Removal													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1													
2								0					
Int 2 - EBT Reassign													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1			0										
2							0						

Int 2 - SBL Removal													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1						0							
2					0								
Int 2 - SBL Reassign													
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	
1				0									
2													

FUTURE AWSC / SIGNAL CONFIGURATION - 2040 PM

INT ID	NBU	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	15	143	456	13	6	388	15	15	0	205	11	1	13	1,266
2	0	0	600	0	0	573	46	27	0	26	0	0	0	1,272

EX CONFIGURATION --> 5-LEG RBT

2040 AM

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	159	351	10	8	365	14	17	1	273	13	4	11	1,226
2	63	501	0	0	599	52	19	0	45	0	0	0	1,279

INT 1 SBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	336	29						

INT 1 EBR Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	251	22						

INT 1 WBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	12	1						

INT 2 NBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	153	338	10									
2												

INT 2 EBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	6	13	0									
2												

Int 1

INT ID	NB-SW	NB-NW	NB-N (NBT)	SB-S (SBT)	SB-SW	SB-NW	NEB-NW	NEB-N	NEB-S	SEB-N	SEB-S	SEB-SW	SWB-S	SWB-SW	SWB-NW	SWB-N
1	63	153	338	344	29	14	6	13	45	17	252	22	12	1	4	11

2040 SCHOOL

EX CONFIGURATION

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	142	467	25	31	444	25	17	1	188	8	1	8	1,357
2	25	607	15	0	590	50	27	0	30	0	0	0	1,344

INT 1 SBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	409	35						

INT 1 EBR Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	173	15						

INT 1 WBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	7	1						

INT 2 NBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	136	446	25									
2												

INT 2 EBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	6	21	0									
2												

Int 1

INT ID	NB-SW	NB-NW	NB-N (NBT)	SB-S (SBT)	SB-SW	SB-NW	NEB-NW	NEB-N	NEB-S	SEB-N	SEB-S	SEB-SW	SWB-S	SWB-SW	SWB-NW	SWB-N
1	25	136	446	440	35	25	6	21	30	17	174	15	7	1	1	8

2040 PM

EX CONFIGURATION

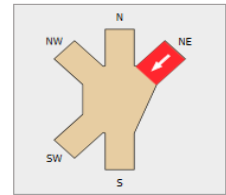
INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total
1	143	456	13	6	388	15	15	0	205	11	1	13	1,266
2	15	585	0	0	573	31	27	0	26	0	0	0	1,257

INT 1 SBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	368	20						

INT 1 EBR Proportioning Out

Approach Selector



INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	194	11						

INT 1 WBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1												
2				0	10	1						

INT 2 NBT Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	137	435	13									
2												

INT 2 EBL Proportioning Out

INT ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
1	6	21	0									
2												

Int 1

INT ID	NB-SW	NB-NW	NB-N (NBT)	SB-S (SBT)	SB-SW	SB-NW	NEB-NW	NEB-N	NEB-S	SEB-N	SEB-S	SEB-SW	SWB-S	SWB-SW	SWB-NW	SWB-N
1	15	137	435	374	20	15	6	21	26	15	194	11	10	1	1	13

Appendix E. Cumulative (2040) with Bypass No Build Conditions SimTraffic Outputs

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.9	0.2	2.7	0.0	0.0	5.3	0.0	0.0	0.0	3.4	0.9	0.5
Total Del/Veh (s)	28.2	17.4	28.9	59.7	20.3	12.0	6.7	1.9	0.4	4.9	3.7	1.2
Vehicles Exited	17	1	271	11	4	13	164	405	10	8	358	14
Hourly Exit Rate	17	1	271	11	4	13	164	405	10	8	358	14
Input Volume	17	1	273	13	5	11	159	389	10	8	365	14
% of Volume	100	100	99	85	80	118	103	104	100	100	98	100

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.9
Total Del/Veh (s)	9.8
Vehicles Exited	1276
Hourly Exit Rate	1276
Input Volume	1265
% of Volume	101

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.4	0.4	3.1	0.3	0.2	0.0	0.4
Total Del/Veh (s)	14.0	8.7	4.3	6.1	7.0	4.5	6.8
Vehicles Exited	39	88	64	508	593	52	1344
Hourly Exit Rate	39	88	64	508	593	52	1344
Input Volume	19	45	63	501	605	52	1285
% of Volume	205	196	102	101	98	100	105

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	SBR	All
Denied Del/Veh (s)	0.0	0.1	0.1
Total Del/Veh (s)	0.7	10.0	6.3
Vehicles Exited	19	28	47
Hourly Exit Rate	19	28	47
Input Volume	19	28	47
% of Volume	100	100	100

Total Network Performance

Denied Del/Veh (s)	1.2
Total Del/Veh (s)	16.7
Vehicles Exited	1431
Hourly Exit Rate	1431
Input Volume	3931
% of Volume	36

Queuing and Blocking Report

Future 2045 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	NB	SB	SB
Directions Served	LT	R	LTR	L	TR	L	TR
Maximum Queue (ft)	328	196	67	120	103	26	50
Average Queue (ft)	45	106	25	47	4	3	5
95th Queue (ft)	203	197	59	92	42	16	26
Link Distance (ft)	1155		52	54	54		1130
Upstream Blk Time (%)			8	6			
Queuing Penalty (veh)			2	15			
Storage Bay Dist (ft)		130				50	
Storage Blk Time (%)	0	15					0
Queuing Penalty (veh)	1	3					0

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	196	48	103	101
Average Queue (ft)	86	17	30	30
95th Queue (ft)	153	39	78	71
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				1
Queuing Penalty (veh)				6
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			1	
Queuing Penalty (veh)			0	

Intersection: 3: Columbia Way & Colombia Way

Movement	SB
Directions Served	R
Maximum Queue (ft)	62
Average Queue (ft)	6
95th Queue (ft)	35
Link Distance (ft)	664
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 26

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.4	0.1	2.4	0.0	0.0	0.0	0.0	0.0	0.0	2.9	0.4	0.4
Total Del/Veh (s)	27.2	19.4	13.2	48.0	16.9	10.7	6.1	1.3	0.3	4.5	2.9	1.2
Vehicles Exited	17	2	180	7	1	10	138	459	26	34	432	24
Hourly Exit Rate	17	2	180	7	1	10	138	459	26	34	432	24
Input Volume	17	1	188	8	2	8	142	467	25	31	444	25
% of Volume	100	200	96	88	50	125	97	98	104	110	97	96

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.6
Total Del/Veh (s)	4.7
Vehicles Exited	1330
Hourly Exit Rate	1330
Input Volume	1358
% of Volume	98

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	NBR	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.1	0.6	0.8	0.0	0.1	0.4
Total Del/Veh (s)	7.2	5.4	5.1	7.0	3.2	6.2	3.3	6.4
Vehicles Exited	26	29	25	597	16	570	53	1316
Hourly Exit Rate	26	29	25	597	16	570	53	1316
Input Volume	27	30	25	607	15	595	50	1349
% of Volume	96	97	100	98	107	96	106	98

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	NBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.1	0.0
Total Del/Veh (s)	0.8	1.5	4.5	1.6
Vehicles Exited	63	16	18	97
Hourly Exit Rate	63	16	18	97
Input Volume	57	15	17	90
% of Volume	111	107	106	108

Total Network Performance

Denied Del/Veh (s)	0.9
Total Del/Veh (s)	11.7
Vehicles Exited	1402
Hourly Exit Rate	1402
Input Volume	4224
% of Volume	33

Queuing and Blocking Report

Future 2045 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	SB	SB
Directions Served	LT	R	LTR	L	L	TR
Maximum Queue (ft)	95	151	58	72	40	67
Average Queue (ft)	13	57	16	39	11	7
95th Queue (ft)	55	117	45	70	34	36
Link Distance (ft)	1155		52	54		1130
Upstream Blk Time (%)			3	3		
Queuing Penalty (veh)			0	10		
Storage Bay Dist (ft)		130			50	
Storage Blk Time (%)	0	1			0	0
Queuing Penalty (veh)	0	0			0	0

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	62	33	126	108
Average Queue (ft)	28	7	35	26
95th Queue (ft)	53	26	93	79
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				1
Queuing Penalty (veh)				10
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			1	
Queuing Penalty (veh)			0	

Intersection: 3: Columbia Way & Colombia Way

Movement	EB	NB	SB
Directions Served	L	T	R
Maximum Queue (ft)	6	9	19
Average Queue (ft)	0	0	1
95th Queue (ft)	4	5	15
Link Distance (ft)	52	113	664
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 21

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Denied Del/Veh (s)	0.4	2.4	0.0	0.0	0.0	0.0	0.0	0.0	3.2	0.3	0.3	0.5
Total Del/Veh (s)	20.5	11.5	44.4	11.2	8.9	4.7	1.2	0.2	4.4	2.5	0.7	4.4
Vehicles Exited	14	207	11	1	12	143	459	14	5	376	16	1258
Hourly Exit Rate	14	207	11	1	12	143	459	14	5	376	16	1258
Input Volume	15	205	11	1	13	143	457	13	6	388	15	1267
% of Volume	93	101	100	100	92	100	100	108	83	97	107	99

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.4	0.5	0.0	0.2	0.3
Total Del/Veh (s)	6.6	4.5	4.3	6.6	5.8	3.3	6.1
Vehicles Exited	26	25	16	588	566	34	1255
Hourly Exit Rate	26	25	16	588	566	34	1255
Input Volume	27	26	15	585	577	31	1261
% of Volume	96	96	107	101	98	110	100

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	SBR	All
Denied Del/Veh (s)	0.0	0.1	0.1
Total Del/Veh (s)	0.6	3.8	2.4
Vehicles Exited	19	24	43
Hourly Exit Rate	19	24	43
Input Volume	19	25	44
% of Volume	100	96	98

Total Network Performance

Movement	All
Denied Del/Veh (s)	0.8
Total Del/Veh (s)	11.1
Vehicles Exited	1306
Hourly Exit Rate	1306
Input Volume	3879
% of Volume	34

Queuing and Blocking Report

Future 2045 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	SB	SB
Directions Served	LT	R	LTR	L	L	TR
Maximum Queue (ft)	39	149	58	73	23	64
Average Queue (ft)	8	60	19	35	2	5
95th Queue (ft)	26	117	50	65	13	30
Link Distance (ft)	1155		52	54		1130
Upstream Blk Time (%)			5	2		
Queuing Penalty (veh)			1	7		
Storage Bay Dist (ft)		130			50	
Storage Blk Time (%)		1				0
Queuing Penalty (veh)		0				0

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	64	38	138	89
Average Queue (ft)	27	4	27	15
95th Queue (ft)	53	22	87	55
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				0
Queuing Penalty (veh)				3
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			1	
Queuing Penalty (veh)			0	

Intersection: 3: Columbia Way & Colombia Way

Movement	SB
Directions Served	R
Maximum Queue (ft)	33
Average Queue (ft)	2
95th Queue (ft)	20
Link Distance (ft)	664
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 11

Appendix F. Cumulative (2040) with Bypass Build Conditions with Signal – SimTraffic Outputs

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.6	0.8	2.7	0.0	0.0	0.1	0.0	0.0	0.0	3.2	0.3	0.4
Total Del/Veh (s)	12.4	13.1	6.8	12.9	13.7	3.7	16.7	5.3	2.7	27.2	15.4	10.5
Vehicles Exited	14	1	270	12	4	13	160	346	9	6	364	14
Hourly Exit Rate	14	1	270	12	4	13	160	346	9	6	364	14
Input Volume	17	1	273	13	5	11	159	351	10	8	365	14
% of Volume	82	100	99	92	80	118	101	99	90	75	100	100

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.7
Total Del/Veh (s)	10.5
Vehicles Exited	1213
Hourly Exit Rate	1213
Input Volume	1227
% of Volume	99

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.2	0.6	0.0	0.0	0.4
Total Del/Veh (s)	7.9	4.8	5.6	9.9	5.4	2.5	7.1
Vehicles Exited	16	43	62	498	600	52	1271
Hourly Exit Rate	16	43	62	498	600	52	1271
Input Volume	19	45	63	501	605	52	1285
% of Volume	84	96	98	99	99	100	99

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	SBR	All
Denied Del/Veh (s)	0.0	0.1	0.1
Total Del/Veh (s)	0.9	0.4	0.6
Vehicles Exited	15	27	42
Hourly Exit Rate	15	27	42
Input Volume	19	28	47
% of Volume	79	96	89

Total Network Performance

Denied Del/Veh (s)	1.1
Total Del/Veh (s)	18.0
Vehicles Exited	1322
Hourly Exit Rate	1322
Input Volume	3893
% of Volume	34

Queuing and Blocking Report
Future 2045 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	NB	SB	SB
Directions Served	LT	R	LTR	L	TR	L	TR
Maximum Queue (ft)	51	142	59	76	75	52	228
Average Queue (ft)	7	56	16	58	54	6	106
95th Queue (ft)	35	106	46	84	87	31	187
Link Distance (ft)	1155		52	54	54		1130
Upstream Blk Time (%)			1	18	10		
Queuing Penalty (veh)			0	46	26		
Storage Bay Dist (ft)		130				50	
Storage Blk Time (%)		0				0	25
Queuing Penalty (veh)		0				1	2

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	65	96	244	103
Average Queue (ft)	29	19	67	26
95th Queue (ft)	55	58	179	80
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				1
Queuing Penalty (veh)				9
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			6	
Queuing Penalty (veh)			4	

Intersection: 3: Columbia Way & Colombia Way

Movement	SB
Directions Served	R
Maximum Queue (ft)	25
Average Queue (ft)	1
95th Queue (ft)	15
Link Distance (ft)	664
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 88

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.5	0.3	2.8	0.0	0.0	0.3	0.0	0.0	0.0	3.0	0.5	0.4
Total Del/Veh (s)	19.5	12.8	7.0	15.6	9.7	4.6	20.4	5.1	2.6	28.6	14.0	7.9
Vehicles Exited	18	1	187	7	1	9	142	460	43	29	443	25
Hourly Exit Rate	18	1	187	7	1	9	142	460	43	29	443	25
Input Volume	17	1	188	8	2	8	142	467	40	31	444	25
% of Volume	106	100	99	88	50	112	100	99	108	94	100	100

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.6
Total Del/Veh (s)	10.6
Vehicles Exited	1365
Hourly Exit Rate	1365
Input Volume	1373
% of Volume	99

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.3	0.6	0.0	0.0	0.4
Total Del/Veh (s)	10.1	5.9	12.1	16.9	6.0	3.5	11.1
Vehicles Exited	23	32	24	622	593	49	1343
Hourly Exit Rate	23	32	24	622	593	49	1343
Input Volume	27	30	25	622	595	50	1349
% of Volume	85	107	96	100	100	98	100

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	NBT	SBR	All
Denied Del/Veh (s)	0.0	0.1	0.1	0.0
Total Del/Veh (s)	0.9	0.5	0.1	0.7
Vehicles Exited	74	12	15	101
Hourly Exit Rate	74	12	15	101
Input Volume	57	15	17	104
% of Volume	130	80	88	97

Total Network Performance

Denied Del/Veh (s)	0.9
Total Del/Veh (s)	22.0
Vehicles Exited	1439
Hourly Exit Rate	1439
Input Volume	4253
% of Volume	34

Queuing and Blocking Report

Future 2045 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	NB	SB	SB
Directions Served	LT	R	LTR	L	TR	L	TR
Maximum Queue (ft)	36	108	46	75	76	95	259
Average Queue (ft)	8	44	11	57	61	23	117
95th Queue (ft)	26	88	35	84	87	62	210
Link Distance (ft)	1155		52	54	54		1130
Upstream Blk Time (%)			0	19	14		
Queuing Penalty (veh)			0	63	45		
Storage Bay Dist (ft)		130				50	
Storage Blk Time (%)		0				3	24
Queuing Penalty (veh)		0				12	7

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	70	115	400	130
Average Queue (ft)	29	15	133	37
95th Queue (ft)	58	68	342	105
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)			0	3
Queuing Penalty (veh)			0	18
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			18	
Queuing Penalty (veh)			4	

Intersection: 3: Columbia Way & Colombia Way

Movement	SB
Directions Served	R
Maximum Queue (ft)	3
Average Queue (ft)	0
95th Queue (ft)	3
Link Distance (ft)	664
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 150

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Denied Del/Veh (s)	0.4	2.7	0.0	0.0	0.0	0.0	0.0	0.0	3.0	0.4	0.4	0.6
Total Del/Veh (s)	13.8	5.7	14.8	10.3	4.9	17.2	4.7	3.0	27.2	14.2	7.8	9.5
Vehicles Exited	15	206	12	1	14	143	464	12	5	394	16	1282
Hourly Exit Rate	15	206	12	1	14	143	464	12	5	394	16	1282
Input Volume	15	205	11	1	13	143	457	13	6	388	15	1267
% of Volume	100	100	109	100	108	100	102	92	83	102	107	101

2: N Washington St/SR 49 & W School St/Columbia Way Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	3.3	0.6	0.0	0.0	0.3
Total Del/Veh (s)	7.5	4.2	5.9	11.6	5.3	2.3	8.2
Vehicles Exited	26	25	17	592	582	35	1277
Hourly Exit Rate	26	25	17	592	582	35	1277
Input Volume	27	26	15	585	577	31	1261
% of Volume	96	96	113	101	101	113	101

3: Columbia Way & Colombia Way Performance by movement

Movement	EBL	SBR	All
Denied Del/Veh (s)	0.0	0.1	0.1
Total Del/Veh (s)	0.8	0.4	0.5
Vehicles Exited	18	27	45
Hourly Exit Rate	18	27	45
Input Volume	19	25	44
% of Volume	95	108	102

Total Network Performance

Movement	All
Denied Del/Veh (s)	0.9
Total Del/Veh (s)	18.5
Vehicles Exited	1331
Hourly Exit Rate	1331
Input Volume	3879
% of Volume	34

Queuing and Blocking Report

Future 2045 Conditions

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	EB	WB	NB	NB	SB	SB
Directions Served	LT	R	LTR	L	TR	L	TR
Maximum Queue (ft)	27	115	59	74	75	56	222
Average Queue (ft)	6	43	16	55	60	5	103
95th Queue (ft)	21	87	46	83	86	29	191
Link Distance (ft)	1155		52	54	54		1130
Upstream Blk Time (%)			1	16	12		
Queuing Penalty (veh)			0	48	36		
Storage Bay Dist (ft)		130				50	
Storage Blk Time (%)		0				0	23
Queuing Penalty (veh)		0				0	1

Intersection: 2: N Washington St/SR 49 & W School St/Columbia Way

Movement	EB	NB	NB	SB
Directions Served	LTR	L	TR	TR
Maximum Queue (ft)	57	50	317	95
Average Queue (ft)	27	5	92	24
95th Queue (ft)	50	29	235	79
Link Distance (ft)	1136		757	54
Upstream Blk Time (%)				1
Queuing Penalty (veh)				7
Storage Bay Dist (ft)		80		
Storage Blk Time (%)			9	
Queuing Penalty (veh)			1	

Intersection: 3: Columbia Way & Colombia Way

Movement	SB
Directions Served	R
Maximum Queue (ft)	18
Average Queue (ft)	1
95th Queue (ft)	15
Link Distance (ft)	664
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 94

**Appendix G. Cumulative (2040) with Bypass Build Conditions with 4-Legged Roundabout
– SIDRA and SimTraffic Outputs**

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road AM (Site Folder: AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h]	[HV %]	[Total veh/h]	[HV %]						[Veh ft]	[Dist ft]				
South: RoadName															
Lane 1 ^d	634	3.0	634	3.0	1253	0.506	100	7.4	LOS A	4.7	119.3	Full	100	0.0	10.5
Approach	634	3.0	634	3.0		0.506		7.4	LOS A	4.7	119.3				
East: RoadName															
Lane 1 ^d	30	3.0	30	3.0	725	0.042	100	5.4	LOS A	0.2	5.7	Full	780	0.0	0.0
Approach	30	3.0	30	3.0		0.042		5.4	LOS A	0.2	5.7				
North: RoadName															
Lane 1 ^d	421	3.0	421	3.0	1011	0.416	100	8.1	LOS A	2.8	70.6	Full	1293	0.0	0.0
Approach	421	3.0	421	3.0		0.416		8.1	LOS A	2.8	70.6				
West: RoadName															
Lane 1 ^d	316	3.0	316	3.0	806	0.392	100	9.2	LOS A	2.6	66.3	Full	1180	0.0	0.0
Approach	316	3.0	316	3.0		0.392		9.2	LOS A	2.6	66.3				
All Vehicles	1401	3.0	1401	3.0		0.506		8.0	LOS A	4.7	119.3				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)												
South: RoadName												
Mov.	U	L2	T1	R2	Total	%HV						
From S							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N	E			veh/h	Satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	68	173	382	11	634	3.0	1253	0.506	100	NA	NA	
Approach	68	173	382	11	634	3.0		0.506				
East: RoadName												
Mov.	L2	T1	R2	Total	%HV							
From E							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N				veh/h	Satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	14	4	12	30	3.0		725	0.042	100	NA	NA	
Approach	14	4	12	30	3.0			0.042				

North: RoadName										
Mov.	L2	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.
From N						Cap.	Satn	Util.	SL Ov.	Lane
To Exit:	E	S	W			veh/h	v/c	%	%	No.
Lane 1	9	397	15	421	3.0	1011	0.416	100	NA	NA
Approach	9	397	15	421	3.0		0.416			
West: RoadName										
Mov.	L2	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.
From W						Cap.	Satn	Util.	SL Ov.	Lane
To Exit:	N	E	S			veh/h	v/c	%	%	No.
Lane 1	18	1	297	316	3.0	806	0.392	100	NA	NA
Approach	18	1	297	316	3.0		0.392			
Total %HV Deg.Satn (v/c)										
All Vehicles	1401	3.0		0.506						

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane	Opposing Flow Rate % veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Capacity Flow Rate veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec	
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
East: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

Site: 101v [School St & N Washington St AM (Site Folder: AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Stop (Two-Way)

Lane Use and Performance																
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.	
	[Total veh/h	HV] %	[Total veh/h	HV] %						[Veh	Dist] ft					
South: RoadName																
Lane 1	615	3.0	615	3.0	1894	0.325	100	2.6	LOS A	0.1	1.7	Full	404	0.0	0.0	
Approach	615	3.0	615	3.0		0.325		2.6	NA	0.1	1.7					
North: RoadName																
Lane 1	776	3.0	776	3.0	1864	0.416	100	0.0	LOS A	0.0	0.0	Full	100	0.0	0.0	
Approach	776	3.0	776	3.0		0.416		0.0	NA	0.0	0.0					
West: RoadName																
Lane 1	70	3.0	70	3.0	105	0.665	100	88.1	LOS F	2.2	57.2	Full	500	0.0	0.0	
Approach	70	3.0	70	3.0		0.665		88.1	LOS F	2.2	57.2					
All Vehicles	1461	3.0	1461	3.0		0.665		7.1	NA	2.2	57.2					

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard (HCM).

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Values used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Approach Lane Flows (veh/h)										
South: RoadName										
Mov.	U	L2	T1	Total	%HV		Deg.	Lane	Prob.	Ov.
From S							Satn	Util.	SL Ov.	Lane
To Exit:	S	W	N			Cap.	v/c	%	%	No.
Lane 1	1	1	613	615	3.0	1894	0.325	100	NA	NA
Approach	1	1	613	615	3.0		0.325			
North: RoadName										
Mov.	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.	
From N						Satn	Util.	SL Ov.	Lane	
To Exit:	S	W				v/c	%	%	No.	
Lane 1	651	125	776	3.0		1864	0.416	100	NA	NA
Approach	651	125	776	3.0			0.416			
West: RoadName										
Mov.	L2	R2	Total	%HV		Deg.	Lane	Prob.	Ov.	
From W						Satn	Util.	SL Ov.	Lane	
To Exit:	N	S				v/c	%	%	No.	

Lane 1	21	49	70	3.0	105	0.665	100	NA	NA
Approach	21	49	70	3.0		0.665			
Total %HV Deg.Satn (v/c)									
All Vehicles	1461	3.0		0.665					

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis											
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane % veh/h	Opposing Flow Rate pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.											

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road School (Site Folder: School)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: RoadName															
Lane 1 ^d	716	3.0	716	3.0	1221	0.587	100	9.2	LOS A	5.8	148.6	Full	100	0.0	18.3
Approach	716	3.0	716	3.0		0.587		9.2	LOS A	5.8	148.6				
East: RoadName															
Lane 1 ^d	18	3.0	18	3.0	658	0.028	100	5.8	LOS A	0.2	4.0	Full	780	0.0	0.0
Approach	18	3.0	18	3.0		0.028		5.8	LOS A	0.2	4.0				
North: RoadName															
Lane 1 ^d	543	3.0	543	3.0	1066	0.510	100	9.2	LOS A	3.9	100.7	Full	1293	0.0	0.0
Approach	543	3.0	543	3.0		0.510		9.2	LOS A	3.9	100.7				
West: RoadName															
Lane 1 ^d	224	3.0	224	3.0	746	0.300	100	8.3	LOS A	1.9	48.5	Full	1180	0.0	0.0
Approach	224	3.0	224	3.0		0.300		8.3	LOS A	1.9	48.5				
All Vehicles	1502	3.0	1502	3.0		0.587		9.0	LOS A	5.8	148.6				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)												
South: RoadName												
Mov.	U	L2	T1	R2	Total	%HV						
From S							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N	E			veh/h	satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	27	154	508	27	716	3.0	1221	0.587	100	NA	NA	
Approach	27	154	508	27	716	3.0		0.587				
East: RoadName												
Mov.	L2	T1	R2	Total	%HV							
From E							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N				veh/h	satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	9	1	9	18	3.0		658	0.028	100	NA	NA	

Approach	9	1	9	18	3.0		0.028				
North: RoadName											
Mov.	L2	T1	R2	Total	%HV						
From N To Exit:	E	S	W			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	34	483	27	543	3.0	1066	0.510	100	NA	NA	
Approach	34	483	27	543	3.0		0.510				
West: RoadName											
Mov.	L2	T1	R2	Total	%HV						
From W To Exit:	N	E	S			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	18	1	204	224	3.0	746	0.300	100	NA	NA	
Approach	18	1	204	224	3.0		0.300				
Total %HV Deg.Satn (v/c)											
All Vehicles	1502	3.0		0.587							

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Capacity Flow Rate veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec		
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
East: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

Site: 101v [School St & N Washington St School (Site Folder: School)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Stop (Two-Way)

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV] %	[Total veh/h	HV] %						[Veh	Dist] ft				
South: RoadName															
Lane 1	689	3.0	689	3.0	1898	0.363	100	3.0	LOS A	0.1	1.6	Full	404	0.0	0.0
Approach	689	3.0	689	3.0		0.363		3.0	NA	0.1	1.6				
North: RoadName															
Lane 1	723	3.0	723	3.0	1880	0.384	100	0.0	LOS A	0.0	0.0	Full	100	0.0	0.0
Approach	723	3.0	723	3.0		0.384		0.0	NA	0.0	0.0				
West: RoadName															
Lane 1	62	3.0	62	3.0	74	0.840	100	152.8	LOS F	3.0	77.5	Full	500	0.0	0.0
Approach	62	3.0	62	3.0		0.840		152.8	LOS F	3.0	77.5				
All Vehicles	1474	3.0	1474	3.0		0.840		9.3	NA	3.0	77.5				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard (HCM).

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Values used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Approach Lane Flows (veh/h)										
South: RoadName										
Mov.	U	L2	T1	Total	%HV		Deg.	Lane	Prob.	Ov.
From S							Satn	Util.	SL Ov.	Lane
To Exit:	S	W	N			Cap.	v/c	%	%	No.
Lane 1	1	1	687	689	3.0	1898	0.363	100	NA	NA
Approach	1	1	687	689	3.0		0.363			
North: RoadName										
Mov.	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.	
From N						Satn	Util.	SL Ov.	Lane	
To Exit:	S	W				v/c	%	%	No.	
Lane 1	641	82	723	3.0		1880	0.384	100	NA	NA
Approach	641	82	723	3.0			0.384			
West: RoadName										
Mov.	L2	R2	Total	%HV		Deg.	Lane	Prob.	Ov.	
From W						Satn	Util.	SL Ov.	Lane	
To Exit:	N	S				v/c	%	%	No.	

Lane 1	29	33	62	3.0	74	0.840	100	NA	NA
Approach	29	33	62	3.0	0.840				
Total %HV Deg.Satn (v/c)									
All Vehicles	1474	3.0		0.840					

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis											
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane % veh/h	Opposing Flow Rate pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Capacity Flow Rate veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec	
There are no Exit Short Lanes for Merge Analysis at this Site.											

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road PM (Site Folder: PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h]	[HV %]	[Total veh/h]	[HV %]						[Veh]	[Dist]				
South: RoadName															
Lane 1 ^d	682	3.0	682	3.0	1259	0.541	100	7.7	LOS A	5.3	135.9	Full	100	0.0	14.9
Approach	682	3.0	682	3.0		0.541		7.7	LOS A	5.3	135.9				
East: RoadName															
Lane 1 ^d	27	3.0	27	3.0	698	0.039	100	5.6	LOS A	0.2	5.3	Full	780	0.0	0.0
Approach	27	3.0	27	3.0		0.039		5.6	LOS A	0.2	5.3				
North: RoadName															
Lane 1 ^d	445	3.0	445	3.0	1080	0.412	100	7.6	LOS A	2.8	71.1	Full	1293	0.0	0.0
Approach	445	3.0	445	3.0		0.412		7.6	LOS A	2.8	71.1				
West: RoadName															
Lane 1 ^d	240	3.0	240	3.0	832	0.289	100	7.5	LOS A	1.8	45.5	Full	1180	0.0	0.0
Approach	240	3.0	240	3.0		0.289		7.5	LOS A	1.8	45.5				
All Vehicles	1393	3.0	1393	3.0		0.541		7.6	LOS A	5.3	135.9				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)												
South: RoadName												
Mov.	U	L2	T1	R2	Total	%HV						
From S							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N	E			veh/h	Satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	16	155	496	14	682	3.0	1259	0.541	100	NA	NA	
Approach	16	155	496	14	682	3.0		0.541				
East: RoadName												
Mov.	L2	T1	R2	Total	%HV							
From E							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N				veh/h	Satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	12	1	14	27	3.0		698	0.039	100	NA	NA	
Approach	12	1	14	27	3.0			0.039				

North: RoadName										
Mov.	L2	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.
From N						Cap.	Satn	Util.	SL Ov.	Lane
To Exit:	E	S	W			veh/h	v/c	%	%	No.
Lane 1	7	422	16	445	3.0	1080	0.412	100	NA	NA
Approach	7	422	16	445	3.0		0.412			
West: RoadName										
Mov.	L2	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.
From W						Cap.	Satn	Util.	SL Ov.	Lane
To Exit:	N	E	S			veh/h	v/c	%	%	No.
Lane 1	16	1	223	240	3.0	832	0.289	100	NA	NA
Approach	16	1	223	240	3.0		0.289			
Total %HV Deg.Satn (v/c)										
All Vehicles	1393	3.0		0.541						

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane %	Opposing Flow Rate veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Capacity Flow Rate veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec	
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
East: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

 **Site: 101v [School St & N Washington St PM (Site Folder: PM)]**

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Stop (Two-Way)

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: RoadName															
Lane 1	654	3.0	654	3.0	1898	0.345	100	2.8	LOS A	0.1	1.5	Full	404	0.0	0.0
Approach	654	3.0	654	3.0		0.345		2.8	NA	0.1	1.5				
North: RoadName															
Lane 1	673	3.0	673	3.0	1893	0.355	100	0.0	LOS A	0.0	0.0	Full	100	0.0	0.0
Approach	673	3.0	673	3.0		0.355		0.0	NA	0.0	0.0				
West: RoadName															
Lane 1	58	3.0	58	3.0	78	0.740	100	125.9	LOS F	2.3	58.0	Full	500	0.0	0.0
Approach	58	3.0	58	3.0		0.740		125.9	LOS F	2.3	58.0				
All Vehicles	1385	3.0	1385	3.0		0.740		7.8	NA	2.3	58.0				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard (HCM).

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Approach Lane Flows (veh/h)											
South: RoadName											
Mov.	U	L2	T1	Total	%HV		Deg. Satn	Lane Util.	Prob. SL	Ov. Lane	
From S To Exit:	S	W	N			Cap. veh/h	v/c	%	%	No.	
Lane 1	1	1	652	654	3.0	1898	0.345	100	NA	NA	
Approach	1	1	652	654	3.0		0.345				
North: RoadName											
Mov.	T1	R2	Total	%HV		Deg. Satn	Lane Util.	Prob. SL	Ov. Lane		
From N To Exit:	S	W			Cap. veh/h	v/c	%	%	%	No.	
Lane 1	623	50	673	3.0		1893	0.355	100	NA	NA	
Approach	623	50	673	3.0			0.355				
West: RoadName											
Mov.	L2	R2	Total	%HV		Deg. Satn	Lane Util.	Prob. SL	Ov. Lane		
From W To Exit:	N	S			Cap. veh/h	v/c	%	%	%	No.	

Lane 1	29	28	58	3.0	78	0.740	100	NA	NA
Approach	29	28	58	3.0	0.740				
Total %HV Deg.Satn (v/c)									
All Vehicles	1385	3.0		0.740					

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis											
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane % veh/h	Opposing Flow Rate pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.											

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Denied Del/Veh (s)	0.3	0.3	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.3
Total Del/Veh (s)	4.6	3.7	4.2	3.6	4.8	3.9	1.5	1.5	2.5	1.4	5.6	7.2
Vehicles Exited	16	1	270	10	4	11	65	156	354	12	8	358
Hourly Exit Rate	16	1	270	10	4	11	65	156	354	12	8	358
Input Volume	17	1	273	13	4	11	63	159	351	10	8	365
% of Volume	94	100	99	77	100	100	103	98	101	120	100	98

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	SBR	All
Denied Del/Veh (s)	0.3	0.2
Total Del/Veh (s)	5.6	4.1
Vehicles Exited	13	1278
Hourly Exit Rate	13	1278
Input Volume	14	1289
% of Volume	93	99

2: N Washington St/SR 49 & W School St Performance by movement

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.4	0.0	0.1	0.2
Total Del/Veh (s)	5.8	4.3	6.1	2.2	1.4	3.9
Vehicles Exited	20	42	567	593	119	1341
Hourly Exit Rate	20	42	567	593	119	1341
Input Volume	19	45	564	607	115	1350
% of Volume	105	93	101	98	103	99

Total Network Performance

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)			0.4			
Total Del/Veh (s)			8.8			
Vehicles Exited			1329			
Hourly Exit Rate			1329			
Input Volume			4020			
% of Volume			33			

Queuing and Blocking Report
Future 2045 AM RAB

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	WB	B3	NB	NB	SB
Directions Served	LTR	LTR	T	UL	TR	LTR
Maximum Queue (ft)	92	78	2	42	48	131
Average Queue (ft)	41	13	0	5	7	48
95th Queue (ft)	76	47	2	25	31	99
Link Distance (ft)	1149	64	667	50	50	1131
Upstream Blk Time (%)		0		0	0	
Queuing Penalty (veh)		0		0	0	
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 2: N Washington St/SR 49 & W School St

Movement	EB	NB	SB
Directions Served	LR	T	TR
Maximum Queue (ft)	58	104	94
Average Queue (ft)	29	22	27
95th Queue (ft)	53	68	75
Link Distance (ft)	1142	763	50
Upstream Blk Time (%)			1
Queuing Penalty (veh)			8
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 9

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Denied Del/Veh (s)	0.2	0.4	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	0.4
Total Del/Veh (s)	4.1	7.3	3.9	2.7	3.8	3.5	1.6	1.6	2.6	1.6	6.7	7.7
Vehicles Exited	18	1	184	5	1	8	25	146	468	28	30	434
Hourly Exit Rate	18	1	184	5	1	8	25	146	468	28	30	434
Input Volume	17	1	188	8	1	8	25	142	467	25	31	444
% of Volume	106	100	98	62	100	100	100	103	100	112	97	98

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	SBR	All
Denied Del/Veh (s)	0.4	0.2
Total Del/Veh (s)	6.2	4.4
Vehicles Exited	24	1372
Hourly Exit Rate	24	1372
Input Volume	25	1382
% of Volume	96	99

2: N Washington St/SR 49 & W School St Performance by movement

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.5	0.0	0.1	0.3
Total Del/Veh (s)	5.9	4.4	6.6	2.1	1.3	4.3
Vehicles Exited	26	28	642	576	79	1351
Hourly Exit Rate	26	28	642	576	79	1351
Input Volume	27	30	632	595	75	1359
% of Volume	96	93	102	97	105	99

Total Network Performance

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)				0.4		
Total Del/Veh (s)				9.3		
Vehicles Exited				1409		
Hourly Exit Rate				1409		
Input Volume				4227		
% of Volume				33		

Queuing and Blocking Report
Future 2045 School RAB

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	WB	NB	NB	SB
Directions Served	LTR	LTR	UL	TR	LTR
Maximum Queue (ft)	90	38	42	55	149
Average Queue (ft)	33	5	6	15	58
95th Queue (ft)	69	24	29	48	115
Link Distance (ft)	1149	64	50	50	1131
Upstream Blk Time (%)		0	0	0	
Queuing Penalty (veh)		0	0	1	
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: N Washington St/SR 49 & W School St

Movement	EB	NB	SB
Directions Served	LR	T	TR
Maximum Queue (ft)	65	124	88
Average Queue (ft)	29	32	23
95th Queue (ft)	55	89	70
Link Distance (ft)	1142	763	50
Upstream Blk Time (%)			1
Queuing Penalty (veh)			7
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 8

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.1	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.4	0.3	0.4
Total Del/Veh (s)	3.5	3.6	3.1	2.1	3.2	1.5	1.5	2.5	1.6	5.8	6.9	5.5
Vehicles Exited	14	208	9	1	15	14	146	448	13	6	390	14
Hourly Exit Rate	14	208	9	1	15	14	146	448	13	6	390	14
Input Volume	15	205	11	1	13	15	143	457	13	6	388	15
% of Volume	93	101	82	100	115	93	102	98	100	100	101	93

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.2
Total Del/Veh (s)	4.0
Vehicles Exited	1278
Hourly Exit Rate	1278
Input Volume	1282
% of Volume	100

2: N Washington St/SR 49 & W School St Performance by movement

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.5	0.0	0.0	0.3
Total Del/Veh (s)	6.1	4.3	6.5	2.0	1.2	4.2
Vehicles Exited	23	27	598	577	50	1275
Hourly Exit Rate	23	27	598	577	50	1275
Input Volume	27	26	600	579	45	1277
% of Volume	85	104	100	100	111	100

Total Network Performance

Movement	All
Denied Del/Veh (s)	0.4
Total Del/Veh (s)	8.9
Vehicles Exited	1312
Hourly Exit Rate	1312
Input Volume	3910
% of Volume	34

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	WB	NB	NB	SB
Directions Served	LTR	LTR	UL	TR	LTR
Maximum Queue (ft)	90	55	36	51	129
Average Queue (ft)	30	10	3	7	44
95th Queue (ft)	67	38	21	32	98
Link Distance (ft)	1149	64	50	50	1131
Upstream Blk Time (%)		0	0	0	
Queuing Penalty (veh)		0	0	0	
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: N Washington St/SR 49 & W School St

Movement	EB	NB	SB
Directions Served	LR	T	TR
Maximum Queue (ft)	66	112	89
Average Queue (ft)	28	29	24
95th Queue (ft)	54	84	68
Link Distance (ft)	1142	763	50
Upstream Blk Time (%)			1
Queuing Penalty (veh)			7
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 7

**Appendix H. Cumulative (2040) with Bypass Build Conditions with 5-Legged Roundabout
– SIDRA Outputs**

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road AM (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: SR 49 (Washington St)															
Lane 1 ^d	602	3.0	602	3.0	1240	0.486	100	7.4	LOS A	4.1	105.7	Full	404	0.0	0.0
Approach	602	3.0	602	3.0		0.486		7.4	LOS A	4.1	105.7				
NorthEast: Columbia Way															
Lane 1 ^d	30	3.0	30	3.0	722	0.042	100	5.4	LOS A	0.2	5.7	Full	780	0.0	0.0
Approach	30	3.0	30	3.0		0.042		5.4	LOS A	0.2	5.7				
North: SR 49															
Lane 1 ^d	421	3.0	421	3.0	1009	0.417	100	8.1	LOS A	2.8	71.0	Full	1293	0.0	0.0
Approach	421	3.0	421	3.0		0.417		8.1	LOS A	2.8	71.0				
NorthWest: Shaws Flat Rd															
Lane 1 ^d	293	3.0	293	3.0	806	0.364	100	8.8	LOS A	2.4	60.4	Full	1180	0.0	0.0
Approach	293	3.0	293	3.0		0.364		8.8	LOS A	2.4	60.4				
SouthWest: School St															
Lane 1 ^d	70	3.0	70	3.0	688	0.101	100	6.4	LOS A	0.6	14.5	Full	1061	0.0	0.0
Approach	70	3.0	70	3.0		0.101		6.4	LOS A	0.6	14.5				
All Vehicles	1416	3.0	1416	3.0		0.486		7.8	LOS A	4.1	105.7				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)											
South: SR 49 (Washington St)											
Mov.	L3	L1	T1	Total	%HV						
From S To Exit:	SW	NW	N			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	68	166	367	602	3.0	1240	0.486	100	NA	NA	
Approach	68	166	367	602	3.0		0.486				
NorthEast: Columbia Way											
Mov.	L1	T1	R2	R3	Total	%HV	Deg.	Lane	Prob.	Ov.	

From NE To Exit:	S	SW	NW	N			Cap. veh/h	Satn v/c	Util. %	SL %	Ov. %	Lane No.
Lane 1	13	1	4	12	30	3.0	722	0.042	100	NA	NA	
Approach	13	1	4	12	30	3.0		0.042				
North: SR 49												
Mov.	T1	R1	R3	Total	%HV							
From N To Exit:	S	SW	NW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	374	32	15	421	3.0		1009	0.417	100	NA	NA	
Approach	374	32	15	421	3.0			0.417				
NorthWest: Shaws Flat Rd												
Mov.	L3	R1	R2	Total	%HV							
From NW To Exit:	N	S	SW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	18	274	1	293	3.0		806	0.364	100	NA	NA	
Approach	18	274	1	293	3.0			0.364				
SouthWest: School St												
Mov.	L2	L1	R3	Total	%HV							
From SW To Exit:	NW	N	S				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	7	14	49	70	3.0		688	0.101	100	NA	NA	
Approach	7	14	49	70	3.0			0.101				
Total %HV Deg.Satn (v/c)												
All Vehicles	1416	3.0		0.486								

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: SR 49 (Washington St)				
Lane 1	0.0	0.0	0.0	0.0
NorthEast: Columbia Way				
Lane 1	0.0	0.0	0.0	0.0
North: SR 49				
Lane 1	0.0	0.0	0.0	0.0
NorthWest: Shaws Flat Rd				
Lane 1	0.0	0.0	0.0	0.0
SouthWest: School St				
Lane 1	0.0	0.0	0.0	0.0

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Project: K:\LDT_TRANS\199023002 - TCTC SR49 Gold Rush\ISOAP_SIDRA\Sonora ISOAP- Bean.sip9

LANE SUMMARY

 **Site: 101 [SR 49 & Shaws Flat Road School (Site Folder: General)]**

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: SR 49 (Washington St)															
Lane 1 ^d	660	3.0	660	3.0	1226	0.538	100	8.3	LOS A	5.0	128.4	Full	404	0.0	0.0
Approach	660	3.0	660	3.0		0.538		8.3	LOS A	5.0	128.4				
NorthEast: Columbia Way															
Lane 1 ^d	18	3.0	18	3.0	666	0.028	100	5.7	LOS A	0.2	3.9	Full	780	0.0	0.0
Approach	18	3.0	18	3.0		0.028		5.7	LOS A	0.2	3.9				
North: SR 49															
Lane 1 ^d	543	3.0	543	3.0	1068	0.509	100	9.2	LOS A	3.9	99.4	Full	1293	0.0	0.0
Approach	543	3.0	543	3.0		0.509		9.2	LOS A	3.9	99.4				
NorthWest: Shaws Flat Rd															
Lane 1 ^d	224	3.0	224	3.0	747	0.300	100	8.3	LOS A	1.9	48.4	Full	1180	0.0	0.0
Approach	224	3.0	224	3.0		0.300		8.3	LOS A	1.9	48.4				
SouthWest: School St															
Lane 1 ^d	62	3.0	62	3.0	663	0.093	100	6.5	LOS A	0.5	13.6	Full	1061	0.0	0.0
Approach	62	3.0	62	3.0		0.093		6.5	LOS A	0.5	13.6				
All Vehicles	1508	3.0	1508	3.0		0.538		8.5	LOS A	5.0	128.4				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)											
South: SR 49 (Washington St)											
Mov.	L3	L1	T1	Total	%HV						
From S To Exit:	SW	NW	N			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	27	148	485	660	3.0	1226	0.538	100	NA	NA	
Approach	27	148	485	660	3.0		0.538				
NorthEast: Columbia Way											
Mov.	L1	T1	R2	R3	Total	%HV	Deg.	Lane	Prob.	Ov.	

From NE To Exit:	S	SW	NW	N			Cap. veh/h	Satn v/c	Util. %	SL %	Ov. %	Lane No.
Lane 1	8	1	1	9	18	3.0	666	0.028	100	NA	NA	
Approach	8	1	1	9	18	3.0		0.028				
North: SR 49												
Mov.	T1	R1	R3	Total	%HV							
From N To Exit:	S	SW	NW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	478	38	27	543	3.0		1068	0.509	100	NA	NA	
Approach	478	38	27	543	3.0			0.509				
NorthWest: Shaws Flat Rd												
Mov.	L3	R1	R2	Total	%HV							
From NW To Exit:	N	S	SW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	18	189	16	224	3.0		747	0.300	100	NA	NA	
Approach	18	189	16	224	3.0			0.300				
SouthWest: School St												
Mov.	L2	L1	R3	Total	%HV							
From SW To Exit:	NW	N	S				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	7	23	33	62	3.0		663	0.093	100	NA	NA	
Approach	7	23	33	62	3.0			0.093				
Total %HV Deg.Satn (v/c)												
All Vehicles	1508	3.0		0.538								

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: SR 49 (Washington St)				
Lane 1	0.0	0.0	0.0	0.0
NorthEast: Columbia Way				
Lane 1	0.0	0.0	0.0	0.0
North: SR 49				
Lane 1	0.0	0.0	0.0	0.0
NorthWest: Shaws Flat Rd				
Lane 1	0.0	0.0	0.0	0.0
SouthWest: School St				
Lane 1	0.0	0.0	0.0	0.0

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Project: K:\LDT_TRANS\199023002 - TCTC SR49 Gold Rush\ISOAP_SIDRA\Sonora ISOAP- Bean.sip9

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road PM (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: SR 49 (Washington St)															
Lane 1 ^d	638	3.0	638	3.0	1230	0.519	100	8.0	LOS A	4.7	120.1	Full	404	0.0	0.0
Approach	638	3.0	638	3.0		0.519		8.0	LOS A	4.7	120.1				
NorthEast: Columbia Way															
Lane 1 ^d	27	3.0	27	3.0	688	0.039	100	5.7	LOS A	0.2	5.5	Full	780	0.0	0.0
Approach	27	3.0	27	3.0		0.039		5.7	LOS A	0.2	5.5				
North: SR 49															
Lane 1 ^d	445	3.0	445	3.0	1077	0.413	100	7.6	LOS A	2.8	72.3	Full	1293	0.0	0.0
Approach	445	3.0	445	3.0		0.413		7.6	LOS A	2.8	72.3				
NorthWest: Shaws Flat Rd															
Lane 1 ^d	239	3.0	239	3.0	831	0.288	100	7.5	LOS A	1.8	45.4	Full	1180	0.0	0.0
Approach	239	3.0	239	3.0		0.288		7.5	LOS A	1.8	45.4				
SouthWest: School St															
Lane 1 ^d	58	3.0	58	3.0	711	0.081	100	5.9	LOS A	0.4	11.4	Full	1061	0.0	0.0
Approach	58	3.0	58	3.0		0.081		5.9	LOS A	0.4	11.4				
All Vehicles	1407	3.0	1407	3.0		0.519		7.7	LOS A	4.7	120.1				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)											
South: SR 49 (Washington St)											
Mov.	L3	L1	T1	Total	%HV						
From S To Exit:	SW	NW	N			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	16	149	473	638	3.0	1230	0.519	100	NA	NA	
Approach	16	149	473	638	3.0		0.519				
NorthEast: Columbia Way											
Mov.	L1	T1	R2	R3	Total	%HV	Deg.	Lane	Prob.	Ov.	

From NE To Exit:	S	SW	NW	N			Cap. veh/h	Satn v/c	Util. %	SL %	Ov. %	Lane No.
Lane 1	11	1	1	14	27	3.0	688	0.039	100	NA	NA	
Approach	11	1	1	14	27	3.0		0.039				
North: SR 49												
Mov.	T1	R1	R3	Total	%HV							
From N To Exit:	S	SW	NW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	407	22	16	445	3.0		1077	0.413	100	NA	NA	
Approach	407	22	16	445	3.0			0.413				
NorthWest: Shaws Flat Rd												
Mov.	L3	R1	R2	Total	%HV							
From NW To Exit:	N	S	SW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	16	211	12	239	3.0		831	0.288	100	NA	NA	
Approach	16	211	12	239	3.0			0.288				
SouthWest: School St												
Mov.	L2	L1	R3	Total	%HV							
From SW To Exit:	NW	N	S				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	7	23	28	58	3.0		711	0.081	100	NA	NA	
Approach	7	23	28	58	3.0			0.081				
Total %HV Deg.Satn (v/c)												
All Vehicles	1407	3.0		0.519								

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: SR 49 (Washington St)				
Lane 1	0.0	0.0	0.0	0.0
NorthEast: Columbia Way				
Lane 1	0.0	0.0	0.0	0.0
North: SR 49				
Lane 1	0.0	0.0	0.0	0.0
NorthWest: Shaws Flat Rd				
Lane 1	0.0	0.0	0.0	0.0
SouthWest: School St				
Lane 1	0.0	0.0	0.0	0.0

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Appendix I. Cumulative (2040) without Bypass Build Conditions with 4-Legged and 5-Legged Roundabouts – SimTraffic and SIDRA Outputs

SITE LAYOUT

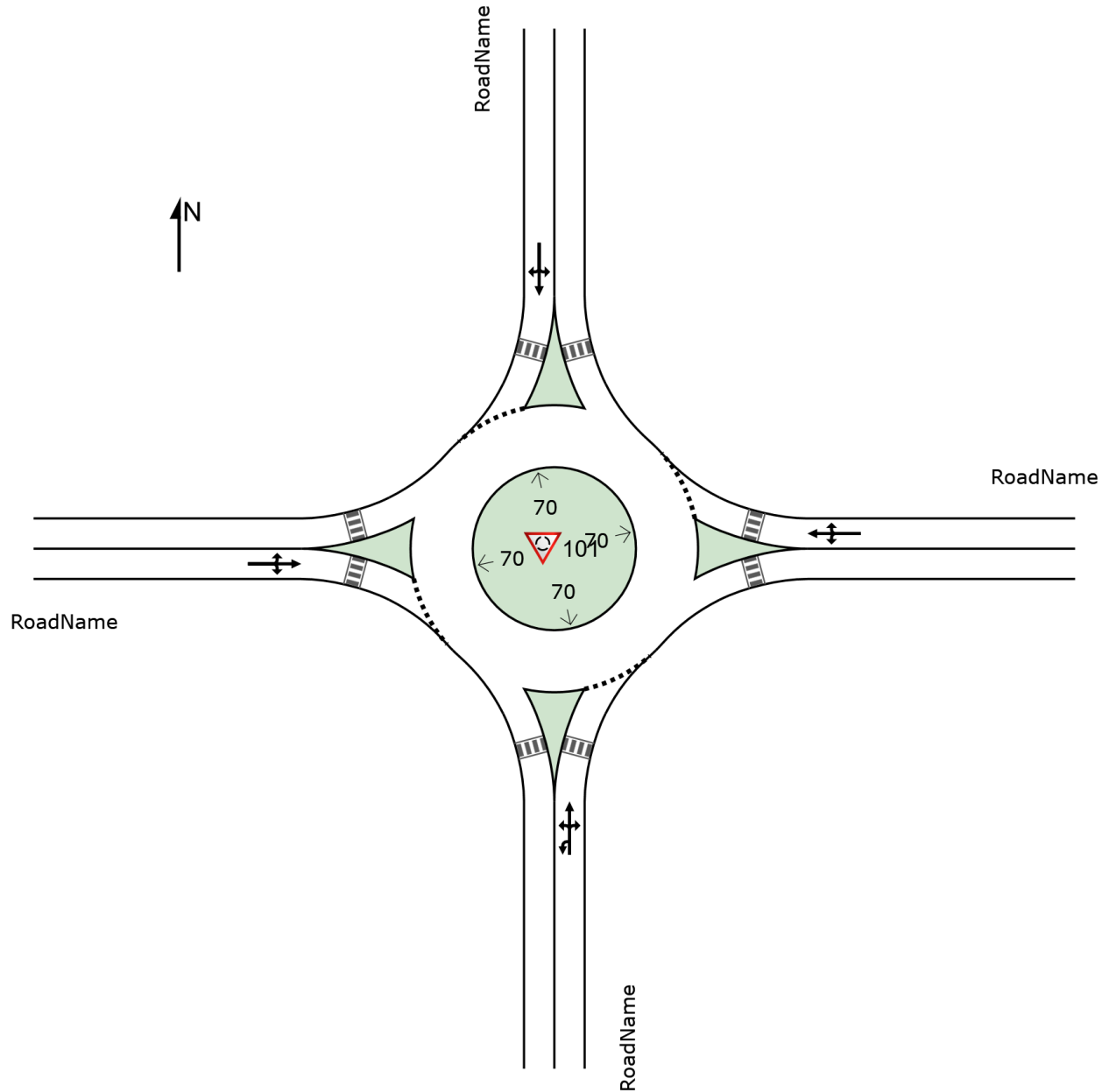
 **Site: 101 [SR 49 & Shaws Flat Road AM (Site Folder: AM)]**

New Site

Site Category: (None)

Roundabout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Project: K:\LDT_TRANS\199023002 - TCTC SR49 Gold Rush\ISOAP_SIDRA\Sonora ISOAP - 4 Leg - Existing.sip9

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road AM (Site Folder: AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Design Life Analysis (Final Year): Results for 1 years

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: RoadName															
Lane 1 ^d	871	3.0	871	3.0	1239	0.703	100	11.0	LOS B	9.8	251.0	Full	100	0.0	51.1
Approach	871	3.0	871	3.0		0.703		11.0	LOS B	9.8	251.0				
East: RoadName															
Lane 1 ^d	21	3.0	21	3.0	540	0.039	100	7.1	LOS A	0.2	5.9	Full	780	0.0	0.0
Approach	21	3.0	21	3.0		0.039		7.1	LOS A	0.2	5.9				
North: RoadName															
Lane 1 ^d	818	3.0	818	3.0	900	0.909	100	31.6	LOS D	26.6	680.7	Full	1293	0.0	0.0
Approach	818	3.0	818	3.0		0.909		31.6	LOS D	26.6	680.7				
West: RoadName															
Lane 1 ^d	310	3.0	310	3.0	427	0.726	100	30.4	LOS D	7.9	203.3	Full	1180	0.0	0.0
Approach	310	3.0	310	3.0		0.726		30.4	LOS D	7.9	203.3				
All Vehicles	2020	3.0	2020	3.0		0.909		22.3	LOS C	26.6	680.7				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)												
South: RoadName												
Mov.	U	L2	T1	R2	Total	%HV						
From S							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N	E			veh/h	Satn v/c	Util. %	SL Ov. %	Lane No.	
Lane 1	96	259	495	21	871	3.0	1239	0.703	100	NA	NA	
Approach	96	259	495	21	871	3.0		0.703				
East: RoadName												
Mov.	L2	T1	R2	Total	%HV							
From E							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N				veh/h	Satn v/c	Util. %	SL Ov. %	Lane No.	
Lane 1	16	1	4	21	3.0		540	0.039	100	NA	NA	

Approach	16	1	4	21	3.0						0.039
North: RoadName											
Mov.	L2	T1	R2	Total	%HV						
From N To Exit:	E	S	W			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	15	768	35	818	3.0	900	0.909	100	NA	NA	
Approach	15	768	35	818	3.0		0.909				
West: RoadName											
Mov.	L2	T1	R2	Total	%HV						
From W To Exit:	N	E	S			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	20	1	289	310	3.0	427	0.726	100	NA	NA	
Approach	20	1	289	310	3.0		0.726				
Total %HV Deg.Satn (v/c)											
All Vehicles	2020	3.0			0.909						

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane % veh/h	Opposing Flow Rate pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Capacity Flow Rate veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec		
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
East: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road School (Site Folder: School)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: RoadName															
Lane 1 ^d	785	3.0	785	3.0	1260	0.623	100	8.7	LOS A	7.3	187.4	Full	100	0.0	28.7
Approach	785	3.0	785	3.0		0.623		8.7	LOS A	7.3	187.4				
East: RoadName															
Lane 1 ^d	18	3.0	18	3.0	623	0.030	100	6.1	LOS A	0.2	4.2	Full	780	0.0	0.0
Approach	18	3.0	18	3.0		0.030		6.1	LOS A	0.2	4.2				
North: RoadName															
Lane 1 ^d	603	3.0	603	3.0	1065	0.567	100	10.3	LOS B	4.6	117.5	Full	1293	0.0	0.0
Approach	603	3.0	603	3.0		0.567		10.3	LOS B	4.6	117.5				
West: RoadName															
Lane 1 ^d	172	3.0	172	3.0	684	0.251	100	8.3	LOS A	1.6	40.2	Full	1180	0.0	0.0
Approach	172	3.0	172	3.0		0.251		8.3	LOS A	1.6	40.2				
All Vehicles	1578	3.0	1578	3.0		0.623		9.2	LOS A	7.3	187.4				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)												
South: RoadName												
Mov.	U	L2	T1	R2	Total	%HV						
From S							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N	E			veh/h	satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	29	155	584	16	785	3.0	1260	0.623	100	NA	NA	
Approach	29	155	584	16	785	3.0		0.623				
East: RoadName												
Mov.	L2	T1	R2	Total	%HV							
From E							Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	S	W	N				veh/h	satn	Util.	SL	Ov.	Lane
								v/c	%	%		No.
Lane 1	12	1	5	18	3.0		623	0.030	100	NA	NA	

Approach	12	1	5	18	3.0		0.030				
North: RoadName											
Mov.	L2	T1	R2	Total	%HV						
From N						Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	E	S	W			veh/h	Satn	Util.	SL	SL	Lane
							v/c	%	%		No.
Lane 1	7	580	16	603	3.0	1065	0.567	100	NA	NA	
Approach	7	580	16	603	3.0		0.567				
West: RoadName											
Mov.	L2	T1	R2	Total	%HV						
From W						Cap.	Deg.	Lane	Prob.	Ov.	
To Exit:	N	E	S			veh/h	Satn	Util.	SL	SL	Lane
							v/c	%	%		No.
Lane 1	15	1	155	172	3.0	684	0.251	100	NA	NA	
Approach	15	1	155	172	3.0		0.251				
Total %HV Deg.Satn (v/c)											
All Vehicles	1578	3.0		0.623							

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length	Percent Opng in Lane	Flow Rate	Opposing Flow Rate	Critical Gap	Follow-up Headway	Lane Capacity	Deg. Satn	Min. Delay	Merge Delay	
		ft		% veh/h	pcu/h	sec	sec	veh/h	veh/h	v/c	sec	sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand	Residual Queued Demand	Time for Residual Demand to Clear	Duration of Oversatn
	veh	veh	sec	sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
East: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road PM (Site Folder: PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h]	[HV %]	[Total veh/h]	[HV %]	veh/h	v/c	%	sec		[Veh]	[Dist] ft		ft	%	%
South: RoadName															
Lane 1 ^d	868	3.0	868	3.0	1268	0.685	100	9.2	LOS A	9.4	240.5	Full	100	0.0	46.4
Approach	868	3.0	868	3.0		0.685		9.2	LOS A	9.4	240.5				
East: RoadName															
Lane 1 ^d	30	3.0	30	3.0	580	0.052	100	6.8	LOS A	0.3	7.7	Full	780	0.0	0.0
Approach	30	3.0	30	3.0		0.052		6.8	LOS A	0.3	7.7				
North: RoadName															
Lane 1 ^d	575	3.0	575	3.0	1056	0.544	100	9.9	LOS A	4.3	108.8	Full	1293	0.0	0.0
Approach	575	3.0	575	3.0		0.544		9.9	LOS A	4.3	108.8				
West: RoadName															
Lane 1 ^d	167	3.0	167	3.0	705	0.238	100	7.9	LOS A	1.5	37.5	Full	1180	0.0	0.0
Approach	167	3.0	167	3.0		0.238		7.9	LOS A	1.5	37.5				
All Vehicles	1641	3.0	1641	3.0		0.685		9.3	LOS A	9.4	240.5				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)												
South: RoadName												
Mov.	U	L2	T1	R2	Total	%HV		Deg. Satn	Lane Util.	Prob. SL	Ov. Lane	
From S	S	W	N	E			Cap. veh/h	v/c	%	%	No.	
To Exit:												
Lane 1	21	164	661	23	868	3.0	1268	0.685	100	NA	NA	
Approach	21	164	661	23	868	3.0		0.685				
East: RoadName												
Mov.	L2	T1	R2	Total	%HV		Deg. Satn	Lane Util.	Prob. SL	Ov. Lane		
From E	S	W	N				Cap. veh/h	v/c	%	%	No.	
To Exit:												
Lane 1	22	1	8	30	3.0		580	0.052	100	NA	NA	
Approach	22	1	8	30	3.0			0.052				

North: RoadName										
Mov.	L2	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.
From N						Cap.	Satn	Util.	SL Ov.	Lane
To Exit:	E	S	W			veh/h	v/c	%	%	No.
Lane 1	5	554	15	575	3.0	1056	0.544	100	NA	NA
Approach	5	554	15	575	3.0		0.544			
West: RoadName										
Mov.	L2	T1	R2	Total	%HV		Deg.	Lane	Prob.	Ov.
From W						Cap.	Satn	Util.	SL Ov.	Lane
To Exit:	N	E	S			veh/h	v/c	%	%	No.
Lane 1	11	1	155	167	3.0	705	0.238	100	NA	NA
Approach	11	1	155	167	3.0		0.238			
Total %HV Deg.Satn (v/c)										
All Vehicles	1641	3.0		0.685						

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane %	Opposing Flow Rate veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Capacity Flow Rate veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec	
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: RoadName				
Lane 1	0.0	0.0	0.0	0.0
East: RoadName				
Lane 1	0.0	0.0	0.0	0.0
North: RoadName				
Lane 1	0.0	0.0	0.0	0.0
West: RoadName				
Lane 1	0.0	0.0	0.0	0.0

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.2	0.2	0.0		0.0	0.0	0.0	0.0	0.0	0.7	0.5	0.5
Total Del/Veh (s)	6.1	5.2	5.2		3.7	1.5	1.5	2.6	1.5	29.3	25.3	23.3
Vehicles Exited	15	229	12	0	4	78	205	406	18	11	616	30
Hourly Exit Rate	15	229	12	0	4	78	205	406	18	11	616	30
Input Volume	16	231	13	1	3	77	207	396	17	12	614	28
% of Volume	94	99	92	0	133	101	99	103	106	92	100	107

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	All
Denied Del/Veh (s)	0.2
Total Del/Veh (s)	12.0
Vehicles Exited	1624
Hourly Exit Rate	1624
Input Volume	1615
% of Volume	101

2: N Washington St/SR 49 & W School St Performance by movement

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.6	0.1	0.0	0.3
Total Del/Veh (s)	5.7	4.5	6.9	2.9	2.1	4.6
Vehicles Exited	16	37	691	776	164	1684
Hourly Exit Rate	16	37	691	776	164	1684
Input Volume	17	40	680	780	161	1678
% of Volume	94	92	102	99	102	100

Total Network Performance

Movement	All
Denied Del/Veh (s)	0.5
Total Del/Veh (s)	17.4
Vehicles Exited	1669
Hourly Exit Rate	1669
Input Volume	4994
% of Volume	33

Queuing and Blocking Report
Existing 2025 AM RAB

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	WB	NB	NB	SB
Directions Served	LTR	LTR	UL	TR	LTR
Maximum Queue (ft)	106	63	56	55	438
Average Queue (ft)	46	10	5	8	185
95th Queue (ft)	79	38	29	34	410
Link Distance (ft)	1149	64	50	50	1131
Upstream Blk Time (%)		0	0	0	
Queuing Penalty (veh)		0	0	0	
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: N Washington St/SR 49 & W School St

Movement	EB	NB	SB
Directions Served	LR	T	TR
Maximum Queue (ft)	59	130	97
Average Queue (ft)	29	33	37
95th Queue (ft)	52	93	91
Link Distance (ft)	1142	763	50
Upstream Blk Time (%)			2
Queuing Penalty (veh)			18
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 19

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Denied Del/Veh (s)	0.2		0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.7	0.4
Total Del/Veh (s)	3.7		3.6	3.2	3.9	4.2	1.4	1.5	2.5	1.5	7.7	8.6
Vehicles Exited	12	0	146	9	1	5	28	140	529	15	5	540
Hourly Exit Rate	12	0	146	9	1	5	28	140	529	15	5	540
Input Volume	14	1	143	11	1	5	27	143	537	15	6	534
% of Volume	86	0	102	82	100	100	104	98	99	100	83	101

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	SBR	All
Denied Del/Veh (s)	0.5	0.2
Total Del/Veh (s)	8.4	4.9
Vehicles Exited	13	1443
Hourly Exit Rate	13	1443
Input Volume	15	1452
% of Volume	87	99

2: N Washington St/SR 49 & W School St Performance by movement

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.6	0.0	0.0	0.3
Total Del/Veh (s)	5.9	4.5	7.0	2.3	1.4	4.6
Vehicles Exited	29	32	683	639	87	1470
Hourly Exit Rate	29	32	683	639	87	1470
Input Volume	30	33	692	637	81	1473
% of Volume	97	97	99	100	107	100

Total Network Performance

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)				0.5		
Total Del/Veh (s)				10.2		
Vehicles Exited				1480		
Hourly Exit Rate				1480		
Input Volume				4449		
% of Volume				33		

Queuing and Blocking Report
Existing 2025 School RAB

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	WB	NB	NB	SB
Directions Served	LTR	LTR	UL	TR	LTR
Maximum Queue (ft)	67	51	42	54	175
Average Queue (ft)	30	8	4	7	67
95th Queue (ft)	58	35	25	34	135
Link Distance (ft)	1149	64	50	50	1131
Upstream Blk Time (%)		0	0	0	
Queuing Penalty (veh)		0	0	1	
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: N Washington St/SR 49 & W School St

Movement	EB	NB	SB
Directions Served	LR	T	TR
Maximum Queue (ft)	64	138	98
Average Queue (ft)	31	35	32
95th Queue (ft)	53	96	82
Link Distance (ft)	1142	763	50
Upstream Blk Time (%)			2
Queuing Penalty (veh)			12
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 12

1: SR 49 & Shaws Flat Rd/Columbia Way Performance by movement

Movement	EBL	EBR	WBL	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.5	0.4	0.4	0.2
Total Del/Veh (s)	3.1	3.4	3.2	4.3	1.4	1.5	2.6	1.5	6.8	8.5	6.2	4.6
Vehicles Exited	8	132	17	8	15	155	613	24	5	507	14	1498
Hourly Exit Rate	8	132	17	8	15	155	613	24	5	507	14	1498
Input Volume	10	143	20	7	19	151	609	21	5	510	14	1509
% of Volume	80	92	85	114	79	103	101	114	100	99	100	99

2: N Washington St/SR 49 & W School St Performance by movement

Movement	EBL	EBR	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.1	0.1	0.6	0.0	0.1	0.4
Total Del/Veh (s)	5.9	4.0	7.6	2.1	1.3	5.0
Vehicles Exited	23	22	783	618	58	1504
Hourly Exit Rate	23	22	783	618	58	1504
Input Volume	25	24	774	637	58	1518
% of Volume	92	92	101	97	100	99

Total Network Performance

Denied Del/Veh (s)	0.5
Total Del/Veh (s)	10.3
Vehicles Exited	1524
Hourly Exit Rate	1524
Input Volume	4612
% of Volume	33

Queuing and Blocking Report
Existing 2025 AM RAB

07/08/2025

Intersection: 1: SR 49 & Shaws Flat Rd/Columbia Way

Movement	EB	WB	NB	NB	SB
Directions Served	LTR	LTR	UL	TR	LTR
Maximum Queue (ft)	59	69	49	59	180
Average Queue (ft)	24	13	4	8	68
95th Queue (ft)	53	45	24	37	143
Link Distance (ft)	1149	64	50	50	1131
Upstream Blk Time (%)		0	0	0	
Queuing Penalty (veh)		0	0	1	
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 2: N Washington St/SR 49 & W School St

Movement	EB	NB	SB
Directions Served	LR	T	TR
Maximum Queue (ft)	62	127	98
Average Queue (ft)	27	43	26
95th Queue (ft)	54	108	73
Link Distance (ft)	1142	763	50
Upstream Blk Time (%)			1
Queuing Penalty (veh)			7
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 8

SITE LAYOUT

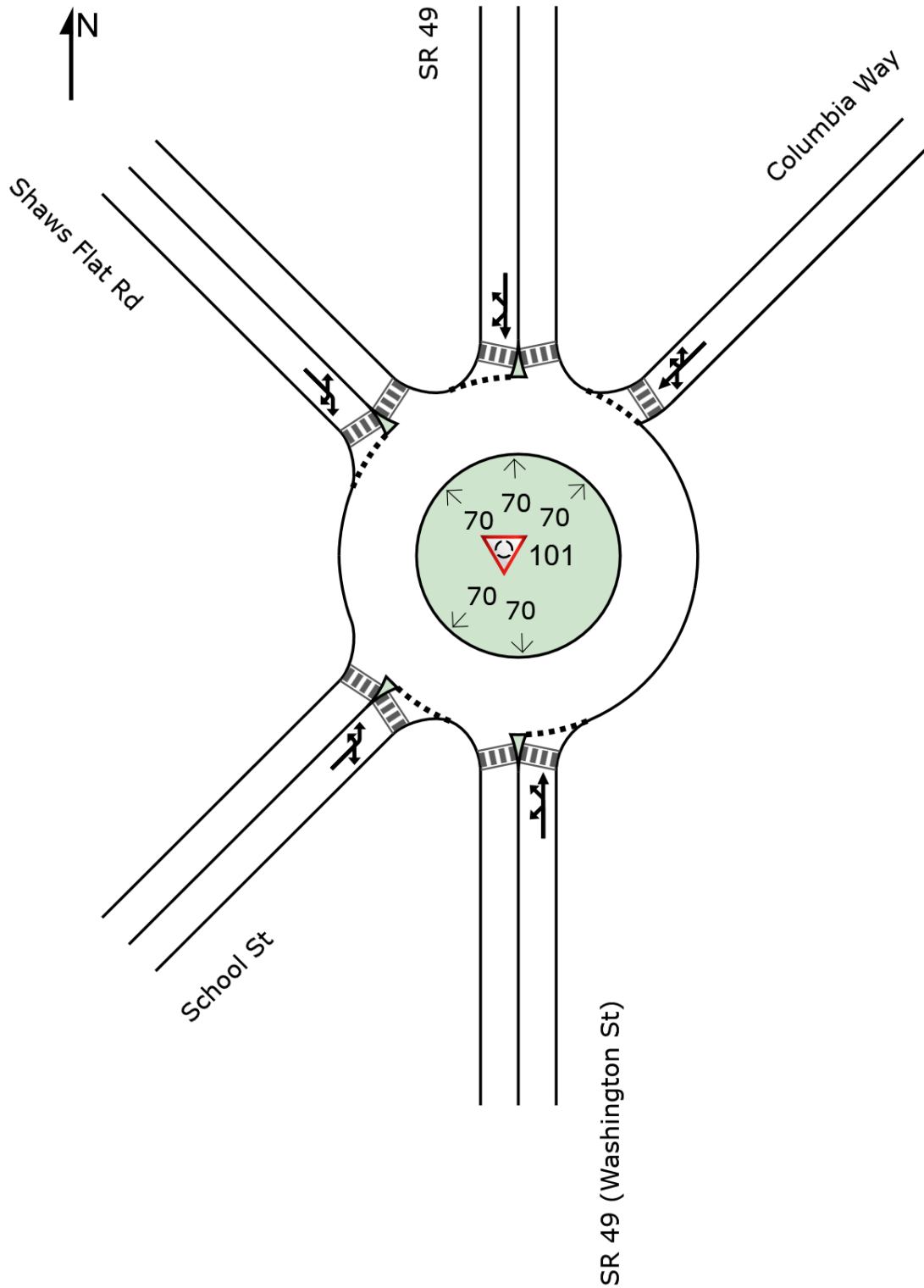
 Site: 101 [SR 49 & Shaws Flat Road AM (Site Folder: General)]

New Site

Site Category: (None)

Roundabout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Project: K:\LDT_TRANS\199023002 - TCTC SR49 Gold Rush\ISOAP_SIDRA\Sonora ISOAP- Bean - Existing.sip9

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road AM (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: SR 49 (Washington St)															
Lane 1 ^d	721	3.0	721	3.0	1242	0.580	100	8.7	LOS A	6.0	152.8	Full	404	0.0	0.0
Approach	721	3.0	721	3.0		0.580		8.7	LOS A	6.0	152.8				
NorthEast: Columbia Way															
Lane 1 ^d	18	3.0	18	3.0	634	0.029	100	6.0	LOS A	0.2	4.1	Full	780	0.0	0.0
Approach	18	3.0	18	3.0		0.029		6.0	LOS A	0.2	4.1				
North: SR 49															
Lane 1 ^d	711	3.0	711	3.0	950	0.749	100	17.3	LOS C	13.2	337.2	Full	1293	0.0	0.0
Approach	711	3.0	711	3.0		0.749		17.3	LOS C	13.2	337.2				
NorthWest: Shaws Flat Rd															
Lane 1 ^d	268	3.0	268	3.0	534	0.503	100	15.7	LOS C	4.2	108.2	Full	1180	0.0	0.0
Approach	268	3.0	268	3.0		0.503		15.7	LOS C	4.2	108.2				
SouthWest: School St															
Lane 1 ^d	62	3.0	62	3.0	498	0.124	100	8.9	LOS A	0.8	20.2	Full	1061	0.0	0.0
Approach	62	3.0	62	3.0		0.124		8.9	LOS A	0.8	20.2				
All Vehicles	1780	3.0	1780	3.0		0.749		13.2	LOS B	13.2	337.2				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)											
South: SR 49 (Washington St)											
Mov.	L3	L1	T1	Total	%HV						
From S To Exit:	SW	NW	N			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	84	218	418	721	3.0	1242	0.580	100	NA	NA	
Approach	84	218	418	721	3.0		0.580				
NorthEast: Columbia Way											
Mov.	L1	T1	R2	R3	Total	%HV	Deg.	Lane	Prob.	Ov.	

From NE To Exit:	S	SW	NW	N			Cap. veh/h	Satn v/c	Util. %	SL %	Ov. %	Lane No.
Lane 1	13	1	1	3	18	3.0	634	0.029	100	NA	NA	
Approach	13	1	1	3	18	3.0		0.029				
North: SR 49												
Mov.	T1	R1	R3	Total	%HV							
From N To Exit:	S	SW	NW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	615	65	30	711	3.0		950	0.749	100	NA	NA	
Approach	615	65	30	711	3.0			0.749				
NorthWest: Shaws Flat Rd												
Mov.	L3	R1	R2	Total	%HV							
From NW To Exit:	N	S	SW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	17	226	25	268	3.0		534	0.503	100	NA	NA	
Approach	17	226	25	268	3.0			0.503				
SouthWest: School St												
Mov.	L2	L1	R3	Total	%HV							
From SW To Exit:	NW	N	S				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	7	12	43	62	3.0		498	0.124	100	NA	NA	
Approach	7	12	43	62	3.0			0.124				
Total %HV Deg.Satn (v/c)												
All Vehicles	1780	3.0				0.749						

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: SR 49 (Washington St)				
Lane 1	0.0	0.0	0.0	0.0
NorthEast: Columbia Way				
Lane 1	0.0	0.0	0.0	0.0
North: SR 49				
Lane 1	0.0	0.0	0.0	0.0
NorthWest: Shaws Flat Rd				
Lane 1	0.0	0.0	0.0	0.0
SouthWest: School St				
Lane 1	0.0	0.0	0.0	0.0

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Project: K:\LDT_TRANS\199023002 - TCTC SR49 Gold Rush\ISOAP_SIDRA\Sonora ISOAP- Bean - Existing.sip9

LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road School (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: SR 49 (Washington St)															
Lane 1 ^d	736	3.0	736	3.0	1225	0.601	100	9.4	LOS A	6.4	163.2	Full	404	0.0	0.0
Approach	736	3.0	736	3.0		0.601		9.4	LOS A	6.4	163.2				
NorthEast: Columbia Way															
Lane 1 ^d	18	3.0	18	3.0	608	0.030	100	6.3	LOS A	0.2	4.4	Full	780	0.0	0.0
Approach	18	3.0	18	3.0		0.030		6.3	LOS A	0.2	4.4				
North: SR 49															
Lane 1 ^d	603	3.0	603	3.0	1060	0.569	100	10.4	LOS B	4.7	120.2	Full	1293	0.0	0.0
Approach	603	3.0	603	3.0		0.569		10.4	LOS B	4.7	120.2				
NorthWest: Shaws Flat Rd															
Lane 1 ^d	172	3.0	172	3.0	682	0.252	100	8.3	LOS A	1.6	40.4	Full	1180	0.0	0.0
Approach	172	3.0	172	3.0		0.252		8.3	LOS A	1.6	40.4				
SouthWest: School St															
Lane 1 ^d	68	3.0	68	3.0	637	0.107	100	6.9	LOS A	0.6	16.1	Full	1061	0.0	0.0
Approach	68	3.0	68	3.0		0.107		6.9	LOS A	0.6	16.1				
All Vehicles	1598	3.0	1598	3.0		0.601		9.5	LOS A	6.4	163.2				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)											
South: SR 49 (Washington St)											
Mov.	L3	L1	T1	Total	%HV						
From S To Exit:	SW	NW	N			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	29	149	558	736	3.0	1225	0.601	100	NA	NA	
Approach	29	149	558	736	3.0		0.601				
NorthEast: Columbia Way											
Mov.	L1	T1	R2	R3	Total	%HV	Deg.	Lane	Prob.	Ov.	

From NE To Exit:	S	SW	NW	N			Cap. veh/h	Satn v/c	Util. %	SL Ov. %	Lane No.
Lane 1	11	1	1	5	18	3.0	608	0.030	100	NA	NA
Approach	11	1	1	5	18	3.0		0.030			
North: SR 49											
Mov.	T1	R1	R3	Total	%HV						
From N To Exit:	S	SW	NW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.
Lane 1	541	46	16	603	3.0		1060	0.569	100	NA	NA
Approach	541	46	16	603	3.0			0.569			
NorthWest: Shaws Flat Rd											
Mov.	L3	R1	R2	Total	%HV						
From NW To Exit:	N	S	SW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.
Lane 1	15	145	12	172	3.0		682	0.252	100	NA	NA
Approach	15	145	12	172	3.0			0.252			
SouthWest: School St											
Mov.	L2	L1	R3	Total	%HV						
From SW To Exit:	NW	N	S				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.
Lane 1	7	26	36	68	3.0		637	0.107	100	NA	NA
Approach	7	26	36	68	3.0			0.107			
Total %HV Deg.Satn (v/c)											
All Vehicles	1598	3.0		0.601							

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h	Opposing Flow Rate pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: SR 49 (Washington St)				
Lane 1	0.0	0.0	0.0	0.0
NorthEast: Columbia Way				
Lane 1	0.0	0.0	0.0	0.0
North: SR 49				
Lane 1	0.0	0.0	0.0	0.0
NorthWest: Shaws Flat Rd				
Lane 1	0.0	0.0	0.0	0.0
SouthWest: School St				
Lane 1	0.0	0.0	0.0	0.0

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LANE SUMMARY

 Site: 101 [SR 49 & Shaws Flat Road PM (Site Folder: General)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site

Site Category: (None)

Roundabout

Lane Use and Performance															
	Demand Flows		Arrival Flows		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Back Of Queue		Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV %	[Total veh/h	HV %	veh/h	v/c	%	sec		[Veh	Dist] ft		ft	%	%
South: SR 49 (Washington St)															
Lane 1 ^d	818	3.0	818	3.0	1238	0.661	100	10.2	LOS B	8.1	208.5	Full	404	0.0	0.0
Approach	818	3.0	818	3.0		0.661		10.2	LOS B	8.1	208.5				
NorthEast: Columbia Way															
Lane 1 ^d	30	3.0	30	3.0	562	0.054	100	7.1	LOS A	0.3	8.1	Full	780	0.0	0.0
Approach	30	3.0	30	3.0		0.054		7.1	LOS A	0.3	8.1				
North: SR 49															
Lane 1 ^d	575	3.0	575	3.0	1051	0.547	100	10.0	LOS A	4.4	111.4	Full	1293	0.0	0.0
Approach	575	3.0	575	3.0		0.547		10.0	LOS A	4.4	111.4				
NorthWest: Shaws Flat Rd															
Lane 1 ^d	166	3.0	166	3.0	704	0.236	100	7.9	LOS A	1.5	37.4	Full	1180	0.0	0.0
Approach	166	3.0	166	3.0		0.236		7.9	LOS A	1.5	37.4				
SouthWest: School St															
Lane 1 ^d	53	3.0	53	3.0	644	0.083	100	6.5	LOS A	0.5	12.2	Full	1061	0.0	0.0
Approach	53	3.0	53	3.0		0.083		6.5	LOS A	0.5	12.2				
All Vehicles	1643	3.0	1643	3.0		0.661		9.7	LOS A	8.1	208.5				

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 6).

Roundabout Capacity Model: SIDRA HCM.

Delay Model: HCM Delay Formula (Stopline Delay: Geometric Delay is not included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

^d Dominant lane on roundabout approach

Approach Lane Flows (veh/h)											
South: SR 49 (Washington St)											
Mov.	L3	L1	T1	Total	%HV						
From S To Exit:	SW	NW	N			Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL Ov. %	Ov. Lane No.	
Lane 1	21	159	639	818	3.0	1238	0.661	100	NA	NA	
Approach	21	159	639	818	3.0		0.661				
NorthEast: Columbia Way											
Mov.	L1	T1	R2	R3	Total	%HV	Deg.	Lane	Prob.	Ov.	

From NE To Exit:	S	SW	NW	N			Cap. veh/h	Satn v/c	Util. %	SL %	Ov. %	Lane No.
Lane 1	21	1	1	8	30	3.0	562	0.054	100	NA	NA	
Approach	21	1	1	8	30	3.0		0.054				
North: SR 49												
Mov.	T1	R1	R3	Total	%HV							
From N To Exit:	S	SW	NW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	527	33	15	575	3.0		1051	0.547	100	NA	NA	
Approach	527	33	15	575	3.0			0.547				
NorthWest: Shaws Flat Rd												
Mov.	L3	R1	R2	Total	%HV							
From NW To Exit:	N	S	SW				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	11	147	9	166	3.0		704	0.236	100	NA	NA	
Approach	11	147	9	166	3.0			0.236				
SouthWest: School St												
Mov.	L2	L1	R3	Total	%HV							
From SW To Exit:	NW	N	S				Cap. veh/h	Deg. Satn v/c	Lane Util. %	Prob. SL %	Ov. %	Ov. Lane No.
Lane 1	5	22	26	53	3.0		644	0.083	100	NA	NA	
Approach	5	22	26	53	3.0			0.083				
Total %HV Deg.Satn (v/c)												
All Vehicles	1643	3.0		0.661								

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Merge Analysis												
	Exit Lane Number	Short Lane Length ft	Percent Opng in Lane ft	Opposing Flow Rate % veh/h	pcu/h	Critical Gap sec	Follow-up Headway sec	Lane Flow Rate veh/h	Capacity veh/h	Deg. Satn v/c	Min. Delay sec	Merge Delay sec
There are no Exit Short Lanes for Merge Analysis at this Site.												

Variable Demand Analysis				
	Initial Queued Demand veh	Residual Queued Demand veh	Time for Residual Demand to Clear sec	Duration of Oversatn sec
South: SR 49 (Washington St)				
Lane 1	0.0	0.0	0.0	0.0
NorthEast: Columbia Way				
Lane 1	0.0	0.0	0.0	0.0
North: SR 49				
Lane 1	0.0	0.0	0.0	0.0
NorthWest: Shaws Flat Rd				
Lane 1	0.0	0.0	0.0	0.0
SouthWest: School St				
Lane 1	0.0	0.0	0.0	0.0

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